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Well R-44 Maintenance Report

EM2023-0241

Well R-44 Maintenance Report

May 2023

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Acronyms and Abbreviations

APV	access port valve
bgs	below ground surface
CV	casing volume
DO	dissolved oxygen
gpm	gallons per minute
I.D.	inside diameter
LANL	Los Alamos National Laboratory
LIC	liquid inflation chamber
NTU	nephelometric turbidity unit
ORP	oxidation-reduction potential
PVC	polyvinyl chloride
SS	stainless steel

1.0 INTRODUCTION

This report presents the well R-44 maintenance activities associated with removing, repairing, and reinstalling the R-44 sampling system performed in October, November, and January 2023. The primary objective of the report is to document the well maintenance activities and the current sampling system configuration in R-44. This work was conducted as prescribed in the October 2022 “Field Implementation Plan for Repair of Wells R-40, R-44, R-49, R-58 and CdV-16-1(i),” which is included as Appendix A. The R-44 sampling system details are presented in Figure 1.0-1 of this report. Figure 1.0-2, Location Map for R-44 and Other Wells Included in Field Implementation Plan, includes the approximate location of R-44.

Planned repairs at R-44 focused on resolving a downhole pressure leak in the sampling system. No exceptions to the field implementation plan occurred.

1.1 Background

Well R-44 is located in a small tributary of Mortandad Canyon in the vicinity of regional well R-13 within Technical Area 05, Los Alamos County, New Mexico. R-44 is used to monitor potential releases of contaminants from Mortandad and Sandia canyon sources, assess the conceptual model for contaminant fate and transport of known chromium contamination beneath Mortandad Canyon, monitor water levels within the regional aquifer, and measure pumping effects from water-supply well PM-5 and other wells in the vicinity. A detailed description of the well installation is in “Completion Report for Regional Aquifer Well R-44” (LANL 2009, 106418). Subsequent maintenance activities are presented in “Well Maintenance Report, R-44 – Mortandad Watershed, December 2008 – February 2021” (Appendix B). Well R-44 was drilled and installed from December 2008 to February 2009. The R-44 borehole was drilled to a total depth of 1094.0 ft below ground surface (bgs). Upon completion of drilling, the groundwater level was measured at 879.1 ft bgs in the borehole.

A 5-in.-inside-diameter (I.D.) stainless-steel well casing with two screened intervals was constructed in the borehole between February 2009 and January 2011. The upper screened interval (screen 1) is 10.0 ft long at a depth of 895.0–905.0 ft bgs. The lower screened interval (screen 2) is 9.9 ft long at a depth of 985.3–995.2 ft bgs. The well screens are separated by an inflatable packer as part of the permanent Baski sampling system to ensure isolation of each screen interval. The sampling system was configured with a shrouded submersible Grundfos 5S30-820CBM pump, one access port valve (APV) for the upper screened interval, and one APV for the lower screened interval. The submersible pump column consisted of threaded and coupled 1-in. I.D. stainless-steel pipe. A weep valve was installed at a depth of 17.4 ft bgs to protect the pump column from freezing. The system included a liquid inflation chamber (LIC) and one Viton-wrapped isolation packer between the screened intervals. To measure water levels in the well, two 1-in. I.D. schedule 80 polyvinyl chloride (PVC) tubes were banded to the pump column for dedicated transducers.

Although well R-44 has remained functional, the sampling system was unable to maintain pressure without the using nitrogen tanks to supply supplemental pressure at the wellhead and preventing cross-flow between screen 1 and screen 2. The Well Maintenance Report in Appendix B indicates that the supplemental pressure has prevented cross-flow up to the time of well repairs.The objectives of the maintenance activities described in this report were to evaluate the cause of sampling system pressure loss, to replace or repair any failed system components, and to reinstall and test the functionality of the sampling system.

2.0 REMOVAL OF DUAL-SCREEN SAMPLING SYSTEM

Transducers were removed from R-44 before October 25, 2022, when mobilization occurred. A pump hoist was used to remove the dual-screen sampling system beginning on October 26, 2022. The drop-pipe, fittings, landing plate, pump shroud, LIC, packer, and APVs appeared to be in good condition.

Visual and pressure tests of the R-44 sampling system inflation lines were conducted as the system was removed. On October 29, 2022, pressure testing of the inflation lines detected a leak at an upper fill plug of the LIC. The leak was confirmed visually using Swagelok Snoop liquid. Because of apparent corrosion and fitting thread damage, it was decided to ship the LIC to Baski, Inc., for repair. The repair was completed and the part was returned on November 8, 2022.

A video log of the 5-in. well casing was performed on October 30, 2022, following sampling system removal. Screens appeared clean with exception of a thin film of bacterial growth covering less than 10% of the upper screen and a thin layer of fine sediment covering the lower screen. A summary of the video logging run is in Table 2.0-1. DVD recordings of the logging runs are presented as Appendix C, included on DVD with this document.

3.0 WELL REDEVELOPMENT

On October 30, 2022, the upper and lower screened intervals were brushed to remove the thin bacterial growth and sediment observed in the initial camera survey. The brushing tool consisted of 5-in.-diameter nylon brushes attached to a cable sand-line. The brush was raised and lowered rapidly through the well screens to remove bacterial growth and sediment. After brushing the screened intervals, a sand bailer was used to remove any dislodged sediment from the well. A follow-up video log of the well was conducted on October 31, 2022, to confirm removal of buildup of material on the screens. The temporary packer was installed downhole on November 1, 2022, to separate the screens pending reinstallation of the sampling system.

After inflation of the permanent packer on January 13, 2023, the well was pumped for 33.5 hr between January 14 and 18, 2023, using the existing 4-in. Grundfos, 3-hp, submersible pump. Approximately 6650 gal. of groundwater was purged using the submersible pump during well redevelopment.

4.0 REINSTALLATION OF DUAL-SCREEN SAMPLING SYSTEM

The R-44 dual-screen sampling system, including the repaired LIC, was reinstalled between January 10 and 13, 2023. Installation activities were conducted according to N3B-GDE-ER-6011, “Groundwater Monitoring Well Dual Screen Sampling System Installation and Testing,” and N3B-SOP-ER-6003, “Pneumatic Leak Testing of Groundwater Sampling and Packer Pressurization Equipment.”

On January 12, 2023, pressure testing of the system during reinstallation detected a very small leak between the Y-block fitting and the inflation line jumper above the pump shroud. The fitting and jumper were replaced and the threads were treated with Teflon tape and Jet-Lube V-2 compound according to the packer system manufacturer’s recommendation. The fitting and jumper were pressure tested and visually examined following the repair and no leak was detected.

No other leaks (in the inflation lines, LIC, packer, APVs) were detected during testing. The submersible Grundfos pump was tested and it performed to specifications.

The 1/4-in. nylon actuation and packer tubing were replaced with new 1/4-in. stainless-steel tubing. The 1/4-in. nylon pump vent tubing was replaced with new 1/4-in. nylon tubing. Fittings were replaced as needed. The lower depth-to-water inlet was modified by adding a 1/4-in.-diameter, 12-in.-long stainless-steel screen to the stainless-steel tubing below the packer and within the PVC above the pump shroud. The upper depth-to-water inlet screen is a 6-in. section of 0.010-in. slot screen with a threaded end cap on the bottom. A new splice was made between the 12-gauge electrical cable and the pump pigtail. The brass bleeder orifice was replaced with a stainless-steel bleeder orifice.

Upon reinstallation, the upper and lower APVs, the LIC and packer were pressure tested. Pressure tests were performed after plumbing nitrogen tubes into any fitting that either actuated the APVs or pressurized the LIC, packer, and closed sides of the APVs. The LIC, packer, and closed sides of the APVs were tested at approximately 250 psi. The upper and lower APV actuation lines were pressure tested at 425 psi. Pressure tests were within the range expected to be applied to the system during operation. No leaks were identified during testing. The 7-day continuous pressure test was conducted between January 13 and 20, 2023, and the test confirmed no detectable sampling system pressure leaks.

The pump shroud was set from 908.3 to 916.4 ft bgs. The upper APV screen was set from 921.4 to 921.7 ft bgs. The LIC was set from 920.3 to 928.8 ft bgs, and the packer was set from 934.9 to 940.6 ft bgs. The lower APV screen was set from 982.0 to 982.3 bgs.

Water-level measurements for each screen are accessed via two 1-in. I.D. schedule 80 PVC transducer gauge tubes. The gauge tubes were installed to a depth of 906.7 ft bgs. The upper transducer gauge tube is fitted with a 6-in. section of 0.010-in. slot screen and bottom cap, providing upper screen water-level measurements. The lower transducer gauge tube bottom cap is fitted to stainless-steel tubing that extends to 942 ft bgs through the pump shroud, LIC, and packer, providing lower screen water-level measurements.

Table 4.0-1 provides R-44 monitoring well and sampling system component details.

Appendix D of N3B-GDE-ER-6011 “Groundwater Monitoring Well Dual-Screen Sampling System Installation and Testing” outlines the packer and access port valve pressure requirements. The pressure requirement calculations are presented in Section 4.1 and 4.2.

4.1 Minimum Packer Pressure Requirements

The formula used to determine the minimum packer inflation pressure is as follows:

$$R_{\min} = 50 + M(50, 0.2h) + \frac{d_p - d_{hswl}}{2.31} \qquad \textbf{Equation 1}$$

where R_{min} = minimum packer inflation pressure required, in psi

$M(a,b)$ = the maximum of a (50) or b (0.2h)

h = head difference above and below packer, in feet

d_p = depth to packer, in feet

d_{hswl} = depth to the higher static water level of the two zones above and below the packer (usually that of the upper zone), in feet

Using the information for the R-44 sampling system configuration of

H = 0.2 ft

d_p = 934.9 ft bgs

d_{hswl} = 885.8 ft bgs,

The minimum packer inflation pressure is 121 psi.

4.2 Maximum Packer Pressure Allowable

The formula used to estimate the maximum safe packer pressure is as follows:

$$R_{\max} = 300 + \frac{M(-27, d_p - d_{lpwl})}{2.31} \qquad \textbf{Equation 2}$$

where, R_{max} = maximum allowable packer inflation pressure, in psi

$M(a,b)$ = the maximum of a (-27) or b (d_p–d_{lpwl})

d_p = depth to packer, in feet

d_{lpwl} = depth to lower pumping water level of the two zones, in feet

Using the information for the R-44 sampling system configuration of:

d_p = 934.9 ft bgs

d_{lpwl} = 887.2 ft bgs

The maximum packer inflation pressure is calculated to be 321 psi

Applying this formula yields a maximum permissible inflation pressure of 321 psi at R-44. Baski, Inc., proof-tested the packer to 300 psi in a 5-in. I.D. pipe without apparent leakage or damage. Baski, Inc., should be contacted for information about operating at inflation pressures in excess of 300 psi.

4.3 Target and Action Packer Pressures

The target packer pressure is the pressure at which the packer operates and is set at halfway between the minimum and maximum packer pressures. The target packer pressure at R-44 is 221 psi.

The action packer pressure is the value below which the packer pressure should not be allowed to drop and is set at halfway between the minimum and target pressures. The action packer pressure at R-44 is 171 psi.

4.4 System Test

After the sampling system was installed, the packer was inflated to approximately 240 psi and remained stable, with no trend, for 7 days, indicating a successful pressure test. Table 4.4-1 presents the pressure test dates, times, and measured packer pressures.

5.0 CROSS-FLOW ESTIMATES

The volume of water that flowed from the upper screen interval to the lower screened interval was estimated using specific capacity and hydraulic head data. This estimate of cross-flow volume is needed to determine the amount of cross-flow water to be purged from the well. The cross-flow rate can be computed using the following formula:

$$Q = h \frac{c_1 c_2}{c_1 + c_2}$$ **Equation 3**

where Q = cross-flow rate, in gpm

c_1 = specific capacity of screen 1, in gpm/ft

c_2 = specific capacity of screen 2, in gpm/ft

h = head difference between screens 1 and 2

Specific capacity of screen 1 is 5.67 gpm/ft and specific capacity of screen 2 is 1.35 gpm/ft, and the head difference between screens 1 and 2 is approximately 0.2 ft (LANL 2009, 106418, Appendix C-11.0). Applying this formula yields a cross-flow rate of approximately 0.22 gpm at R-44.

When the sampling system was removed, the packer was deflated at 1:20 p.m. on October 25. The temporary packer was inflated at 3:00 p.m. on November 1 and deflated at 10:15 a.m. on January 10. After reinstallation, the permanent packer was inflated at 5:35 p.m. on January 13. Thus, cross-flow occurred for 14,938 min during system maintenance activities.

The estimated cross-flow volume is calculated by multiplying the cross-flow period duration by the cross-flow rate, yielding 3286.4 gal. Two hundred percent of the estimated cross-flow volume yields a lower screen purge volume of 6572.8 gal.

Approximately 6650 gal. of water was purged from the lower screen zone between January 14 and January 18, 2023.

6.0 PURGE VOLUME REQUIREMENTS

One casing volume (CV) of water from the upper screen section, based on a groundwater elevation of 886.6 ft bgs and a water column of 48.4 ft above the packer in the 5-in. I.D. stainless-steel casing (1.02 gal./ft) is about 49.4 gal. The 1-in. stainless-steel drop pipe from surface to the top of the upper APV screen contains approximately 37.8 gal., for a single CV plus drop pipe volume of 87.2 gal. Three CVs plus drop pipe is about 186 gal. At a pumping rate of 3.3 gpm, the time to purge three CVs plus drop pipe from the lower screen section is about 1 hr.

One CV of water from the lower screen section, based on a water column from below the packer at 940.6 ft bgs to the bottom of the well (1016.0 ft bgs) in the 5-in. I.D. casing (75.4 ft purge length), is about 76.9 gal. The 1-in. stainless-steel drop pipe from surface to the top of the lower APV screen contains approximately 40.3 gal., for a single CV plus drop pipe volume of 117.2 gal. Three CVs plus drop pipe is about 271.0 gal. At a pumping rate of 3.3 gpm, the time to purge three CVs plus drop pipe from the lower screen section is about 1.4 hr.

Table 6.0-1 lists the parameters associated with calculation of the purge volumes at each screen.

7.0 GROUNDWATER QUALITY PARAMETERS

The pumping stage of well redevelopment occurred during 12-hour day shifts on January 14, 15, 16 and 18, 2023. During the pumping stage, the groundwater turbidity, temperature, pH, dissolved oxygen (DO), oxidation-reduction potential (ORP), and specific conductance were measured using a flow-through cell connected to the well discharge pipe. The pH ranged from 7.96 to 8.09 and temperature ranged from 13.2°C to 20.6°C. DO concentrations varied from 5.50 to 6.03 mg/L. ORP values for rate of electron transfer varied from 150.4 to 365.8 mV. The pH/ORP sensor used to determine ORP values consisted of a silver/silver chloride reference electrode and platinum reference junction. Specific conductance ranged from 356.8 to 556.8 µS/cm, and turbidity values varied from 0.07 to 17.00 NTUs. Suspended solids concentration as measured by the Imhoff cone was 0 ml/L.

The final parameters at the end of well development (at 2 p.m. on January 18, 2023) were pH of 7.96, temperature of 18.5ºC, DO of 5.85 mg/L, ORP of 251.4 mV, specific conductance of 382.8 µS/cm, and turbidity of 0.08 NTU. Table 7.0-1 shows groundwater quality parameters and purge volumes measured during well development.

8.0 SUMMARY

A leaking LIC fill plug was identified during diagnostic testing of the sampling system. The LIC was removed and repaired according to the manufacturer's recommendation using Teflon tape and V-2 compound applied to threads. Following the repair, visual and pressure gauge tests of the system, including the 7-day continuous pressure test, confirmed that the system was holding pressure. Overall, the testing of the R-44 dual-screen sampling system demonstrated that the system functions properly following repairs.

The 2022–2023 well maintenance event at R-44 was successful and the well was returned to service on March 09, 2023, as part of the Interim Facility-Wide Groundwater Monitoring Program at Los Alamos National Laboratory. The as-built schematic of the sampling system (see Figure 1.0-1) should be used as a reference for future groundwater monitoring activities at R-44.

9.0 REFERENCE

LANL (Los Alamos National Laboratory), May 2009. "Completion Report for Regional Aquifer Well R-44," Los Alamos National Laboratory document LA-UR-09-3066, Los Alamos, New Mexico. (LANL 2009, 106418)

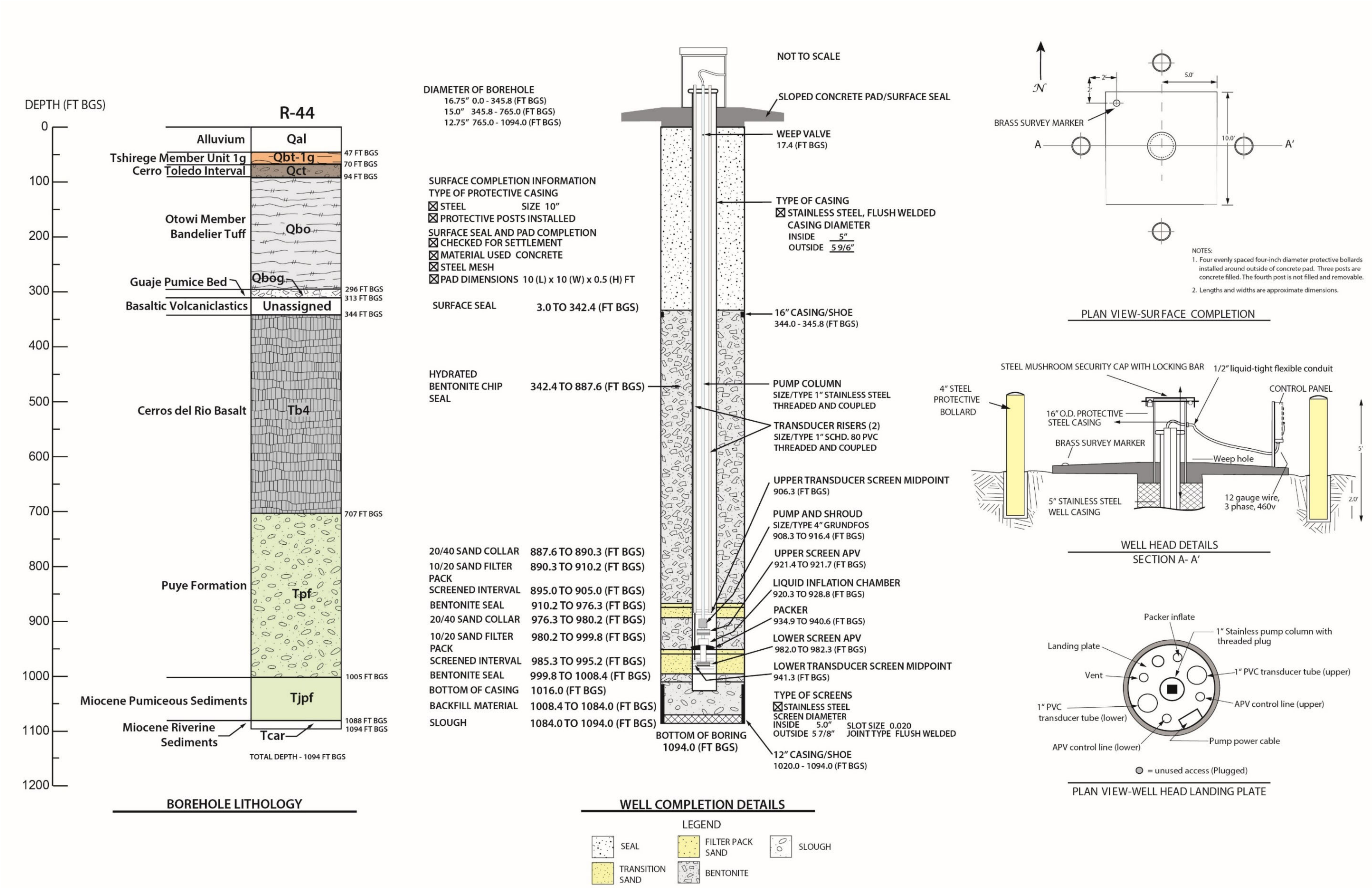

Figure 1.0-1 Monitoring well R-44 as-built diagram with borehole lithology and technical well completion details

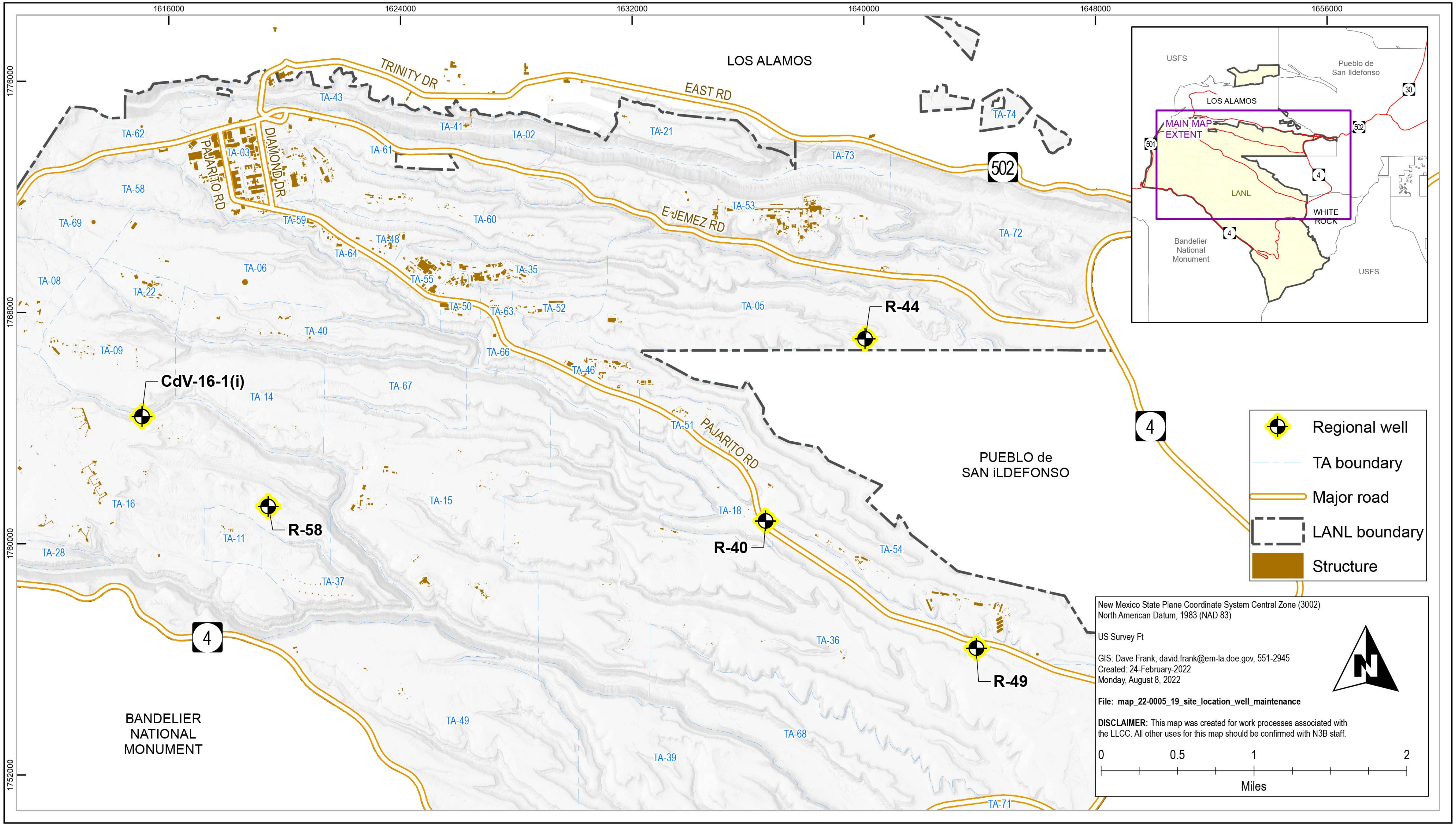

Figure 1.0-2 Location Map for R-44 and other wells identified in field implementation plan

Table 2.0-1
R-44 Video Logging Runs

Date	Logging Interval	Description
10/30/2022	0 to 1015 ft bgs	Video log run in the completed well casing
10/31/2022	0 to 1000 ft bgs	Video log run in the completed well casing

Table 4.0-1
R-44 Well and Sampling System Details

	Upper Screen (ft bgs)	Lower Screen (ft bgs)	Sump (ft bgs)*	Upper Gauge Tube Screen (ft bgs)	Pump Shroud (ft bgs)	Upper APV Screen (ft bgs)	LIC (ft bgs)	Packer (ft bgs)	Lower Gauge Tube Intake (ft bgs)	Lower APV Screen (ft bgs)
Top	895.0	985.3	934.9	906.0	908.3	921.4	920.3	934.9	940.85	982.0
Bottom	905.0	995.2	1016.0	906.5	916.4	921.7	928.8	940.6	941.85	982.3

*The sump at 934.9 ft bgs is the top of the packer. The sump at 1016.0 ft bgs is the bottom of the well casing.

Table 4.4-1
R-44 Packer Pressure Monitoring

Date	Time	Packer Pressure (psi)
1/14/2023	1740	240
1/15/2023	1730	246
1/16/2023	1800	240
1/17/2023	No reading due to Los Alamos National Laboratory weather shutdown.	
1/18/2023	1030	239
1/19/2023	1219	240
1/20/2023	1340	240

Table 6.0-1
R-44 Purge Volume Requirements

Screen	Top of Purge Zone (ft bgs)	Bottom of Purge Zone (ft bgs)	Length of Purge Zone (ft)	5-in. SS* Casing Volume (gal./ft)	1 CV (gal.)	1-in. SS Drop Pipe Volume (gal.)	1 CV + Drop Pipe (gal.)	3 CV + Drop Pipe (gal.)	Purge Rate (gpm)	Purge Time (min)	Purge Time (hr)
Upper	886.60	935.00	48.40	1.020	49.40	37.80	91.10	186.00	3.30	56.40	0.93
Lower	940.60	1016.00	75.40	1.020	76.90	40.30	117.20	271.00	3.30	82.20	1.40

* SS = Stainless-steel.

Table 7.0-1
R-44 Purge Volumes and Groundwater Quality Parameters During Well Development

Date	pH	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
1/14/2023	7.78	13.2	5.91	187.30	554.00	17.00	17.50	17.50
	7.76	14.4	6.03	183.80	549.00	11.80	15.50	33.00
	7.79	15.4	5.74	183.50	553.40	4.93	17.00	50.00
	7.78	16.8	5.65	184.20	553.80	1.53	17.00	67.00
	7.83	18.6	5.56	188.00	555.20	1.36	17.00	84.00
	7.88	18.6	5.56	193.80	554.20	0.71	16.50	100.50
	7.91	18.8	5.55	198.90	556.40	0.49	16.50	117.00
	7.91	18.9	5.60	202.90	556.80	0.75	16.50	133.50
	7.92	18.9	5.62	208.60	555.60	0.32	16.50	150.00
	7.91	18.9	5.62	213.80	555.00	0.26	16.50	166.50
	7.91	19.3	5.60	217.00	555.90	0.26	16.50	183.00
	7.92	19.2	5.58	220.30	556.10	0.21	16.50	199.50
	7.92	19.4	5.58	224.10	556.30	0.18	16.50	216.00
	7.89	19.2	5.69	243.80	554.80	0.22	210.00	426.00
	7.95	20.1	5.76	242.90	553.30	0.18	210.00	636.00
	7.92	20.6	5.61	257.10	549.40	0.13	210.00	846.00
	7.93	20.5	5.63	274.30	544.20	0.22	210.00	1056.00
	7.89	20.4	5.65	298.50	540.90	0.20	210.00	1266.00
	7.91	20.5	5.61	314.00	535.90	0.17	210.00	1476.00
	7.91	20.3	5.62	329.70	530.80	0.10	210.00	1686.00
	7.57	14.3	5.50	196.30	547.80	0.73	198.00	1884.00
1/15/2023	7.73	16.8	5.78	150.40	515.30	0.53	192.00	2076.00
	7.80	20.0	5.68	182.80	511.50	0.17	186.00	2262.00
	7.79	20.2	5.65	191.10	504.80	0.21	192.00	2454.00
	7.76	19.9	5.69	193.40	496.90	0.28	186.00	2640.00
	7.72	19.3	5.70	195.80	487.50	0.36	198.00	2838.00
	7.77	19.7	5.69	190.20	483.40	0.26	210.00	3048.00
	7.81	19.6	5.73	184.90	472.20	0.33	198.00	3246.00
	7.75	19.2	5.77	191.80	465.30	0.25	198.00	3444.00
	7.81	19.5	5.77	191.70	451.50	0.71	198.00	3642.00
	7.77	19.2	5.75	196.90	449.70	0.28	198.00	3840.00
	7.86	17.8	5.92	243.50	441.70	0.23	192.00	4032.00

Table 7.0-1 (continued)

Date	pH	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
1/16/2023	7.85	18.60	5.81	236.60	435.30	0.11	192.00	4224.00
	7.85	18.80	5.82	296.50	431.20	0.13	192.00	4416.00
	7.85	19.00	5.83	317.20	426.00	0.11	186.00	4602.00
	7.90	19.80	5.80	331.40	417.80	0.11	192.00	4794.00
	7.86	20.00	5.77	343.10	375.30	0.14	192.00	4986.00
	7.80	19.60	5.78	352.30	366.70	0.11	192.00	5178.00
	7.80	19.20	5.85	357.60	361.50	0.19	192.00	5370.00
	7.79	19.10	5.85	362.30	356.80	0.10	198.00	5568.00
	7.77	18.00	5.91	365.80	397.90	0.12	204.00	5772.00
	7.81	18.50	5.92	365.10	394.00	0.09	204.00	5976.00
	8.09	18.60	5.92	174.10	389.10	0.23	192.00	6168.00
1/18/2023	8.02	18.50	5.87	202.10	386.50	0.11	99.00	6267.00
	7.99	19.50	5.80	221.20	385.90	0.11	99.00	6366.00
	8.03	19.10	5.81	151.20	386.10	0.10	96.00	6462.00
	7.98	19.30	5.80	223.10	385.20	0.07	96.00	6558.00
	7.96	18.50	5.85	251.40	382.80	0.08	96.00	6654.00

Appendix A

Field Implementation Plan for Repair of
Wells R-40, R-44, R-49, R-58 and CdV-16-1(i)

Field Implementation Plan for Repair of Wells R-40, R-44, R-49, R-58 and CdV-16-1(i)

October 2022

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October 2022

Field Implementation Plan for Repair of Wells R-40, R-44, R-49, R-58 and CdV-16-1(i)

N3B Los Alamos

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ACRONYM LIST

DOE	U.S. Department of Energy
EPA	U.S. Environmental Protection Agency
ER	Environmental Remediation
ES&H	Environment, Safety and Health
FIP	field implementation plan
FTL	field team leader
IDW	Investigation derived waste
IWCP	integrated work control process
LANL	Los Alamos National Laboratory
LIC	liquid inflation chamber
NMED	New Mexico Environmental Department
N3B	Newport News Nuclear BWXT-Los Alamos, LLC
OM	operations manager
PLY	Pajarito Laydown Yard
POD	plan of the day
PPRR	Project Plan and Readiness Review
PVC	polyvinyl chloride
QA	quality assurance
RCT	radiological control technician
RLM	responsible line manager
SMO	sample management office
SOM	shift operations manager
SOP	standard operating procedure
SSEH&SP	site-specific environmental health and safety plan
SWPPP	Stormwater Pollution Prevention Plan
STR	subcontract technical representative
T&E	threatened and endangered
T2S	Tech2Solutions
VFD	Variable Frequency Drive
WCSF	Waste Characterization Strategy Form

1.0 INTRODUCTION

1.1 Background

Newport News Nuclear BWXT – Los Alamos, LLC (N3B) via Tech2Solutions (T2S) has contracted with Layne Christensen Company (Layne) to perform well repair activities of existing monitoring wells at Los Alamos National Laboratory (LANL), Los Alamos County, New Mexico (Figure 1). All work will be performed in accordance with the following:

- The IWCP for Well Repair of R-40, R-44, R-49, R-58 and CdV-16-1(i)
- The statement of work and technical specifications for Well Repair (Statement of Work)

This Field Implementation Plan (FIP) provides technical guidance for field activities associated with the Los Alamos National Laboratory (LANL) well repair project at monitoring wells R-40, R-44, R-49, R-58 and CdV-16-1(i), located in Los Alamos, New Mexico, as shown in Figure 1, Well Location Map.

The activities associated with the project include mobilization/demobilization of equipment, decontamination of equipment/tools, pressure leak testing, removal/assembly of plumbing between wellhead and manifold, pump system removal, packer removal/installation, swabbing/bailing, aquifer testing, collection of water quality parameters and water samples, video logging, reinstallation and testing of pump system.

As-built well diagrams and technical notes for the referenced wells are presented in Figures 2 through 10.

Project staff, health and safety are also discussed in this document.

1.2 Objectives

This FIP outlines the objectives for evaluation of nitrogen leaks and rehabilitation of Baski sampling systems in wells R-40, R-44 and R-49 and removal and replacement of pumping systems in wells R-58 and CdV-16-1(i) and well redevelopment at each well.

2.0 ORGANIZATIONAL STRUCTURE

This project is a joint effort of Newport News Nuclear BWXT (N3B), its subcontractor Tech2 Solutions and second-tier subcontractor Layne Christensen Company (Layne). An organizational chart is presented in Table 1.

2.1 N3B Project Management Team

The management team includes the Water Program Director, Program Manager, Project Manager, Environmental, Safety and Health (ES&H) Manager, Quality Assurance (QA) Manager, Procurement Manager, and ancillary staff to support and assist in all areas of the project. The management team will provide project management, prepare reports and deliverables, provide field support and oversight of repair tasks, and manage waste streams and sample analyses.

The ES&H Manager will provide ES&H assistance in accordance with Exhibit F of the request for proposal and the integrated work control process documents (IWCPs) and site-specific environmental, health and safety plan (SSEH&SP). Water Program field team leaders (FTLs) are trained as ES&H and QA representatives to provide ES&H and QA field oversight.

2.2 N3B Field Team

During the repair activities, there will be one full-time, on-site, Field Team Lead (FTL), who will act as site manager, ES&H representative, and QA representative. The FTL will maintain field notes detailing daily site activities including standby and documenting sample system installation. The FTL will also be responsible for, but not limited to, conducting daily safety meetings, compiling and submitting daily field reports, review and approval of Layne daily field reports, and collecting/documenting groundwater samples. A list of relevant standard operating procedures (SOPs) for the field project is presented in Table 2. The FTL will serve as a point of contact in conjunction with other field staff. Other on-site support personnel may be added to the field team as needed.

2.3 Well Repair Subcontractor

The Layne field team shall include a qualified pump hoist operator and additional personnel needed to safely and efficiently carry out planned activities. Other qualified staff or subcontracted service providers may be added as necessary to ensure all project requirements are met.

Layne personnel must be U.S. citizens, badged and trained before being approved for field work. Training has been outlined in a training matrix and supplied to Layne. Work crews must be of sufficient size to safely and effectively conduct the planned work, or the FTL on duty will pause/stop work until adequate manpower is present.

As the well repair subcontractor, Layne will support N3B with site safety and quality assurance at all times. All field staff are empowered to pause/stop work in accordance with N3B procedures.

Layne will ensure that equipment is appropriate for the goals of the field project and in proper working order, and that daily logs are maintained. In addition, Layne will support Water Program staff in video logging of the wells, as specified below.

3.0 FIELD ACTIVITIES

Field activities typically will include the following:

- Mobilization/demobilization
- pressure leak testing of packer inflation system
- removal/assembly of plumbing between wellhead and manifold
- pump system or Baski packer removal and reinstallation of new equipment
- video logging
- well redevelopment activities
- reinstallation and testing of the pumping system

The table below indicates the general tasks to be completed at each well site:

Well Number	Repair Tasks
R-40	Evaluate Baski sampling system and replace Baski packer, as needed
R-44	Evaluate Baski sampling system and replace Baski packer, as needed
R-49	Evaluate Baski sampling system and replace Baski packer, as needed
R-58	Replace sampling system pump
CdV-16-1(i)	Replace sampling system pump

The Exhibit A, statement of work, for well repair tasks will be used to guide field operations and ensure all objectives are met.

3.1 Readiness

N3B will coordinate readiness activities.

N3B will coordinate or be responsible for the following:

- Quality Management – Provide review of Layne's Quality Program for compliance and train field personnel to T2S 512.00.01, Rev. 0 "Project Quality Implementation Plan" before field operations.
- ES&H – Coordinate with Layne for their assistance in preparing the IWCP and in reviewing the SSEH&SP. Review training records for health and safety needs.
- Waste Characterization Strategy Form (WCSF) – Prepare plan, acquire required containers, and provide waste sampling criteria.
- Training Requirements – Define requirements and review all field staff records for completeness.
- Stormwater Pollution Prevention Plan (SWPPP) – Prepare or review SWPPP, if applicable, and implement engineered features to minimize impacts from storm water at drill site.
- Project Plan & Readiness Review (PPRR) – Compile all relevant documentation and determine resolutions for issues associated with the National Environmental Policy Act cultural resources and threatened and endangered (T&E) species.
- Spark and Flame Permit – Obtain and verify permit before all spark and flame producing operations.
- Training and Badges – Provide training and badges for all proposed field staff.
- Location of Potable Water Source – Define source, see 3.4 Mobilization
- Requests for Plan of the Day (POD) – Coordinate with Environmental Remediation (ER Ops) Operations staff regarding schedule of activities.
- Access Keys and Radios – Obtain keys and radios for field team.
- Inspections – Define items/tasks to be inspected and coordinate schedule for qualified inspections (e.g., rig inspection, electrical systems, sampling and pumping system assembly).
- Radiological Services – Coordinate schedule with radiological control technicians (RCTs) for the documentation and screening of incoming equipment and at final demobilization of equipment.
- Water Hauling – Provide potable water from J-stand, to be transported to sites by Layne for decontamination, as needed. Contaminated water to be stored temporarily in poly tanks at the site for WCSF sampling, waste characterization and disposition.

Layne will coordinate, or cooperate with the following:

- Assure that all personnel are U.S. citizens and are trained to applicable corporate ES&H and QA standards
- Assist N3B staff with IWCP and SSEH&SP preparation and review, and make all personnel available for LANL/N3B-required training and badging

- Provide hoist rig maintenance records and conduct a robust equipment inspection before delivery to LANL
- Assist N3B in inspection of rig and equipment at the Pajarito Laydown Yard, and provide decontamination of rig and equipment, before mobilization to well sites
- Assist N3B in inspection of rig and equipment at rig up inspection at each well site

3.2 Equipment

Well repair tasks will be facilitated with a pump hoist rig provided by Layne, with suitable auxiliary equipment including, but not limited to, air compressors, water truck/rig tender, forklifts, and manlift, as needed. Light plants will be provided by Layne, in case of work during night shifts, and be sufficient for adequate well pad lighting as verified by N3B light surveys.

This pump hoist will perform well redevelopment, installation of temporary pump systems for aquifer testing, and installation of the dedicated sampling system.

Material approvals and receipt inspections will be conducted by both Layne and N3B for all items, including initial inspection of rig and equipment when mobilized to LANL, any new wire rope and other hoist rigging delivered to site after mobilization.

Layne will be responsible for delivery of all fuel necessary for equipment operation to the well sites for R-40, R-44 and R-49 and to the Pajarito Laydown Yard (PLY). Fuel deliveries to wells R-58 and CdV-16-1(i), both of which are located in the Weapons Facility Operations (WFO) at Technical Area TA-16, will be coordinated with Triad. The placement of an aboveground storage tank on-site is allowed, with placement on secondary containment. No more than 1320 gals of fuel will be allowed at well sites R-40, R-44 and R-49 site at any time, excluding vehicle fuel tanks, to avoid application of spill prevention control and countermeasure (SPCC) rules.

3.3 Waste Collection

Investigation-derived waste (IDW) will be managed in accordance with standard operating procedure (SOP) N3B-EP-SOP-10021, “Characterization and Management of Environmental Program Waste.” This SOP incorporates the requirements of applicable U.S. Environmental Protection Agency (EPA) and New Mexico Environmental Department (NMED) regulations, Department of Energy (DOE) orders, and N3B requirements. The primary waste streams will include development water, purge water generated during redevelopment, decontamination water, and contact waste. Details are located in the WCSFs for the individual wells.

3.4 Mobilization

Equipment and supplies for the completion of the project will be staged at each work site in an organized and secure manner. Surplus and/or inactive equipment and supplies may be stored at the PLY located at the northwest corner of Pajarito Road and New Mexico State Road 4. Access to the laydown yard is through a locked gate and is limited to the hours of 7 a.m. to 7 p.m. unless prior authorization is granted.

Mobilization to each site will consist of transporting and setting up equipment at the well site and will include the following:

- Mobilize pump hoist rig, trailers, support vehicles, materials, and tools to the well site.
- Set up pump hoist rig, trailers, support vehicles and tools at the location.
- Complete pump hoist rig up inspection.

- Review scope of work and project-specific health and safety issues with crew.
- Complete all required training for all personnel.
- Obtain Environmental Remediation (ER) Responsible Line Manager (RLM)/ Operations Manager's (OM) authorization through the Plan-of the-Day (POD), including rig inspection and Integrated Work Control Process form (IWCP) review.

Site access routes have been established for all sites. The water source for the project will be the J-stand located on Eniwetok Drive, adjacent to building number 60-0287.

Since no soil disturbance exceeding one acre per site is expected, no SWPPP is required. In the event pad repairs or snow removal are required during repair operations, Layne will support N3B ER Crafts crews in these operations. If snow removal is necessary, N3B will maintain access to the well pad, and Layne will be responsible for clearing snow from the pad. Layne will ensure that work areas will always be kept free of ice to maintain safe working conditions.

Decontamination of any pumping system components that will be placed downhole during well repair and redevelopment (including packer, drop pipe, APVs, pump, pump shroud, liquid inflation chamber (LIC), etc.) will be hot water/steam pressure rinsed, washed with non-phosphatic Alconox® or Liquinox® detergent, hot water/pressure rinsed again, then wrapped in plastic after air drying prior to the start of repair and redevelopment activities. Decontamination water will be containerized in 55-gal drums or poly-tanks, properly labeled, and stored on-site for characterization and disposal. For water quality testing, it is anticipated that samples would be collected directly from a spigot mounted at the wellhead.

Decontamination of sample tools will be performed with a wire brush followed by spraying with Fantastik® and wiping clean with paper towels. If bailers are used for collecting groundwater samples, they will be washed with Liquinox® detergent and potable water and rinsed with deionized water before sample collection. The deionized water would be provided by N3B.

3.5 Planned Repair Tasks at Well Sites

Wells R-40, R-44 and R-49 - Baski Sampling System Evaluation and Packer Replacement

At each of these wells, all of which are 5-inch inside diameter (ID) dual-screen monitoring wells with Baski sampling and pumping systems in place, Layne Christensen will perform pressurized leak tests with nitrogen and troubleshoot pneumatic fittings for inflation lines for the inflatable packer, and upper and lower access port valves at the wellhead.

Upon confirmation that the apparent pressure leak is downhole, Layne will begin removing the sampling system from the well, performing pressure testing of all fittings at each stage. If it is determined that the existing packer is the source of the leak, a new packer will be prepared for installation in the well. The packer is provided by N3B.

Upon removal of the complete sampling system, Layne will provide access and assist T2S crew for video logging.

Layne will reinstall the sampling system, consisting of the existing pump, pump shroud, upper and lower access port valves (APVs), liquid inflation chamber (LIC), new packer, 1-inch diameter pump column pipe and two 1-inch PVC gauge tubes. Existing PVC gauge tubes and 1-inch-diameter stainless steel pump column will be evaluated and reinstalled or replaced, depending on condition .Layne, under FTL oversight, will assist with inspection of the existing drop pipe for wear, erosion, thread damage, etc. Damaged pipe will be replaced as-needed prior to re-installation

Replacement PVC gauge tubes and pump column pipe will be provided by N3B.

The existing pump power cable will be evaluated by Layne, under FTL oversight, and replaced, depending on condition. N3B will provide the replacement cable. Electrical terminations/splices to the pump motor will be made by N3B craft electricians or by Subcontractor's N3B-approved licensed electricians offsite. ~~.~~Electrical terminations in the electrical panel will be made by N3B craft electricians.

With reinstallation of the system, Layne will install new stainless steel inflation/actuation lines and new nylon tubing line for pump shroud air vent, all secured with new stainless steel banding and buckles, and new stainless steel screens for lower zone gauge tube modification. The inflation/action lines, tubing, banding, buckles and stainless steel screens will be provided by N3B.

Layne will conduct pressure leak tests at all inflation line fittings as re-installation of the system proceeds, including at surface prior to start of installation.

Once the sampling system is installed, 200% of the calculated cross flow volume may be pumped from the affected screen. The cross flow times include from the time the packer was deflated after the last aquifer test was completed until the temporary packer is installed, and from the time the temporary packer is deflated until the permanent packer is inflated.

All waste water from deconning, purging, bailing and surging during repair and redevelopment activities must be collected in poly-tanks stored at the sites.

Well R-58 – Pump Replacement

At well R-58, a 5-inch ID monitoring well with a 4-inch pumping system in place, Layne will remove the existing pumping system and assist with video logging of well by T2S. Expect potential separation of the pump from the motor, broken shaft, etc.

Layne will then perform brushing of screen interval followed by surging and will bail the well until visible clarity of water improves. If requested, Layne will assist in collection of water samples during the bailing period. Layne will then redevelop the screen interval with jetting as directed by T2S.

Layne will then reinstall the sampling system with new environmentally retrofitted 5 HP pump and motor, including shroud and two 1-inch PVC gauge tubes. Existing PVC gauge tubes and 1-inch-diameter stainless steel pump column will be evaluated and reinstalled or replaced, depending on condition. Layne, under FTL oversight, will assist with inspection of the existing drop pipe for wear, erosion, thread damage, etc. Damaged pipe will be replaced as-needed prior to re-installation

The existing pump power cable will be evaluated by Layne, under oversight of the FTL, and replaced, depending on condition. N3B will provide the replacement cable. Electrical terminations/splices to the pump motor will be made by N3B craft electricians or by Subcontractor's N3B-approved licensed electricians offsite.~~.~~ Electrical terminations in the electrical panel will be made by N3B craft electricians.

Layne will then perform functional testing of the pump. The pump, pump motor, shroud and replacement PVC gauge tubes and pump column pipe will be provided by N3B.

All waste water from deconning, purging, bailing and surging during repair and redevelopment activities must be collected in poly-tanks stored at the site.

Well CdV-16-1(i) – Pump Replacement

At well CdV-16-1(i), a 4.5-inch ID monitoring well with a 4-inch pumping system in place, Layne will remove the existing pumping system and assist with video logging of well by T2S. Foot valve is holding so the pull will be wet. Take precautions based on ambient temperature to protect crew and work area (footing, collection of water as required).

Layne will then perform brushing of screen interval followed by surging and will bail the well until visible clarity of water improves. If requested, Layne will assist in collection of water samples during the bailing period. Layne will then redevelop the screen interval with jetting as directed by T2S.

Layne will then reinstall the sampling system with new environmentally retrofitted 5 HP pump and motor, including shroud and two 1-inch PVC gauge tubes. Existing PVC gauge tubes and 1-inch-diameter stainless steel pump column will be evaluated and reinstalled or replaced, depending on condition. Layne, under FTL oversight, will assist with inspection of the existing drop pipe for wear, erosion, thread damage, etc. Damaged pipe will be replaced as-needed prior to re-installation.

The existing pump power cable will be evaluated by Layne, under FTL oversight, and replaced, depending on condition. N3B will provide the replacement cable. Electrical terminations/splices to the pump motor will be made by N3B craft electricians or by Subcontractor's N3B-approved licensed electricians offsite.~~.~~ Electrical terminations in the electrical panel will be made by N3B craft electricians.

Layne will then perform functional testing of the pump. The pump, pump motor, shroud and replacement PVC gauge tubes and pump column pipe will be provided by N3B.

All waste water from deconning, purging, bailing and surging during repair and redevelopment activities must be collected in poly-tanks stored at the site.

3.6 Demobilization

Demobilization activities will include:

- Loading and removal of the equipment.
- Removal of the pump hoist rig and support vehicles from the site.
- Staging and securing of IDW for future disposition.
- Removal of municipal waste (e.g. materials packaging).
- Final site cleanup of all materials used during well repair activities.

The N3B subcontract technical representative (STR) and shift operations manager (SOM) will inspect the sites prior to final demobilization of the drill crew. Final demobilization of the drill crew will not be permitted until the condition of the sites are acceptable to the STR and SOM.

4.0 REPORTING

Updated as-built diagram and technical notes will be prepared within 30 calendar days of project completion. Technical notes will include dates and descriptions of project activities.

Table 1
Key Team Personnel Roles and Responsibilities

Name	Role	Responsibilities
Ryan Flynn	Water Program Director	Responsible for the successful execution of the project
Amanda White	Water Program Deputy Director	Responsible for the successful execution of the project in support of the Director
Sherry Gaddy	Drilling Program Manager (PgM)	Leadership for overall drilling and well repair program
Phil Walkup	Project Manager (PM)	Responsible for monitoring and documenting the subcontractor’s day-to-day performance, providing day-to-day oversight, and assuring work is performed in a safe manner. Project and field management, N3B interaction, subcontractor coordination, IWCP and ES&H compliance
Thomas Klepfer	Back-up Project Manager (PM)	Responsible as above as needed
Jeffrey Richeson	Subcontract Technical Representative (STR)	Responsible to the Project Manager for monitoring and documenting the subcontractor’s day-to-day performance, communications, procurement support, providing day-to-day oversight, IWCP and ES&H compliance
Christina Rampley	N3B/T2S Procurement Manager	Responsible for solicitation, negotiation, award, and administration of subcontracts and has overall commercial responsibility for subcontracts
Kenneth Hoffman	ES&H Oversight	Primary contact for ES&H oversight, ESH Professional
Al Medina	Quality Control Manager	Primary contact for N3B QA oversight
Ken Wright Karen Warren Chris Harper Isaiah Sedillo Alicia Lopez	FTL/PIC	Field management, subcontractor coordination, IWCP and ES&H compliance, ESH & QA site Representative
Adam Zimmerman	Waste Coordinator	Lead for waste generation and management oversight
Charles Smith	Layne Drilling Manager	Project and field management, N3B interaction, budget, resource commitments, subcontractor coordination, IWCP and ES&H compliance
Alex Gustafson	Layne Project Manager	Project and field management, budget and resource commitments, subcontractor coordination and ES&H compliance
Joshua Walsh Jody Woods	Layne Field Supervisors	Project and field management, N3B interaction, subcontractor coordination, IWCP and ES&H compliance
Hunter Clement	Layne Safety Specialist	Responsible for Layne corporate ES&H programs, site visits and 24/7 on-call oversight
Steve Maze	N3B Operations Manager	Facility Operations and Security Management/Coordination. Authorizes and approves project work release
Ralph Rupp	N3B Shift Operations Manager (SOM)	Responsible for authorization and coordination of field operations

Table 2
Project-Specific Procedures, Standing Orders, and SOPs

Procedure #	Title
N3B-AP-ER-1002	Environmental Remediation (ER) Field Work Requirements
N3B-P101-1	Ergonomics
N3B-P101-4	Forklifts and Powered Industrial Trucks
N3B-P101-6	Personal Protection Equipment
N3B-P101-7	Vehicle and Pedestrian Safety
N3B-P101-13	Electrical Safety Program
N3B-P101-18	Procedure for Pause/Stop Work
N3B-P101-26	Welding, Cutting, and Other Spark- or Flame-Producing Operations
N3B-P101-34	Pressure Safety
N3B-P330-9	Suspect/Counterfeit Items
N3B-SO-ER-0006	Access Restrictions in Canada del Buey
N3B-SO-ER-0024	ER Protocols During Migratory Bird Season
N3B-SO-ER-0026	ER Requirements for Opening New Empty Metal Drums
N3B-SO-ER-0032	Event or Injury Reporting Requirements for Pre-Job Briefing and Tailgate Meeting Forms
N3B-SOP-ER-2002	Field Decontamination of Equipment
N3B-SOP-ER-3001	Manual Groundwater Level Measurements
N3B-SOP-ER-3003	Groundwater Sampling
N3B-SOP-ER-6001	Pressure Transducer Installation, Removal and Maintenance
N3B-SOP-ER-6002	Well Development
N3B-SOP-ER-6003	Pneumatic Leak Testing of Packer - GW Water Sampling Equip
N3B-SOP-ER-6004	Borehole Camera and Geophysical Logging System Use
N3B-SOP-ER-6007	Packer Pressure Monitoring and Maintenance
N3B-GDE-ER-6011	GW Well Double Screen Sampling System - Install-Test
N3B-SOP-SDM-1100	Sample Containers, Preservation, and Field Quality Control
N3B-SOP-SDM-1101	Sample Control and Field Documentation
N3B-SOP-SDM-1102	Sample Receiving and Shipping by the N3B Sample Management Office
UI-PROC-64-00-125-R4	Fire Hydrant Operation and Non-emergency Use

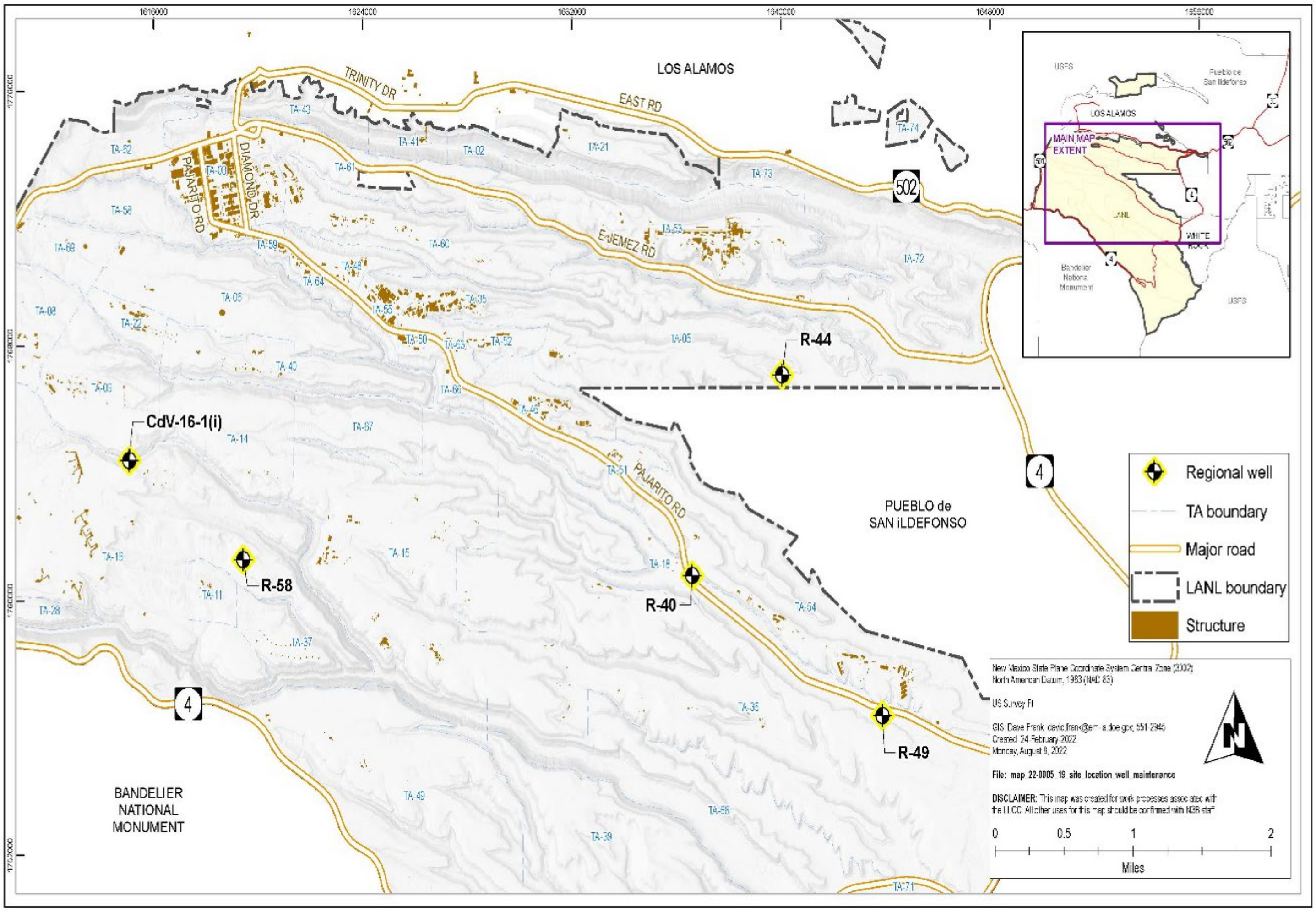


Figure 1 - Well Location Map

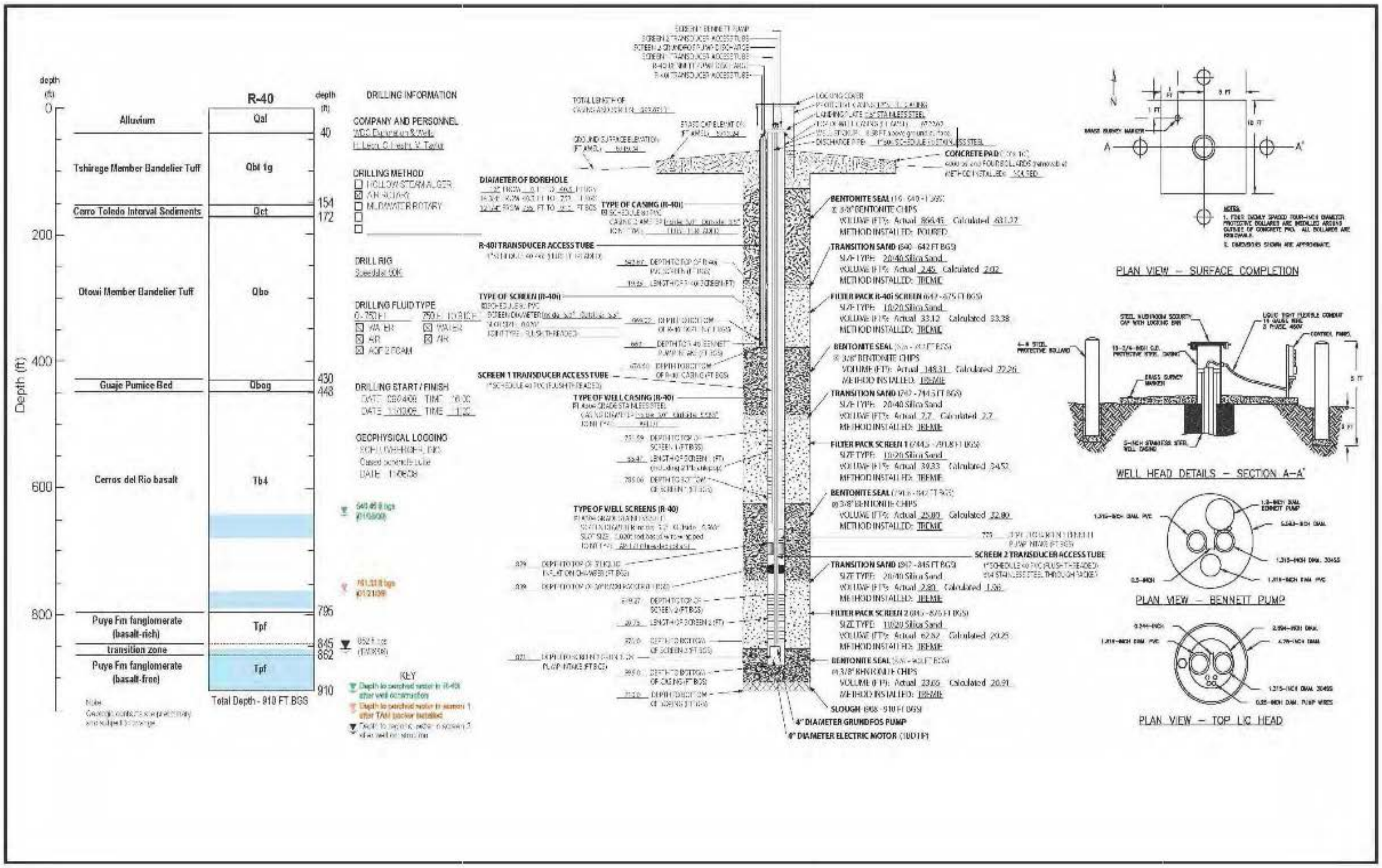


Figure 2 - R-40 As-Built Completion Schematic

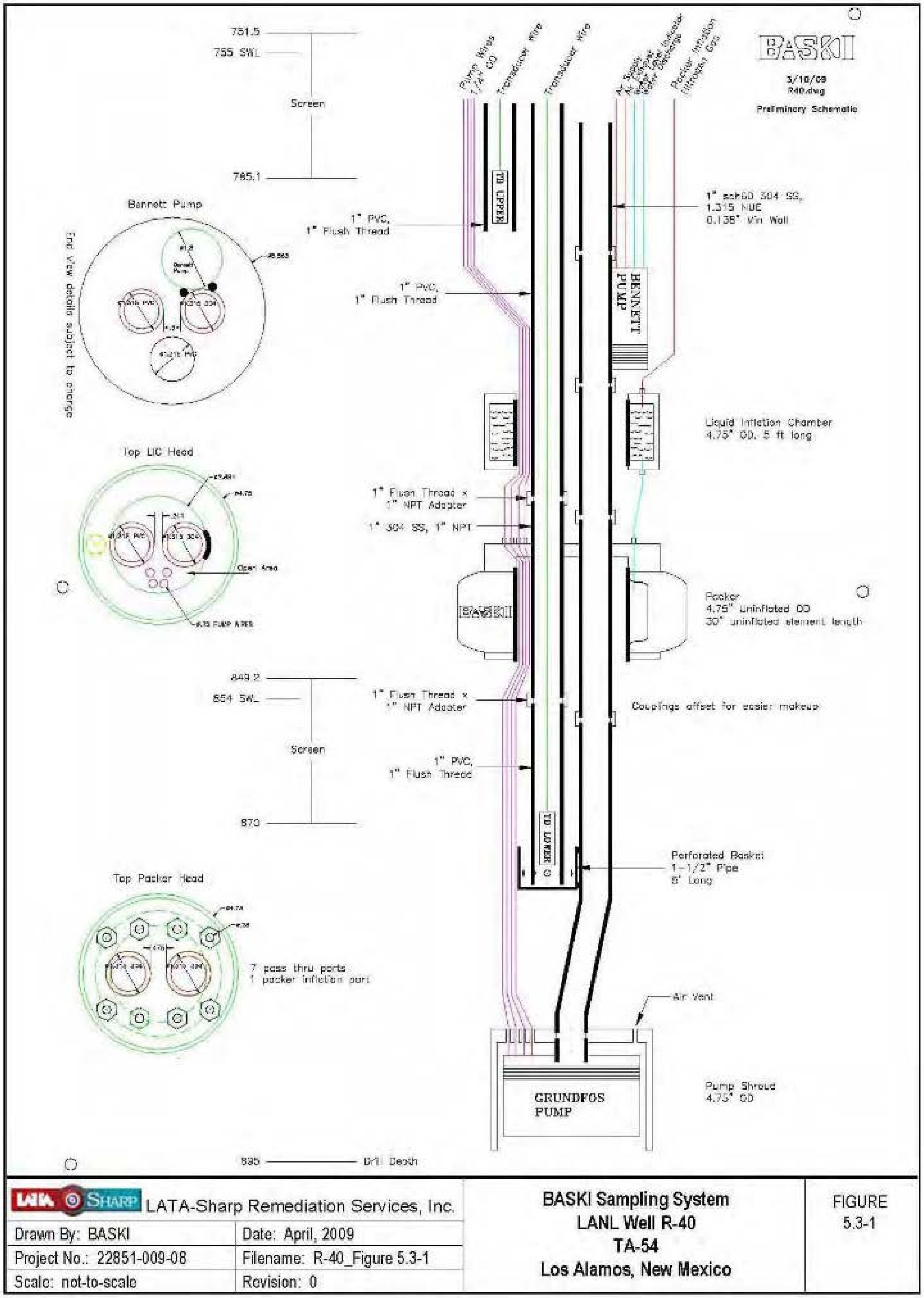


Figure 3 - R-40 Baski Sampling System

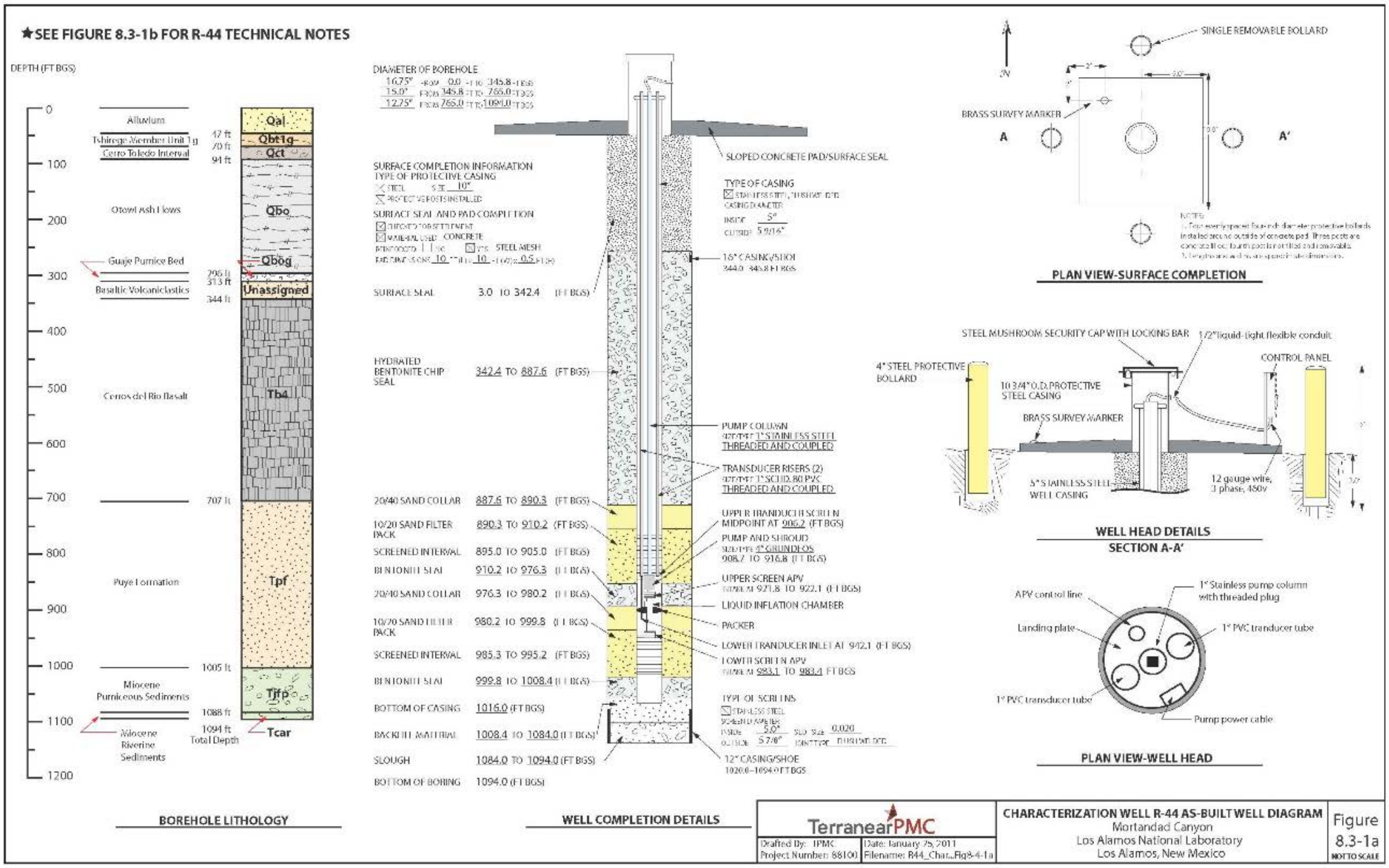


Figure 4 - R-44 As-Built Well Diagram

R-44 TECHNICAL NOTES:[1]

SURVEY INFORMATION[2]

Brass Marker

Northing:	1767109.85 ft
Easting:	1640061.34 ft
Elevation:	6714.91 ft AMSL

Well Casing (top of stainless steel)

Northing:	1767104.36 ft
Easting:	1640063.49 ft
Elevation:	6717.56 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL: natural gamma ray, induction, video
Schlumberger: natural gamma ray, elemental capture (ECS), compensated neutron (CNTG), litho-density (TLD)

DRILLING INFORMATION

Drilling Company
Boart Longyear

Drill Rig
Foremost DR-24HD

Drilling Methods
Dual Rotary
Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids
Air, potable water, AQF-2 Foam

MILESTONE DATES

Drilling

Start:	11/10/2008
Finished:	12/08/2008

Well Completion

Start:	12/13/2008
Finished:	01/15/2009

Well Development

Start:	01/15/2009
Finished:	01/20/2009

WELL DEVELOPMENT

Development Methods
Performed swabbing, bailing, and pumping
Total Volume Purged: 16005 gallons (both screens)

Parameter Measurments (Final, upper screen/lower screen)

pH:	8.22/8.19
Temperature:	18.48/18.78°C
Specific Conductance:	142/193 µS/cm
Turbidity:	0.0/0.0 NTU

AQUIFER TESTING

Step-Tests and Constant Rate Pumping Tests

Upper Screen

Water Produced:	38223 gallons
Average Flow Rate:	24.1 gpm
Performed on:	02/14–17/2009

Lower Screen

Water Produced:	38701 gallons
Average Flow Rate:	23.9 gpm
Performed on:	02/19–22/2009

DEDICATED SAMPLING SYSTEM

Pump
Type: Grundfos
Model: 5S30-820CBM
5 U.S. gpm, APVs (Acccess Port Valves) midpoints at 921.9 (upper) and 983.3 (lower) ft bgs
Environmental Retrofit

Motor
Type: Franklin Electric
Model: 2343265202
3hp, 3-phase

Pump Column
1-in. threaded/coupled stainless steel tubing

Transducer Tubes
2 × 1-in. flush threaded schd. 80 PVC tubing upper 0.01-in. slot × 0.5-ft screen at 906.2 ft bgs (midpoint), lower flexible tube from transducer set at 942.1 ft bgs

Transducers
Model: Level TROLL 500
30 psig range (vented)
S/Ns: 148101, 148136

NOTES:
1) Additional information available in "Final Completion Report, Characterization Well R44 and R45, Los Alamos National Laboratory, Los Alamos, New Mexico, TBD 2009".
2) Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83); Elevation expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

TerranearPMC		R-44 TECHNICAL NOTES Mortandad Canyon Los Alamos National Laboratory Los Alamos, New Mexico	Figure 8.3-1b NOT TO SCALE
Drafted By: TPMC Project Number: 86000	Date: January 25, 2011 Filename: R44_TechnicalNotes_Fig8-3-1b_r1		

Figure 5 - R-44 Technical Notes

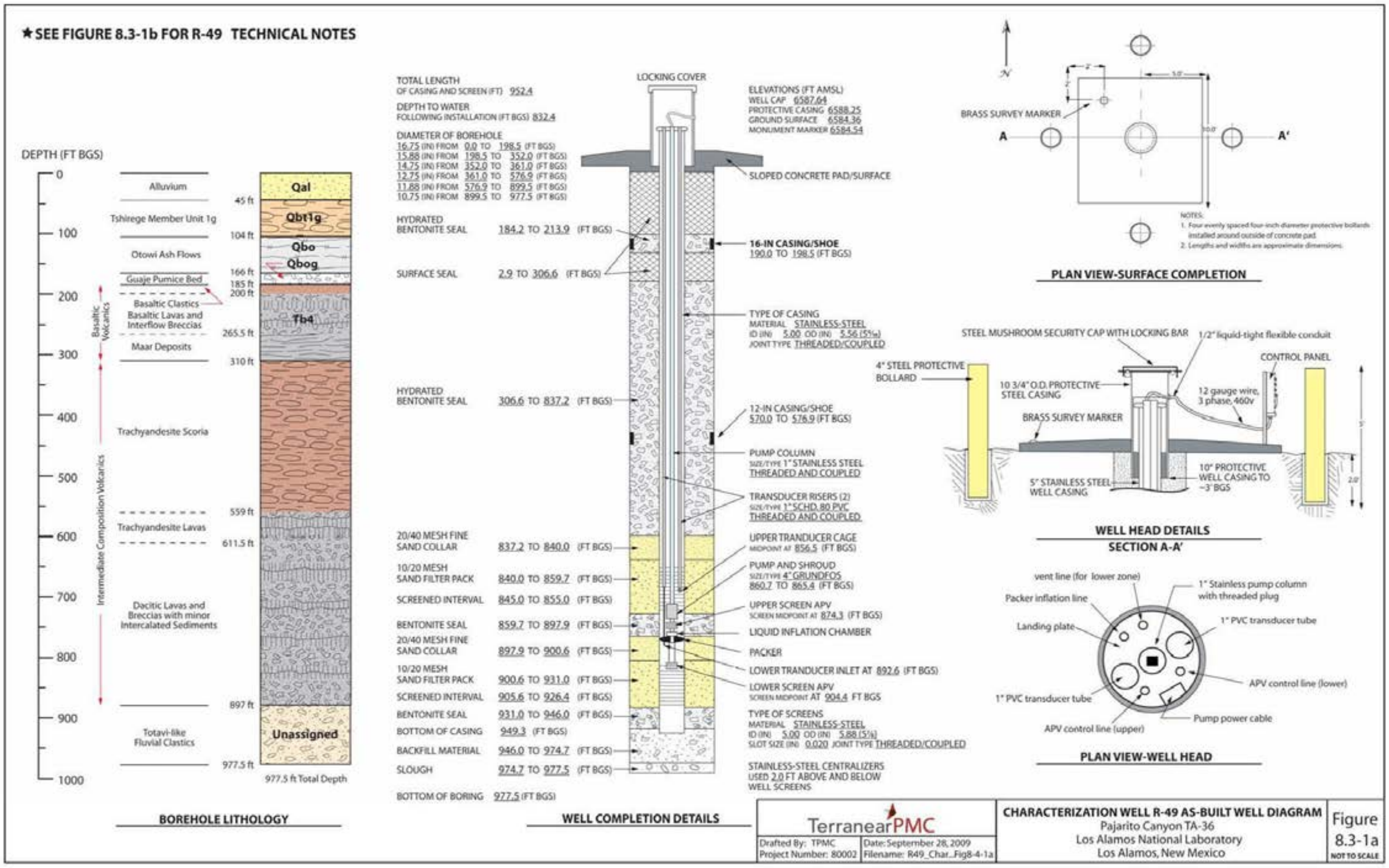


Figure 6 - R-49 As-Built Well Diagram

R-49 TECHNICAL NOTES: *

SURVEY INFORMATION[2]

Brass Marker
Northing: 1756401.85 ft
Easting: 1643900.90 ft
Elevation: 6584.54 ft AMSL

Well Casing (top of stainless steel)
Northing: 1756396.44 ft
Easting: 1643903.62 ft
Elevation: 6587.64 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL: natural gamma ray, induction (× 3)
Schlumberger: HNGS, APS, FMI, CMR, AIT

DRILLING INFORMATION

Drilling Company
Boart Longyear

Drill Rig
Foremost DR-24HD

Drilling Methods
Dual Rotary
Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids
Air, potable water, AQF-2 Foam

MILESTONE DATES

Drilling
Start: 03/30/2009
Finished: 04/30/2009

Well Completion
Start: 05/03/2009
Finished: 06/01/2009

Well Development
Start: 06/03/2009
Finished: 06/13/2009

WELL DEVELOPMENT

Development Methods
Performed swabbing, bailing, and pumping
Total Volume Purged: 25075 gallons (both screens)

Parameter Measurments (Final, upper screen/lower screen)
pH: 8.15/8.03
Temperature: 25.51/22.26°C
Specific Conductance: 151/122 µS/cm
Turbidity: 498/3.0 NTU

AQUIFER TESTING

Constant Rate Pumping Tests

Upper Screen
Water Produced: 2413 gallons
Average Flow Rate: 1.5 gpm
Performed on: 06/14–18/2009

Lower Screen
Water Produced: 38021 gallons
Average Flow Rate: 23.3 gpm
Performed on: 06/19–23/2009

DEDICATED SAMPLING SYSTEM

Pump
Type: Grunfos
Model: 5520-39DS
5 U.S. gpm, APVs (Access Port Valves) midpoints at 874.3 (Upper) and 904.4 (Lower) ft bgs

Motor
Type: Franklin Electric
Model: 2343258600
2hp, 3-phase

Pump Column
1-in. threaded/coupled sched. 40 stainless-steel tubing

Transducer Tubes
1-in. flush threaded schd. 80 PVC tubing
Upper: 0.01-in. slot screen at 856.2–856.8 ft bgs
Lower: flexible tube from transducer set at 892.6 ft bgs

Transducers
Make: In-Situ, Inc.
Model: Level TROLL 500
30 psig range (vented)
S/N: 149360, 149409

NOTES:
* Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83)
Elevation expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

TerranearPMC		R-49 TECHNICAL NOTES Pajarito Canyon (TA-36) Los Alamos National Laboratory Los Alamos, New Mexico	Figure 8.3-1b NOT TO SCALE
Drafted By: TPMC Project Number: 80002	Date: September 28, 2009 Filename: R49_TechnicalNotes_Fig8-3-1b		

Figure 7 - R-49 Technical Notes

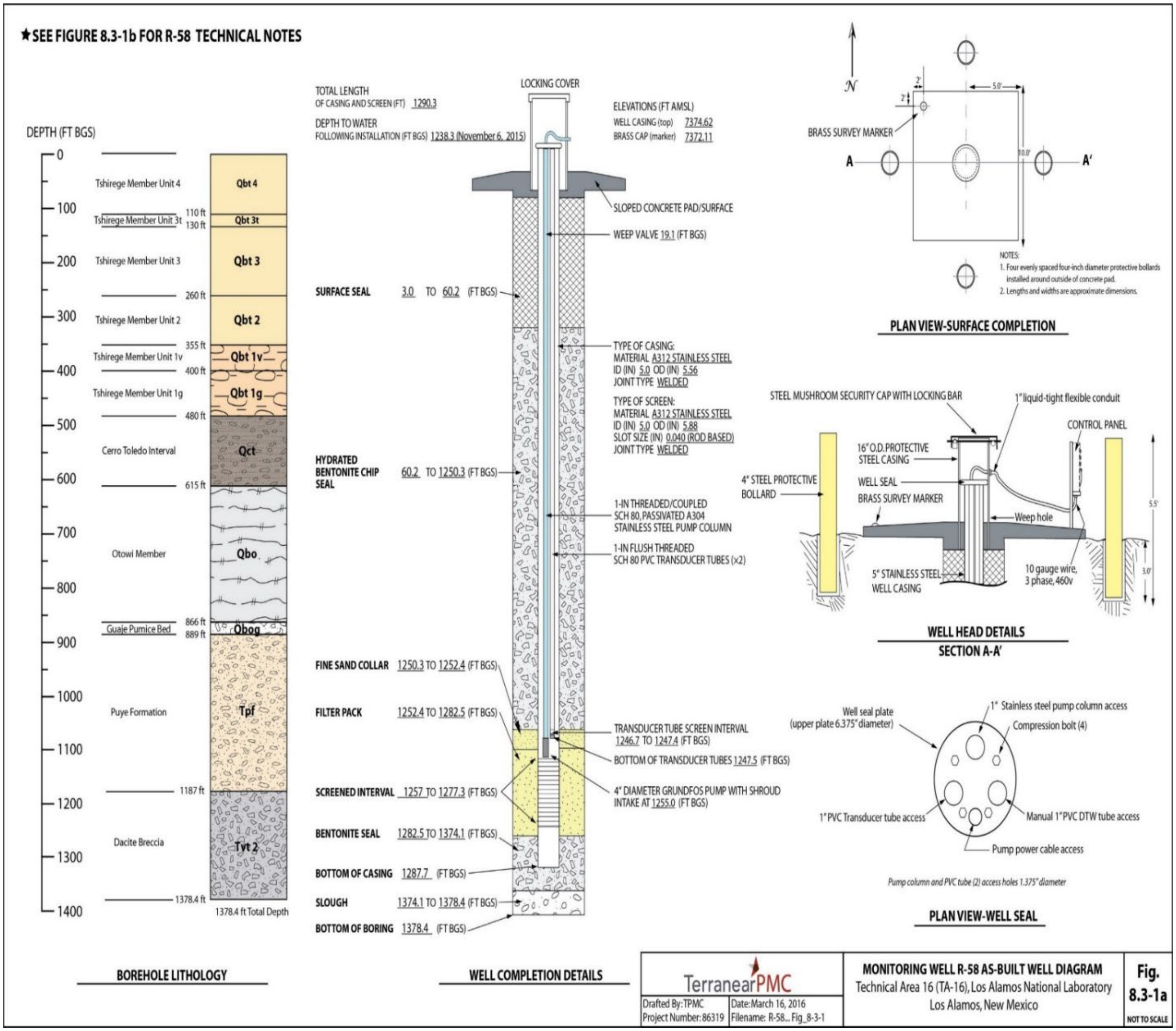


Figure 8 - R-58 As-built Diagram

R-58 TECHNICAL NOTES:

SURVEY INFORMATION*

Brass Marker

Northing:	1761298.75 ft
Easting:	1619435.65 ft
Elevation:	7372.11 ft AMSL

Well Casing (top of stainless steel)

Northing:	1761295.35 ft
Easting:	1619437.86 ft
Elevation:	7374.62 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL natural gamma log

DRILLING INFORMATION

Drilling Company
Boart Longyear

Drill Rig
Foremost DR-24HD

Drilling Methods
Dual Rotary
Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids
Air, potable water, AQF-2 Foam (to 1178 ft bgs)

MILESTONE DATES

Drilling

Start:	09/02/2015
Finished:	09/17/2015

Well Completion

Start:	09/28/2015
Finished:	11/05/2015

Well Development

Start:	11/06/2015
Finished:	11/13/2015

WELL DEVELOPMENT

Development Methods
Performed swabbing, bailing, and pumping
Total Volume Purged: 39,640 gal.

Parameter Measurements (Final)

pH:	8.04
Temperature:	19.52°C
Specific Conductance:	107 µS/cm
Turbidity:	5.0 NTU

AQUIFER TESTING

Constant Rate Pumping Test

Water Produced:	25,626 gal.
Average Flow Rate:	18.8 gpm
Performed on:	11/14–19/2015

DEDICATED SAMPLING SYSTEM

Pump (Shrouded)
Make: Grundfos
Model: 10S50-930CBM
S/N: P115450003
Environmental retrofit
Top of pump intake 1252.6 ft bgs
Base of shroud 1255.0 ft bgs

Motor
Make: Franklin Electric
Model: 2343278602
5 hp, 3-phase, 460V

Pump Shroud
Pumps of Oklahoma custom 4.6-in. O.D. schd. 5 A304 stainless steel with schd. 40 pipe connections

Pump Column
1-in. threaded/coupled schd. 80, pickled and passivated A304 stainless steel tubing
Weep valve installed at 19.1 ft bgs
Check valve installed at 1222.5 ft bgs

Transducer Tubes
2 × 1-in. flush threaded schd. 80 PVC tubing, 0.010-in. slot screens at 1246.7-1247.4 ft bgs

Transducer
Make: In-Situ, Inc.
Model: Level TROLL 500
30 psig range (vented)
S/N: 431623

NOTES:
* Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83); Elevation expressed in feet amsl using the National Geodetic Vertical Datum of 1929.

TerranearPMC		R-58 TECHNICAL NOTES Technical Area 16 (TA-16) Los Alamos National Laboratory Los Alamos, New Mexico	Fig. 8.3-1b NOT TO SCALE
Drafted By: TPMC Project Number: 86319	Date: February 3, 2016 Filename: R-58_TechnicalNotes_Fig8.3-1b		

Figure 9 - R-58 Technical Notes

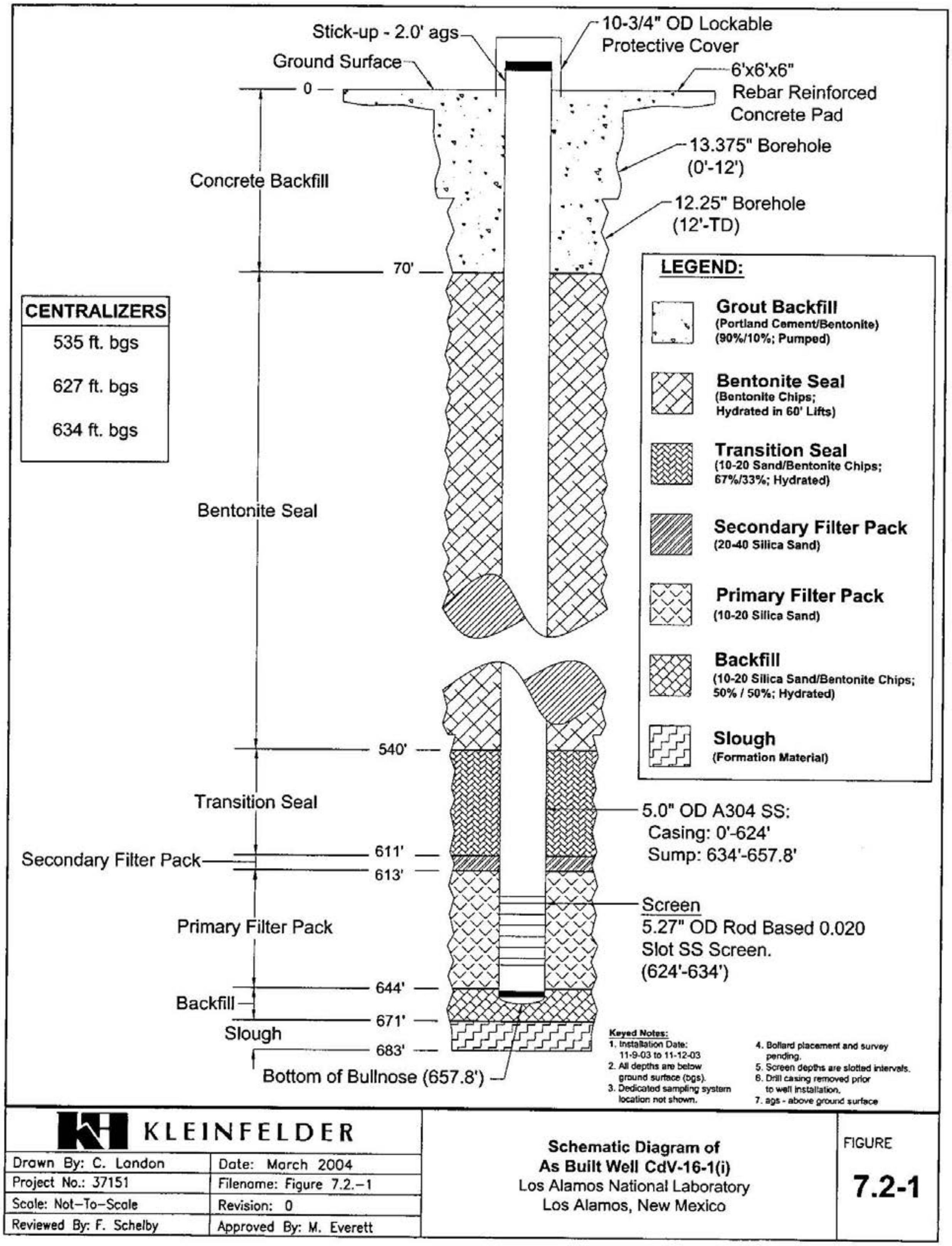


Figure 10 - CdV16-1(i) Schematic Diagram of As-Built Well

Appendix B

Well Maintenance Report, R-44 – Mortandad Watershed, December 2008 – February 2021

Well Maintenance Report

R-44 – Mortandad Watershed

December 2008 – February 2021

N3B Los Alamos

CONTENTS

Tables

Appendixes

1.0 Executive Summary

Well R-44, located in the Mortandad watershed, has been experiencing issues holding packer pressure. Although the well remains functional, it has been placed on life support to ensure no crossflow occurs between Screen 1 (S1) and Screen 2 (S2). This report contains data collected throughout the installation, completion of the well, and sample events from MY 2019 Q2 to MY 2021 Q2.

2.0 Well Install and Completion

R-44 well installation and completion began on December 13, 2008 and continued until January 25, 2011.

- Well installation from December 13, 2008 to January 15, 2009
- Well development from January 15, 2009 to January 20, 2009
- Aquifer testing from February14, 2009 to February 17, 2009
- Sample system installation occurred between February 18, 2009 and January 25, 2011 (No exact beginning or end dates are provided and February 18 is based on the end of the aquifer testing on February 17. January 25, 2011 is listed on the updated as-built included in Appendix A)

2.1 Deviations

Three deviations of note were found during review. First, heavy score marks were discovered on the bit just above the cones after drilling in the Puye formation, indicative of the bit cutting into the steel drive shoe. Upon further inspection, it was decided that there was a misalignment somewhere toward the bottom of the 12-in casing string, and that removal of the entire casing string was imperative. After removal, the suspicion that there was a misalignment was confirmed with a slightly bent bottom casing joint and a cracked drive shoe weld. Both the casing joint and the drive shoe weld were replaced and the casing string was reinstalled in the borehole. Second, groundwater was originally encountered in the Puye formation at 739 ft bgs. It was later discovered, however, that the water encountered at that depth was drilling fluid and the true water level was 879.1 ft bgs. Lastly, during R-44 aquifer testing, a leak was identified in the threaded joints in the 1 ½-in. stainless steel drop pipe, which created voids beneath the check valves. This created elevated pumping rates at the beginning of the tests causing the data to be corrupted and add uncertainty to the analysis of the early drawdown data. This leads to wide variations in turbidity in both the well completion report and the parameter plots in Appendix D

2.2 Camera Survey Information

A downhole camera survey was conducted on November 17, 2008. This video log determined the groundwater level to be at 739 ft bgs. The logbook noted that there was a knot in the wire line, however, making the actual depth uncertain. Upon completion of the well, the water level of 739 ft bgs was found to be incorrect, and that the water found during the survey was actually fluid introduced into the borehole during drilling. No additional camera survey was conducted after this was discovered.

2.3 Equipment List with Set Depths

Table 1 provides a list of downhole equipment with associated set depths.

Table 1 – R-44 Downhole Equipment

Downhole Equipment	Set Depth (ft BGS)
Flush Welded Stainless Steel Well Casing (ID: 5in OD: 5.56 in)	1016.0
Flush Welded Stainless Steel Screens (ID: 5in OD: 5.88in Slot Size: 0.020)	Screened interval midpoints 900.0 (S1) and 990.35 (S2)
1-in threaded/coupled Stainless Steel Pump Column	908.6
4" diameter Grundfos pump with shroud (Model# 5S30-820CBM)	908.7 to 916.8; APV midpoints at 921.9(S1) and 983.3(S2)
1-in flush threaded Schedule 80 PVC transducer tubes (x2)	Screen 1 midpoint 906.2 with 0.01-in slot screen. Lower transducer at 942.1

**See Appendix A for the well completion diagram, Appendix B for the pump curve, and Appendix E for the full well completion report.*

3.0 Sampling and pumping data

Sampling and pumping data presented in this report was collected from 21 sample events from MY 2019 Q2 to MY 2021 Q2. Graphs and tables presenting standing water levels before and after purge, purge volume over time, discharge head pressure, and field data from the 24 most recent sample events from screens 1 and 2 are located in Appendix D. The data in Appendix D show that the standing water level has fallen over time for the measurements before and after purging for both screen 1 and screen 2. Discharge head pressure fell by ~0.8psi in screen 1 and was stable for screen 2 over the time period reviewed. Specific conductivity was relatively stable throughout the observed period, but contained two outliers in screen 1 (11/20/19 and 12/10/19) and one outlier in screen 2 (9/12/19). ORP in screen 1 was erratic throughout the observed period but screen 2 was stable. Turbidity was erratic in both screen 1 and 2. All other observed parameters were stable throughout the reviewed period. The pump is working as intended and has experienced no issues during the sample events presented here. Also included in Appendix D, due to the nature of the issues occurring at R-44, is the packer pressure graph over time.

3.1 Sampling Observations of Note

Sampling between MY 2019 Q2 and MY 2021 Q2 went smoothly and two issues were reported during operations. On 7/29/20 the field crew had issues connecting to the lower transducer in well R-44. This led to no data being collected for water levels both before and after sampling R-44 S2. This is reflected in the graphs. Secondly, on 8/11/20, tank psi dropped from 950psi to 800psi during a single sample event. Relevant logbook pages are available in Appendix C.

3.2 Purge Description

Table 2 provides water quality comments from purging events completed at R-44 S1 and Table 3 provides water quality comments from purging events completed at R-44 S2.

Table 2 – R-44 S1 Water Quality

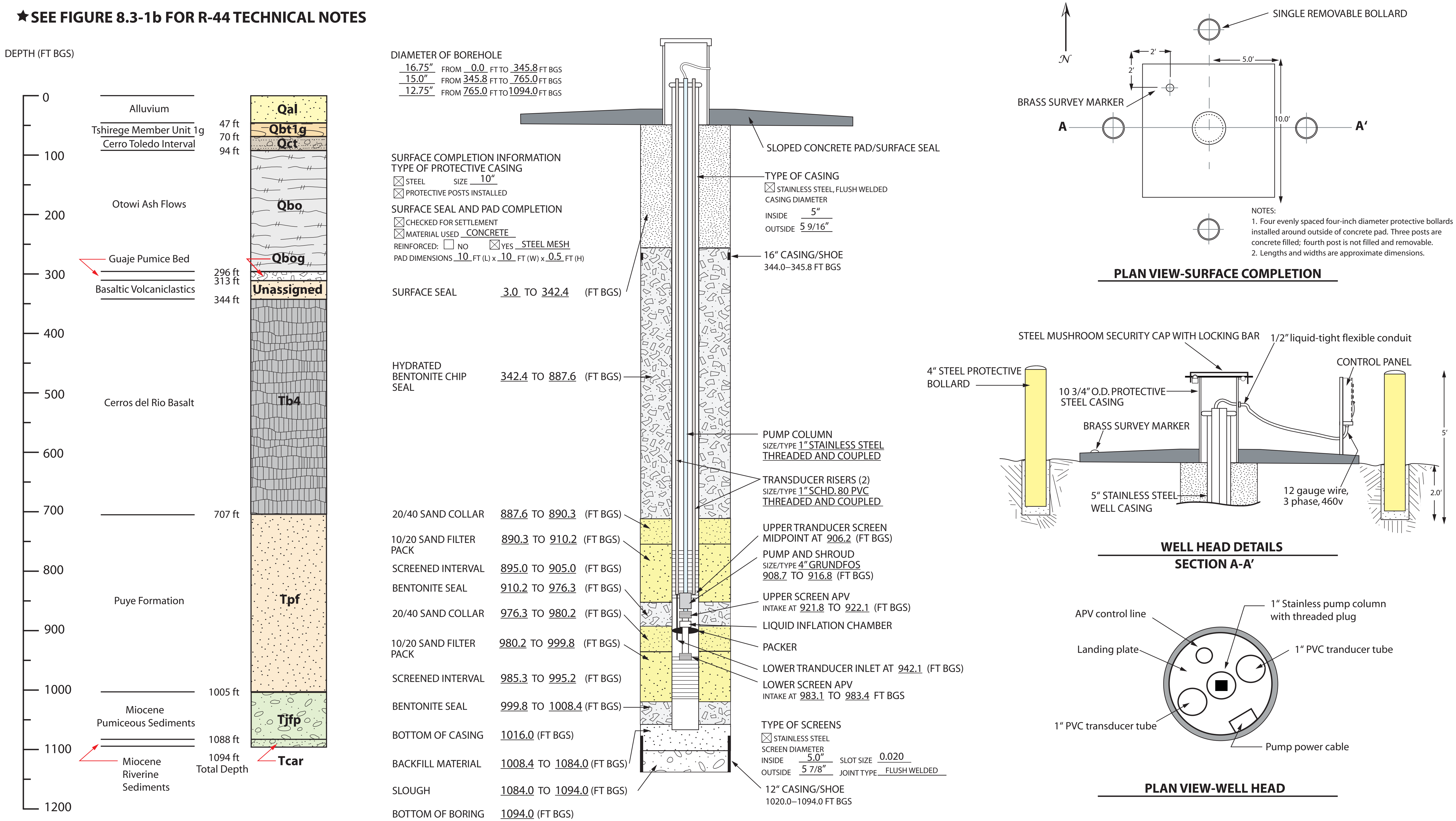
Date	Clarity	Odor	Effervescence	Other Comments
1/22/2019	Clear	None	N/A	Used HACH turbidity meter
2/11/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
3/20/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
4/30/2019	Clear	None	N/A	None
5/15/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
6/18/2021	Clear	None	N/A	None
7/15/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
8/27/2019	Clear	None	Slight Effervescence	None
9/12/2019	Clear	None	Slight Effervescence	None
10/11/2019	Clear	None	N/A	None
11/20/2019	Clear	None	Slight to Effervescent	Used HACH turbidity meter
12/10/2019	Clear	None	N/A	None
1/21/2020	Clear	None	Slight Effervescence	None
2/25/2020	Clear	None	N/A	None
3/12/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
6/25/2020	Clear	None	Slight to Effervescent	Used HACH turbidity meter
7/29/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
8/11/2020	Clear	None	Slight to High Effervescence	Used HACH turbidity meter
9/20/2020	Clear	None	Slight to Effervescent	Used HACH turbidity meter
10/8/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
11/17/2020	Clear	None	Slight to Effervescent	Used HACH turbidity meter
12/14/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
2/1/2021	Clear	None	Slight Effervescence	Used HACH turbidity meter

Table 3 – R-44 S2 Water Quality

Date	Clarity	Odor	Effervescence	Other Comments
1/22/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
2/11/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
3/20/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
4/30/2019	Clear	None	N/A	None
5/15/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
6/18/2019	Clear	None	Slight Effervescence	None
7/16/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
8/27/2019	Clear	None	Slight to Effervescent	Used HACH turbidity meter
9/12/2019	Clear	None	Effervescent	Used HACH turbidity meter
10/11/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
11/20/2019	Clear	None	Slight Effervescence	Used HACH turbidity meter
12/10/2019	Clear	None	N/A	Changed batteries in EXO
1/21/2020	Clear	None	N/A	None
2/20/2020	Clear	None	N/A	Used HACH turbidity meter
3/12/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
6/26/2020	Clear	None	N/A	Used HACH turbidity meter
7/29/2020	Clear	None	Slight to Effervescent	Used HACH turbidity meter
8/11/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
9/10/2020	Clear	None	Slight to Effervescent	Used HACH turbidity meter
10/8/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
11/17/2020	Clear	None	Slight Effervescence	Used HACH turbidity meter
12/14/2020	Clear	None	N/A	Used HACH turbidity meter
2/1/2021	Clear	None	Slight Effervescence	Used HACH turbidity meter

Appendix A

Well Completion Diagram



TerranearPMC		CHARACTERIZATION WELL R-44 AS-BUILT WELL DIAGRAM Mortandad Canyon Los Alamos National Laboratory Los Alamos, New Mexico	Figure 8.3-1a NOT TO SCALE
Drafted By: TPMC Project Number: 88100	Date: January 25, 2011 Filename: R44_Char...Fig8-4-1a		

R-44 TECHNICAL NOTES: [1]

SURVEY INFORMATION[2]

Brass Marker

Northing:	1767109.85 ft
Easting:	1640061.34 ft
Elevation:	6714.91 ft AMSL

Well Casing (top of stainless steel)

Northing:	1767104.36 ft
Easting:	1640063.49 ft
Elevation:	6717.56 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL: natural gamma ray, induction, video
Schlumberger: natural gamma ray, elemental capture (ECS), compensated neutron (CNTG), litho-density (TLD)

DRILLING INFORMATION

Drilling Company
Boart Longyear

Drill Rig
Foremost DR-24HD

Drilling Methods
Dual Rotary
Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids
Air, potable water, AQF-2 Foam

MILESTONE DATES

Drilling

Start:	11/10/2008
Finished:	12/08/2008

Well Completion

Start:	12/13/2008
Finished:	01/15/2009

Well Development

Start:	01/15/2009
Finished:	01/20/2009

WELL DEVELOPMENT

Development Methods
Performed swabbing, bailing, and pumping
Total Volume Purged: 16005 gallons (both screens)

Parameter Measurments (Final, upper screen/lower screen)

pH:	8.22/8.19
Temperature:	18.48/18.78˚C
Specific Conductance:	142/193 µS/cm
Turbidity:	0.0/0.0 NTU

AQUIFER TESTING

Step-Tests and Constant Rate Pumping Tests

Upper Screen

Water Produced:	38223 gallons
Average Flow Rate:	24.1 gpm
Performed on:	02/14–17/2009

Lower Screen

Water Produced:	38701 gallons
Average Flow Rate:	23.9 gpm
Performed on:	02/19–22/2009

DEDICATED SAMPLING SYSTEM

Pump
Type: Grundfos
Model: 5S30-820CBM
5 U.S. gpm, APVs (Acccess Port Valves) midpoints at 921.9 (upper) and 983.3 (lower) ft bgs
Environmental Retrofit

Motor
Type: Franklin Electric
Model: 2343265202
3hp, 3-phase

Pump Column
1-in. threaded/coupled stainless steel tubing

Transducer Tubes
2 × 1-in. flush threaded schd. 80 PVC tubing upper 0.01-in. slot × 0.5-ft screen at 906.2 ft bgs (midpoint), lower flexible tube from transducer set at 942.1 ft bgs

Transducers
Model: Level TROLL 500
30 psig range (vented)
S/Ns: 148101, 148136

NOTES:
1) Additional information available in "Final Completion Report, Characterization Well R44 and R45, Los Alamos National Laboratory, Los Alamos, New Mexico, TBD 2009".
2) Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83); Elevation expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

TerranearPMC	R-44 TECHNICAL NOTES Mortandad Canyon Los Alamos National Laboratory Los Alamos, New Mexico	Figure 8.3-1b NOT TO SCALE
Drafted By: TPMC Project Number: 86000 Date: January 25, 2011 Filename: R44_TechnicalNotes_Fig8-3-1b_r1		

Appendix C

Logbook Pages

7-29-2020 R-44 Screen 2

1146 crew switching from screen 1 to screen 2, see p 256 of this log book for morning activities. Crew: Andrew Stocker, Luke Rykaskey, Ashley Kowalewski Objective: purge & sample R-44 S2 per applicable SOPs on p2 of this log book using a baski & GSP. weather: sunny, breezy, 80s 2 min safety: again, uneven surfaces Note: Ashley connects lower transducer to HP 2046

I = NC LSD = 67 ~~04.02~~ 14.91' msl TD = 1016' bgs
DTW = Bottom of packer = ~~926.0~~ 941.0' bgs AMK 7-29-20 WC = (TD - DTW) = ~~90.0~~ 74.9' AMK 7-29-20
1" ID DP = (0.041 gal/ft) TD = 41.66 gal
5" ID 1CV = (1.020 gal/ft) WC = ~~91.80 gal~~ 76.40 gal AMK 7-29-20
3CVs = ~~275.40 gal~~ 229.19 gal AMK 7-29-20

1216 pump on, water to surface
flow rate measured via 5 gal carboy. Q = 3.00 gpm
Note: having trouble connecting lower screen transducer to HP 2046

Note: flow meter still not functioning
Note: Packer pressure before actuation at 440 psi is 210 psi, at opening it is 215 psi

1350 3 CVs purged, parameters stable, begin sampling
1357 Sampling complete, pump off objective to purge and sample R-45 S2 per SOPs on p2 of this log book using a GSP + Baski complete. Flow meter working again and final reading is 17808.8 gal
Note: Luke preserves and Andrew QA's $HNO_3 + H_2SO_4 \leq 2$pH and $NaOH \geq 12$pH.
Final packer pressure is 210 psi
1418 crew offsite.
1500 Submit samples to SMO, care of Kat Papura
Event ID: 13162 Field sample ID: CAML0-20-205416, -205415, -205417

AMK 7-29-2020

126

8/11/2020 R-44 S2

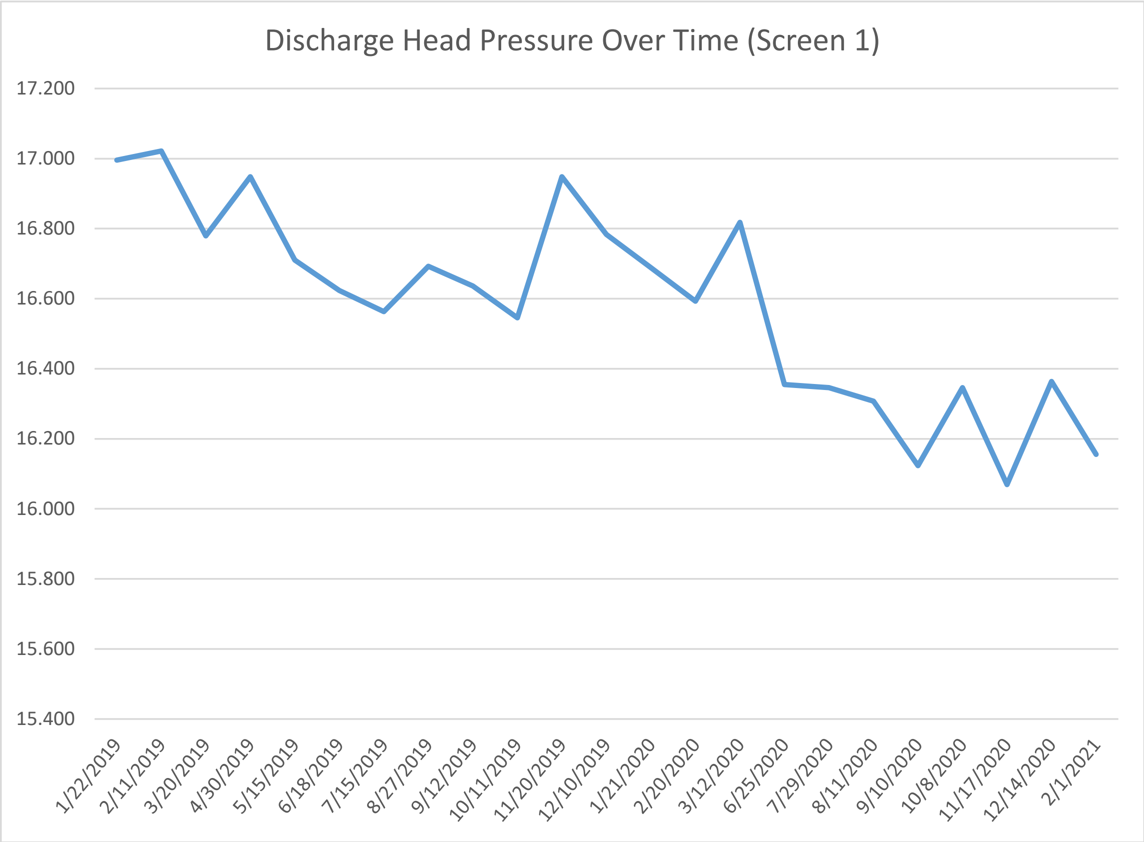
Note: See pg. 123 of this logbook for morning activities
Note: Initial Flowmeter Reading = 18025.2 gal
1033 Crew (J. Meyer, A. Vigil and D. Jaramillo)
Note: A. Vigil Connects transducer to lower screen w/ Rugged Reader #4; D. Jaramillo QAs
Σ = 5829.89' msl LSD = 6714.91' msl TD = 1016.0' bgs
DTW = BOP = 941.1' bgs WC = TD – DTW = 74.90'
1" ID DP = (0.041 gal/ft) TD = 41.66 gal
5" ID 1CV = (1.020 gal/ft) WC = 76.40 gal
3CVs = 229.19 gal
Initial tank pressure = 950 psi
1040 Pump on; water to surface; Q = 3.00
1054 DP clear (41.66 gal); Begin logging parameters on YSI #80
File name r44S2 + GW sampling log + IPAD #3 (J. Meyer)
1214 Begin Sampling, 3CVs + DP cleared + Parameters stable
1220 Sampling complete; Pump off
Note: Final Flowmeter Reading = 18310.80 gal
Summary Objective met to purge + Sample R-44 S2 per all applicable SOPs and reference documents on pg. 3 of this logbook
Note: A. Vigil Preserves; D. Jaramillo QAs
Note: J. Meyer Reconnects transducer to Net troll hub; A. Vigil QAs
Final N_2 tank pressure = 800 psi
Note: A. Vigil Plunges well
1240 Crew offsite
Relinquish Samples to SMO c/o Kat Popova
Event ID: 13059
Samples ID: CAMO-20-198886, 198885, 206158, 198887

JM 8/11/2020

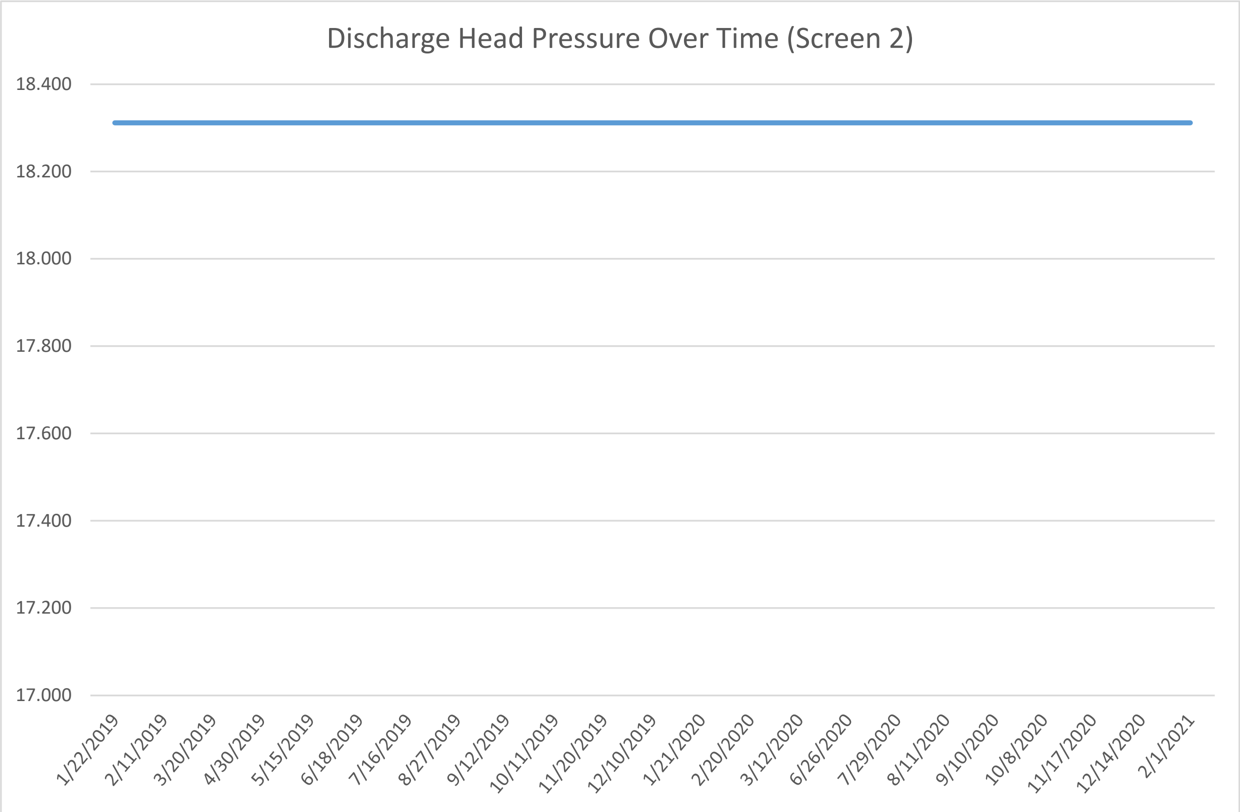
Appendix D

Graphs and Tables

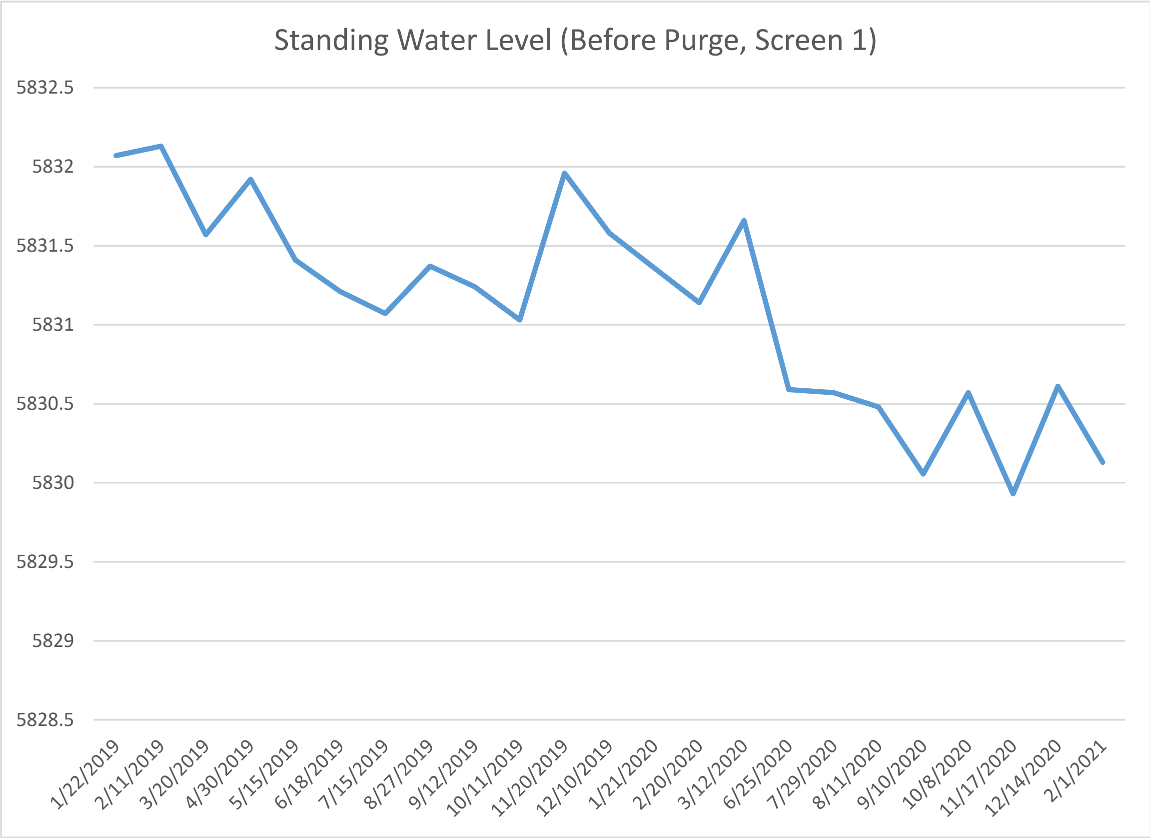
Date	Discharge Head Pressure (PSI)
1/22/2019	16.996
2/11/2019	17.022
3/20/2019	16.779
4/30/2019	16.948
5/15/2019	16.710
6/18/2019	16.623
7/15/2019	16.563
8/27/2019	16.693
9/12/2019	16.636
10/11/2019	16.545
11/20/2019	16.948
12/10/2019	16.784
1/21/2020	16.688
2/20/2020	16.593
3/12/2020	16.818
6/25/2020	16.355
7/29/2020	16.346
8/11/2020	16.307
9/10/2020	16.123
10/8/2020	16.346
11/17/2020	16.069
12/14/2020	16.364
2/1/2021	16.15584416



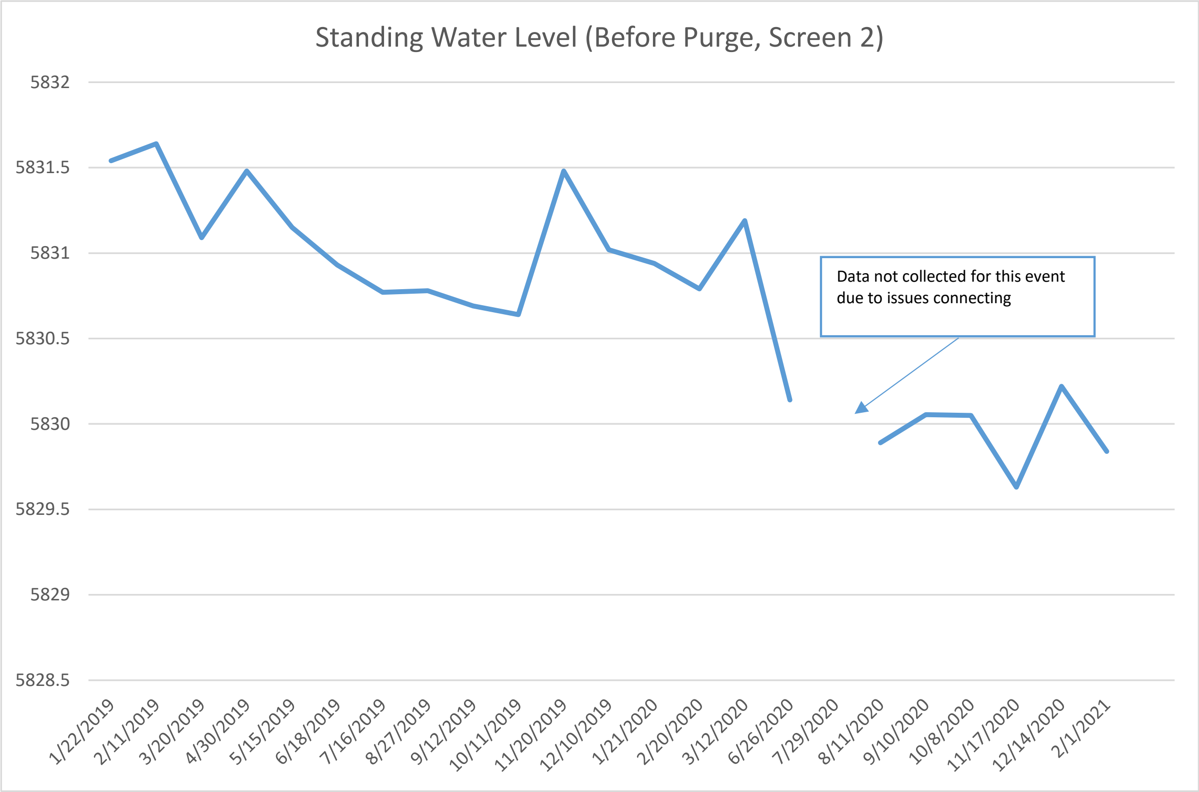
Date	Discharge Head Pressure (PSI)
1/22/2019	18.312
2/11/2019	18.312
3/20/2019	18.312
4/30/2019	18.312
5/15/2019	18.312
6/18/2019	18.312
7/16/2019	18.312
8/27/2019	18.312
9/12/2019	18.312
10/11/2019	18.312
11/20/2019	18.312
12/10/2019	18.312
1/21/2020	18.312
2/20/2020	18.312
3/12/2020	18.312
6/26/2020	18.312
7/29/2020	18.312
8/11/2020	18.312
9/10/2020	18.312
10/8/2020	18.312
11/17/2020	18.312
12/14/2020	18.312
2/1/2021	18.31168831



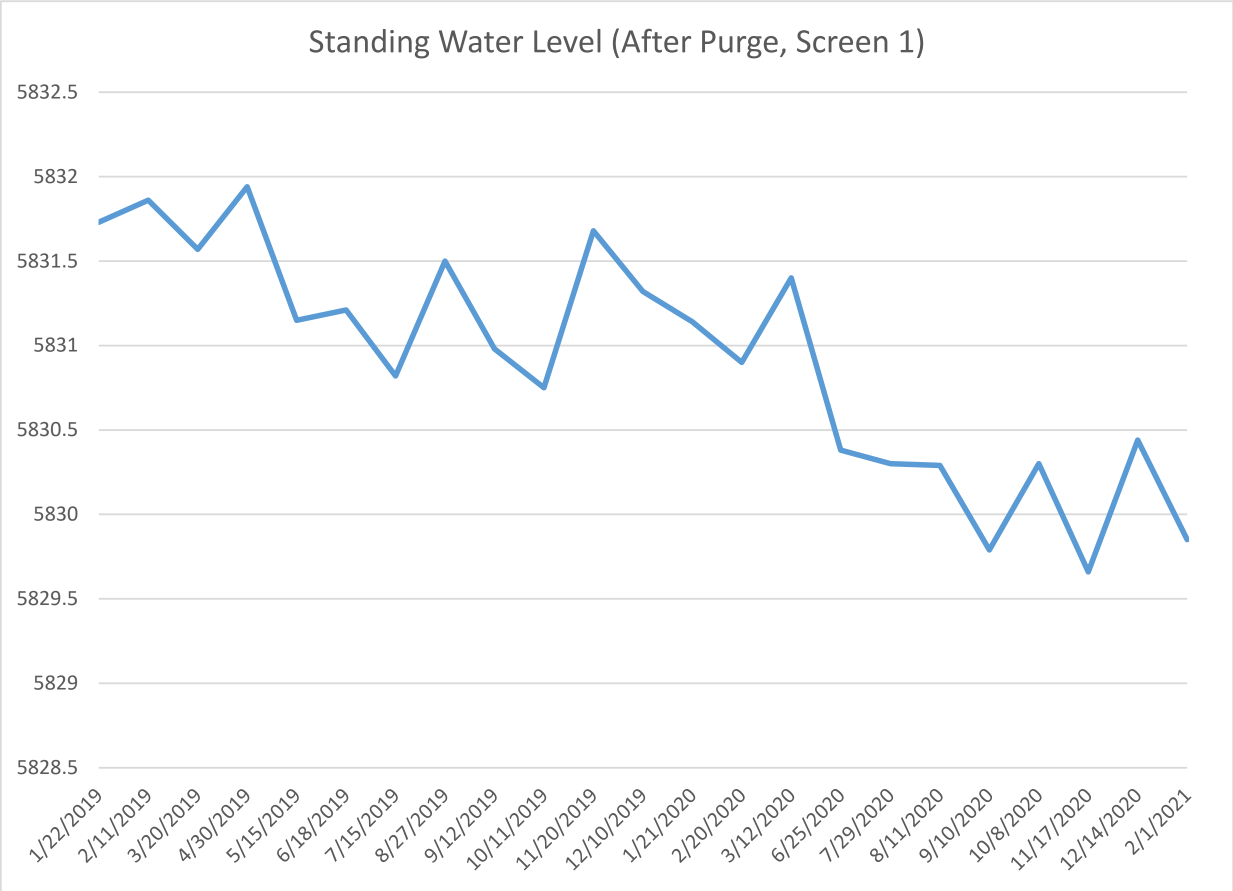
Date	Time	Water Level (ft msl)
1/22/2019	1000	5832.07
2/11/2019	1145	5832.13
3/20/2019	920	5831.57
4/30/2019	958	5831.92
5/15/2019	950	5831.41
6/18/2019	953	5831.21
7/15/2019	925	5831.07
8/27/2019	1004	5831.37
9/12/2019	940	5831.24
10/11/2019	945	5831.03
11/20/2019	940	5831.96
12/10/2019	1005	5831.58
1/21/2020	905	5831.36
2/20/2020	1015	5831.14
3/12/2020	1350	5831.66
6/25/2020	1027	5830.59
7/29/2020	945	5830.57
8/11/2020	840	5830.48
9/10/2020	1010	5830.055
10/8/2020	915	5830.57
11/17/2020	845	5829.93
12/14/2020	1025	5830.61
2/1/2021	825	5830.13



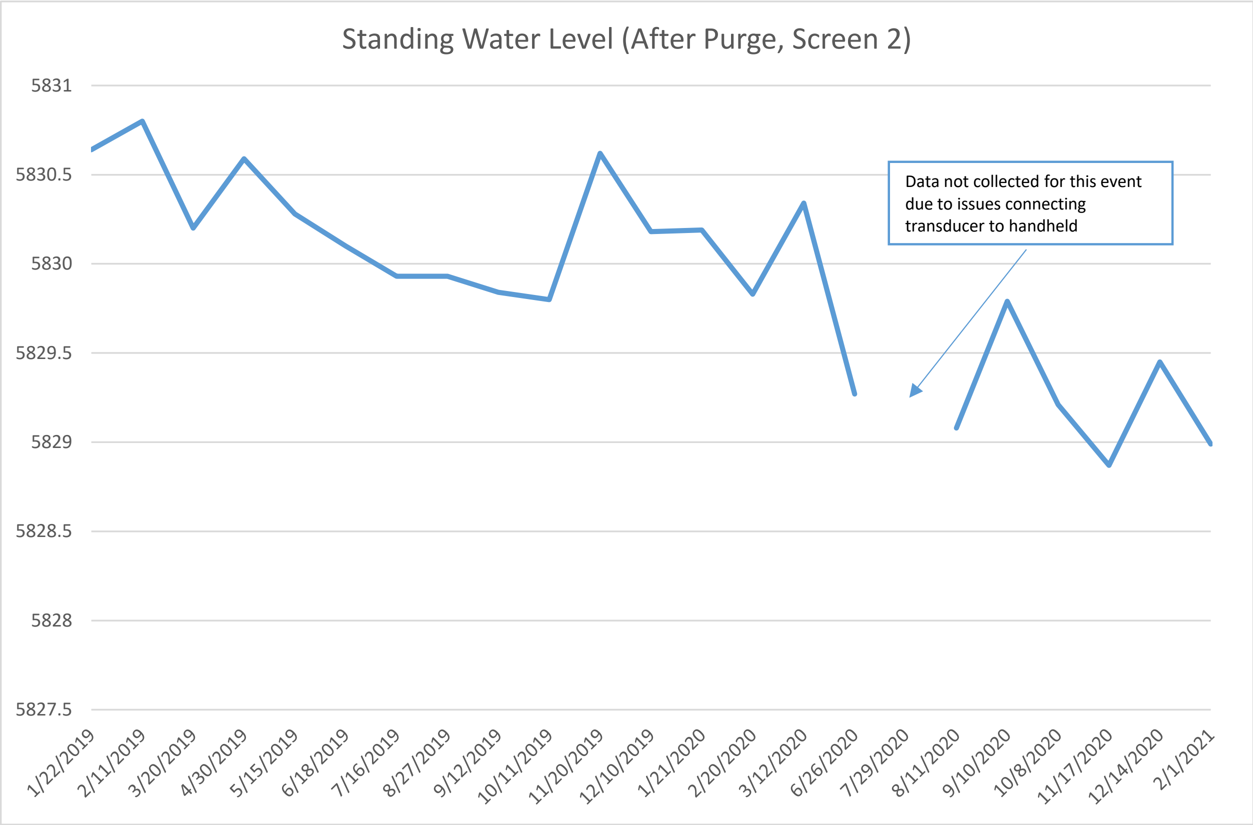
Date	Time	Water Level (ft msl)
1/22/2019	1125	5831.54
2/11/2019	955	5831.64
3/20/2019	1100	5831.09
4/30/2019	1230	5831.48
5/15/2019	1120	5831.15
6/18/2019	1145	5830.93
7/16/2019	1130	5830.77
8/27/2019	1140	5830.78
9/12/2019	1220	5830.69
10/11/2019	1125	5830.64
11/20/2019	1126	5831.48
12/10/2019	1208	5831.02
1/21/2020	1120	5830.94
2/20/2020	1310	5830.79
3/12/2020	1115	5831.19
6/26/2020	920	5830.14
7/29/2020	1210	Not Collected
8/11/2020	1035	5829.89
9/10/2020	1005	5830.055
10/8/2020	1100	5830.05
11/17/2020	1030	5829.63
12/14/2020	1231	5830.22
2/1/2021	1015	5829.84



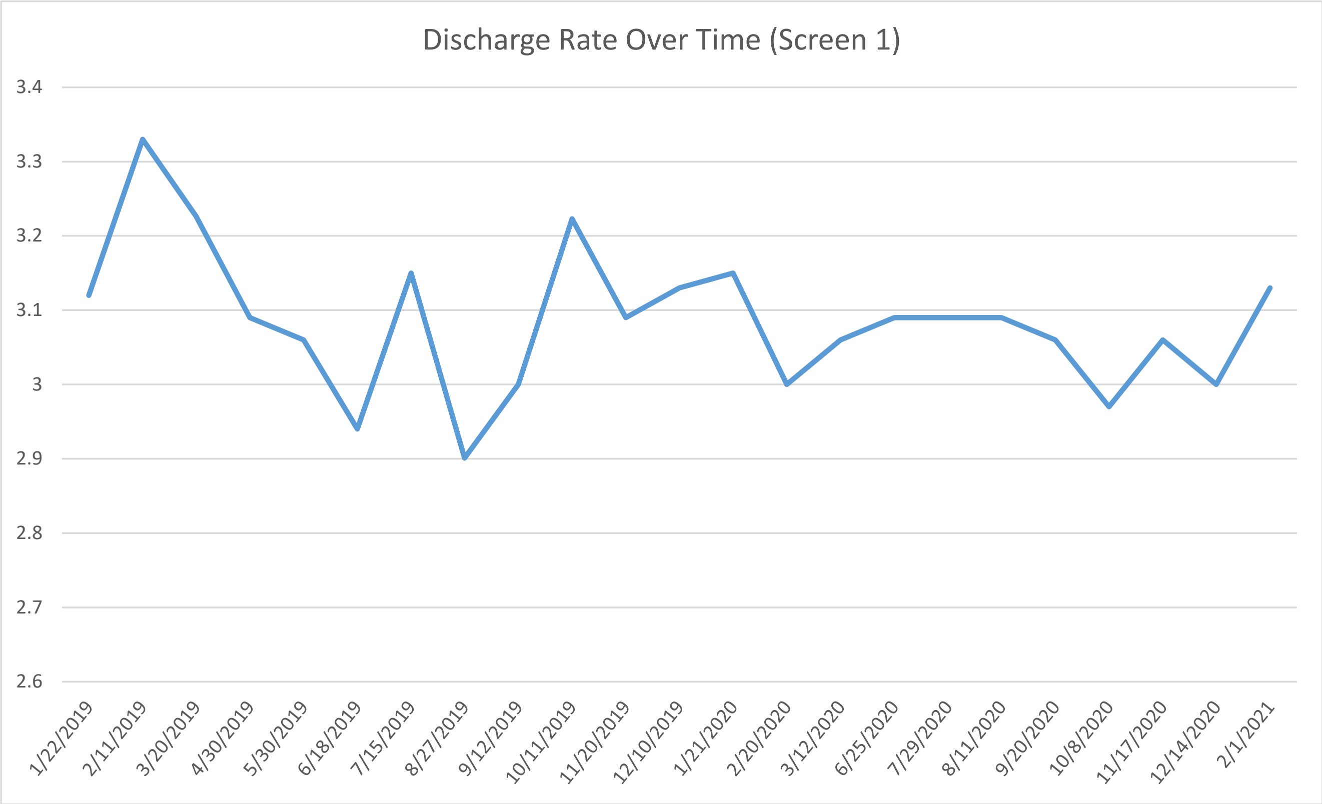
Date	Time	Water Level (ft msl)
1/22/2019	1315	5831.73
2/11/2019	1300	5831.86
3/20/2019	1050	5831.57
4/30/2019	1155	5831.94
5/15/2019	1115	5831.15
6/18/2019	1140	5831.21
7/15/2019	1130	5830.82
8/27/2019	1133	5831.5
9/12/2019	1156	5830.98
10/11/2019	1115	5830.75
11/20/2019	1121	5831.68
12/10/2019	1201	5831.32
1/21/2020	1044	5831.14
2/20/2020	1250	5830.9
3/12/2020	1515	5831.4
6/25/2020	1506	5830.38
7/29/2020	1140	5830.3
8/11/2020	1030	5830.29
9/10/2020	1216	5829.79
10/8/2020	1055	5830.3
11/17/2020	1030	5829.66
12/14/2020	1214	5830.44
2/1/2021	1000	5829.85



Date	Time	Water Level (ft msl)
1/22/2019	1310	5830.64
2/11/2019	1140	5830.8
3/20/2019	1235	5830.2
4/30/2019	1435	5830.59
5/15/2019	1300	5830.28
6/18/2019	1345	5830.1
7/16/2019	1309	5829.93
8/27/2019	1325	5829.93
9/12/2019	1405	5829.84
10/11/2019	1314	5829.8
11/20/2019	1312	5830.62
12/10/2019	1407	5830.18
1/21/2020	1300	5830.19
2/20/2020	1510	5829.83
3/12/2020	1331	5830.34
6/26/2020	1125	5829.27
7/29/2020	1400	Not Collected
8/11/2020	1225	5829.08
9/10/2020	1216	5829.79
10/8/2020	1250	5829.21
11/17/2020	1219	5828.87
12/14/2020	1430	5829.45
2/1/2021	1205	5828.99



Date	Rate (gal/min)
1/22/2019	3.12
2/11/2019	3.33
3/20/2019	3.226545455
4/30/2019	3.09
5/30/2019	3.06
6/18/2019	2.94
7/15/2019	3.15
8/27/2019	2.901071429
9/12/2019	3
10/11/2019	3.2232
11/20/2019	3.09
12/10/2019	3.13
1/21/2020	3.15
2/20/2020	3
3/12/2020	3.06
6/25/2020	3.09
7/29/2020	3.09
8/11/2020	3.09
9/20/2020	3.06
10/8/2020	2.97
11/17/2020	3.06
12/14/2020	3
2/1/2021	3.13



Date	Rate (gal/min)
1/22/2019	3.19
2/11/2019	3.19
3/20/2019	3.23
4/30/2019	3.19
5/15/2019	3.13
6/18/2019	2.8875
7/16/2019	3.13
8/27/2019	2.97
9/12/2019	3
10/11/2019	3.03
11/20/2019	3.09
12/10/2019	3.06
1/21/2020	3.26
2/20/2020	3
3/12/2020	3.003777778
6/26/2020	3.09
7/29/2020	3
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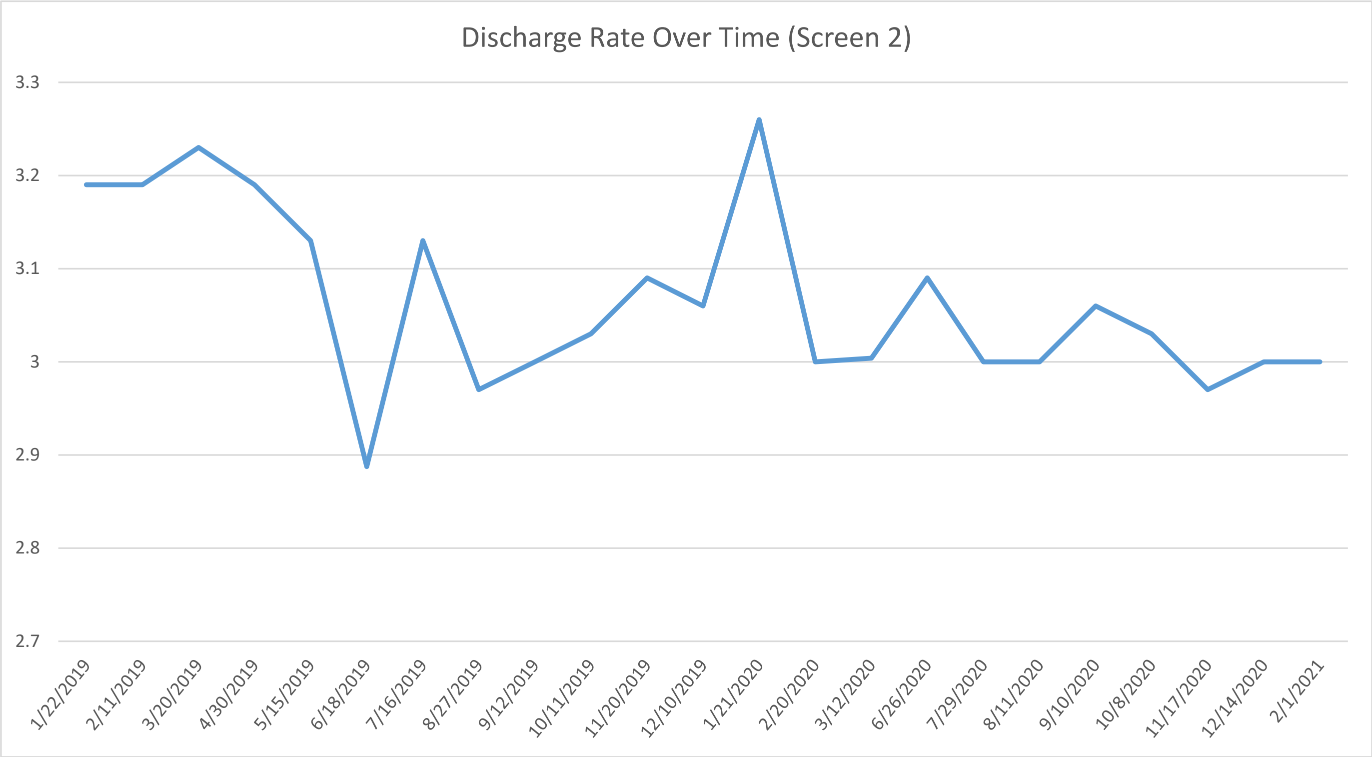
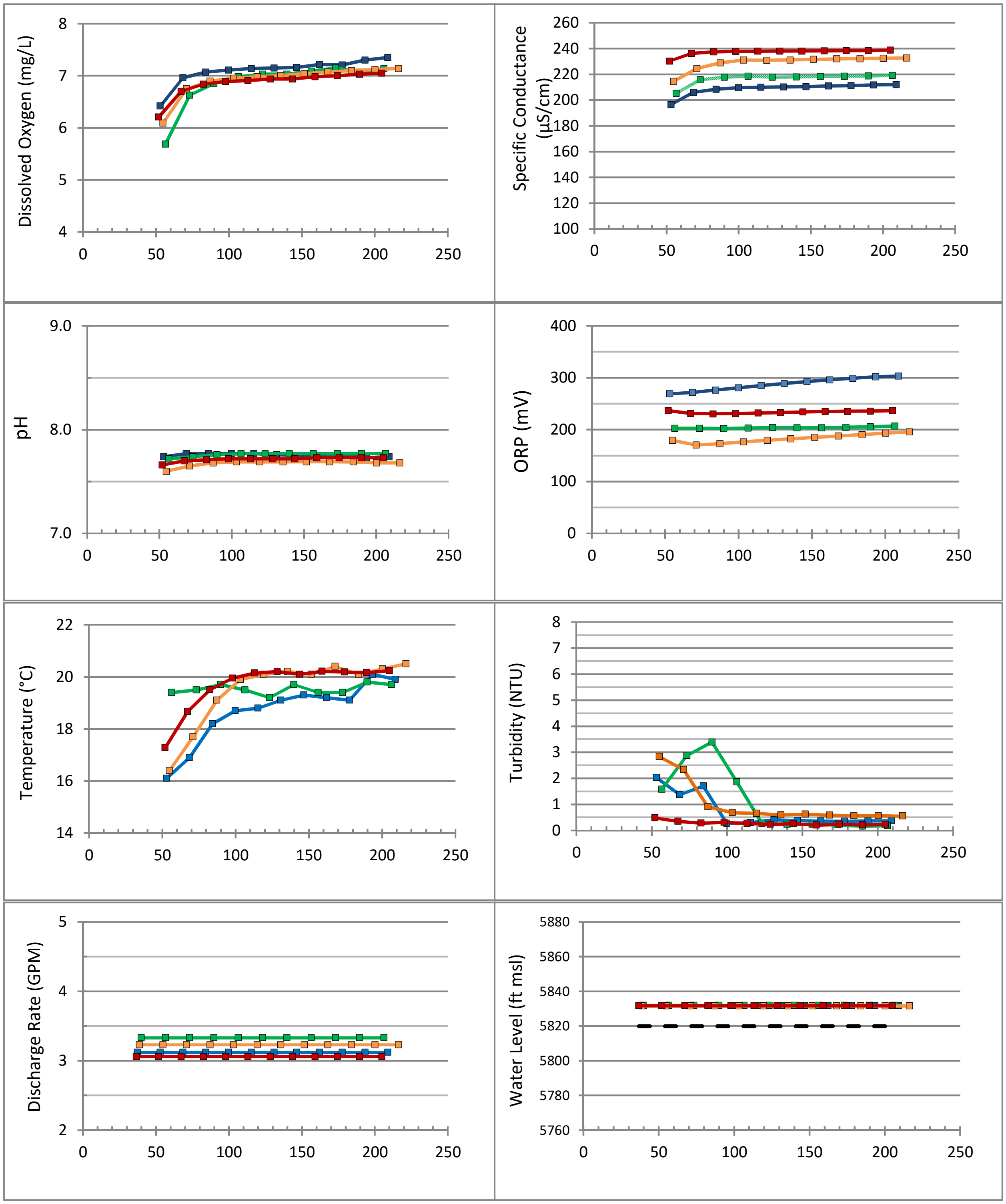
10/8/2020	3.03
11/17/2020	2.97
12/14/2020	3
2/1/2021	3



8/2/2019

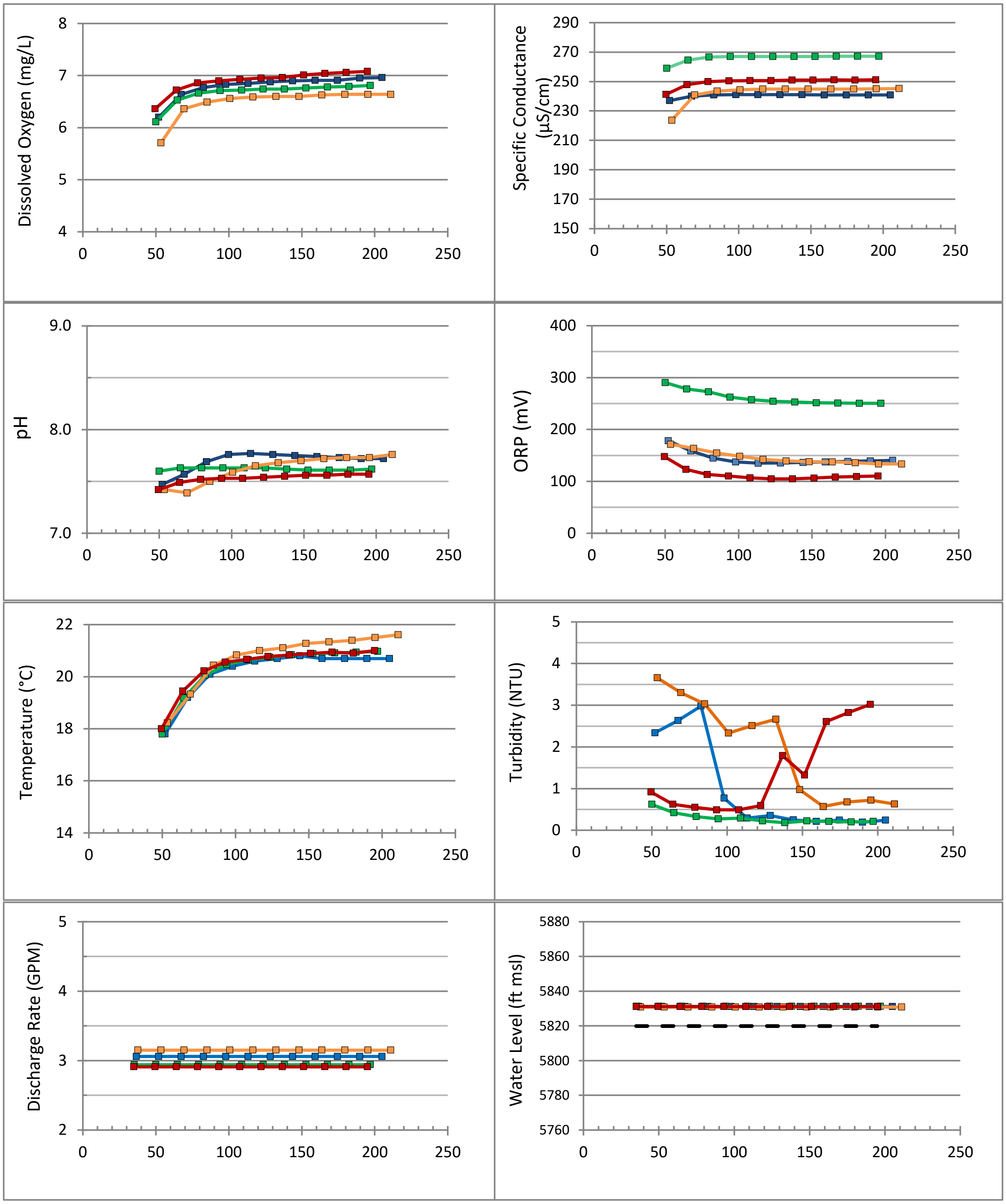
R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **54.42**	3CV+DP= **201.65**	DP= **38.39**
		Watershed: Mortandad	**Installed:** 1/15/19



x-axis = cumulative volume purged (gals.), including drop pipe

01/22/19 — 02/11/19 — 03/20/19 — 04/30/19 — scr top

R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **53.82**	3CV+DP= **199.85**	DP= **38.39**
		Watershed: Mortandad	**Installed:** 1/15/19

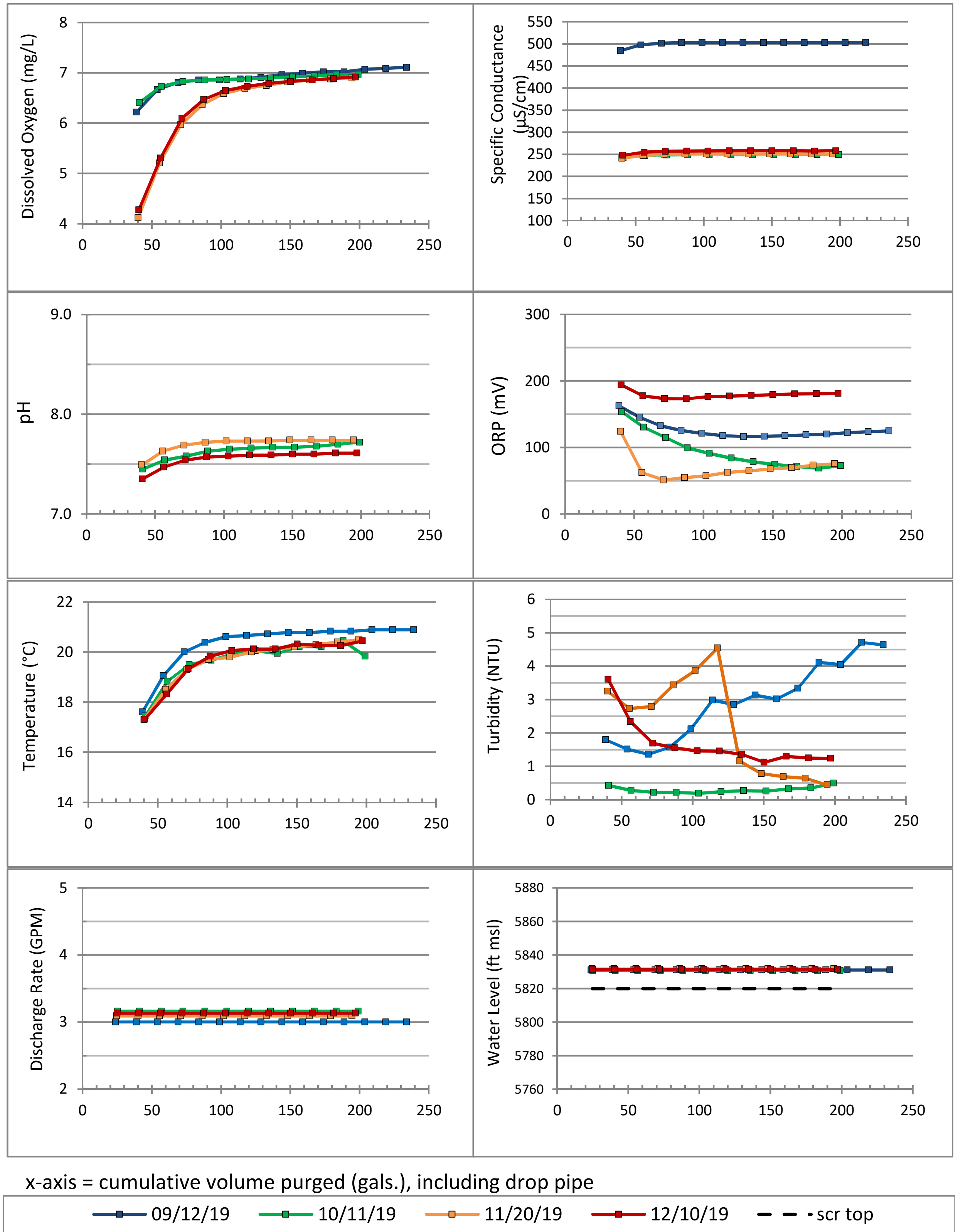


x-axis = cumulative volume purged (gals.), including drop pipe

05/15/19 06/18/19 07/15/19 08/27/19 scr top

5/21/2020

R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **54.03**	3CV+DP= **200.48**	DP= **38.39**
		Watershed: **Mortandad**	**Installed:** 1/15/19



R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **53.02**	3CV+DP= **197.45**	DP= **38.39**
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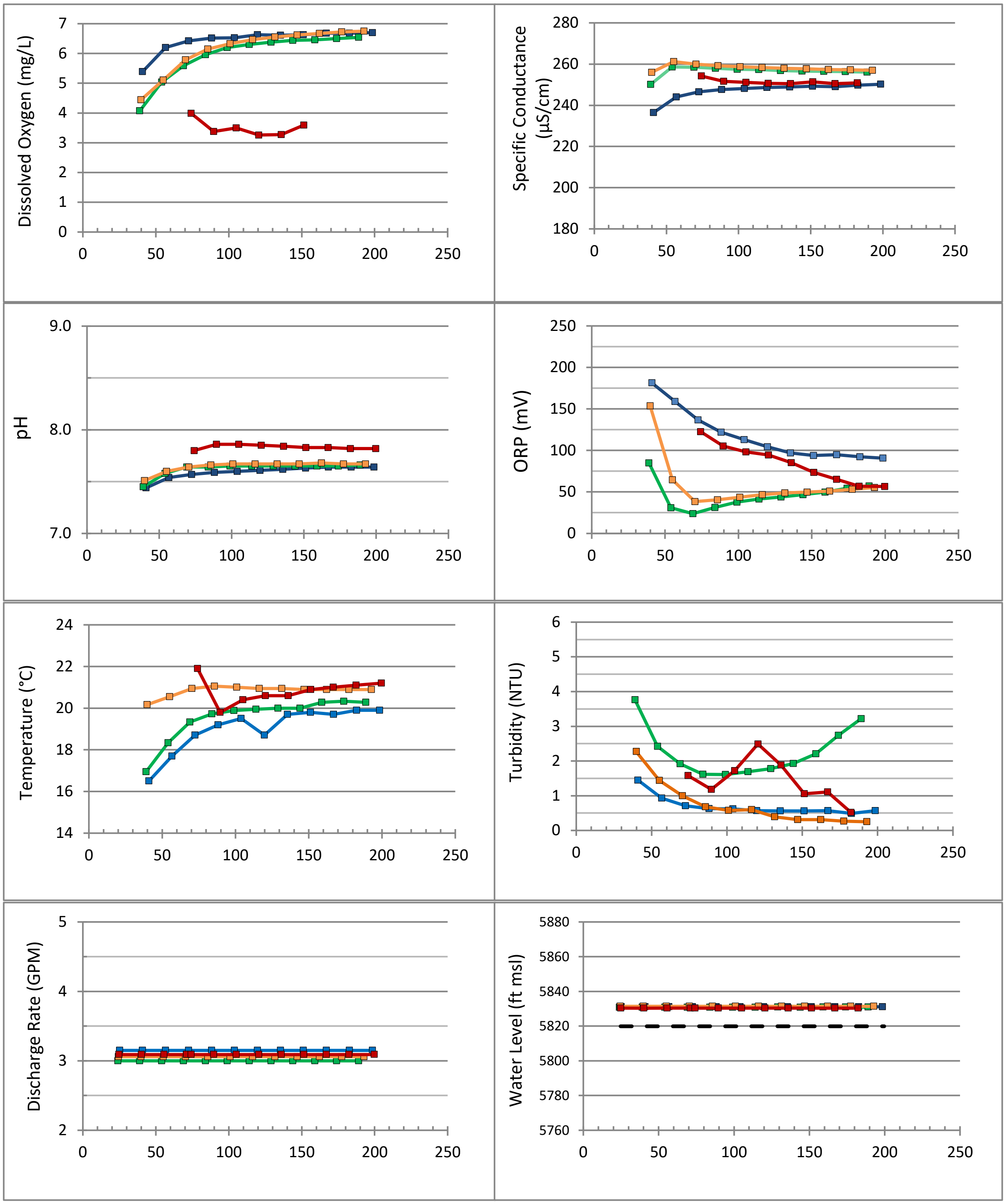
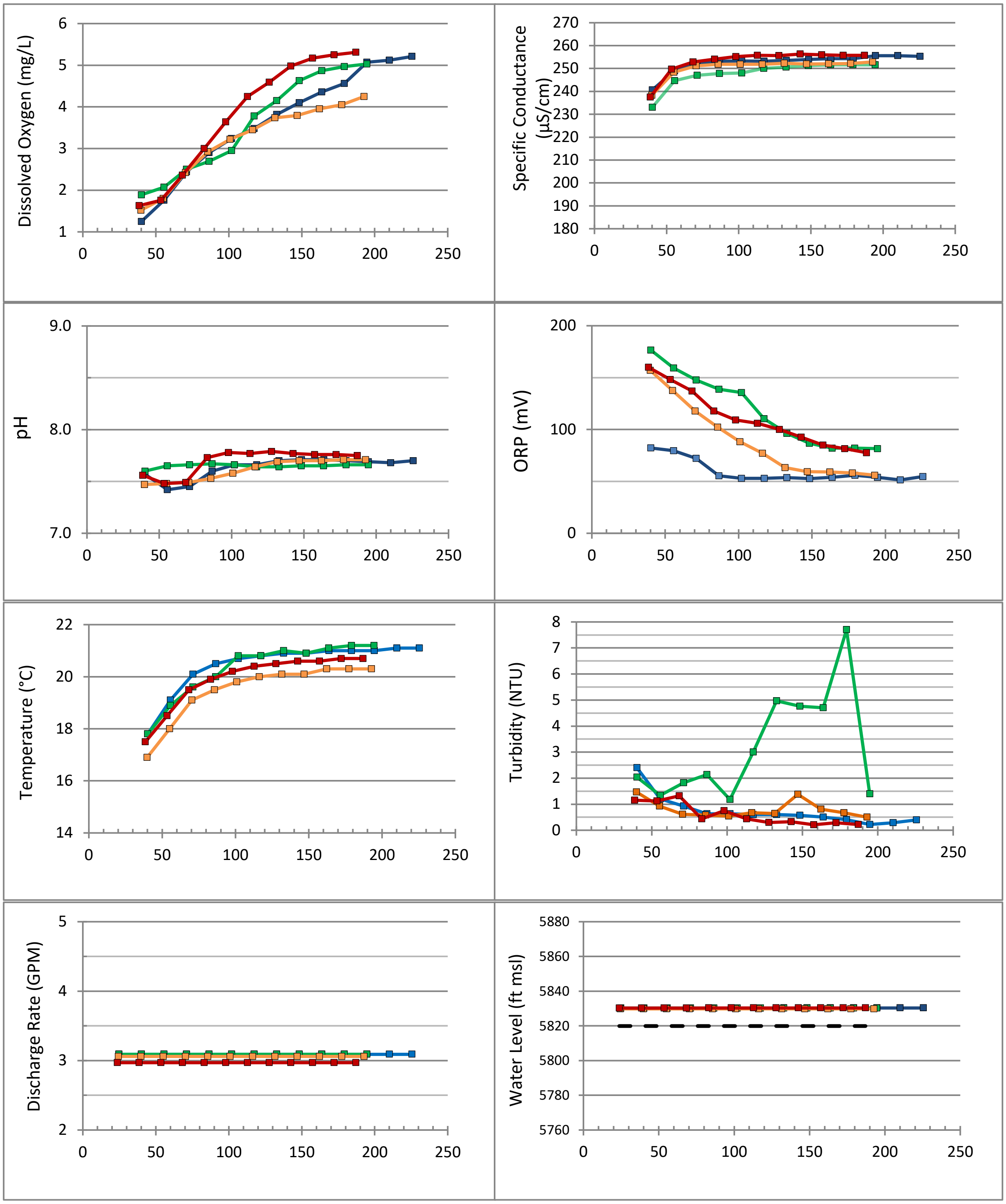


x-axis = cumulative volume purged (gals.), including drop pipe

01/21/20 02/20/20 03/12/20 06/25/20 scr top

** June data starts after YSI was recalibrated

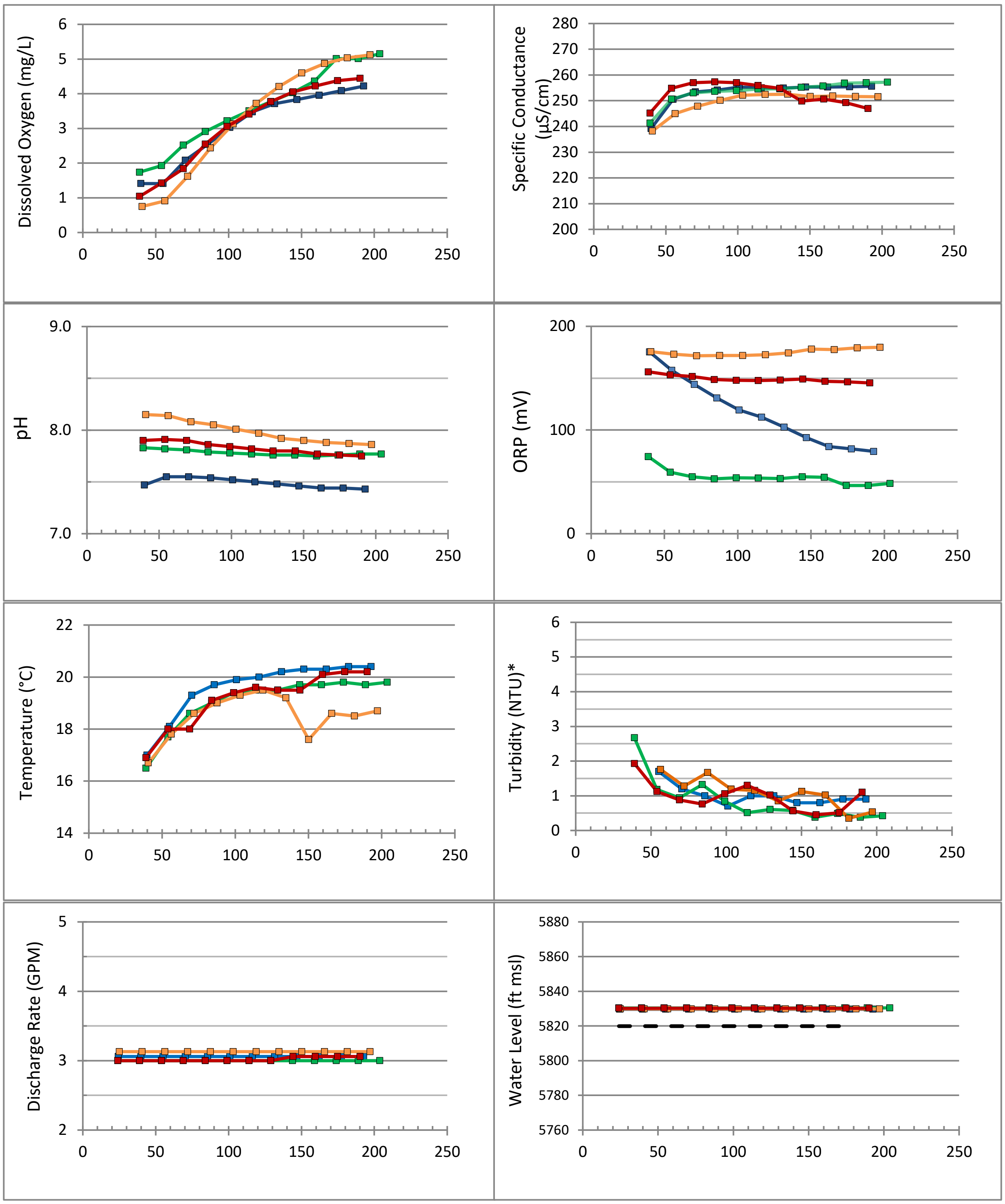
R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **53**	3CV+DP= **197.39**	DP= **38.39**
		Watershed: Mortandad	**Installed:** 1/15/09



x-axis = cumulative volume purged (gals.), including drop pipe

07/29/20 | 08/11/20 | 09/10/20 | 10/08/20 | scr top

R-44 S1	Top of Screen: **895** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **52.86**	3CV+DP= **196.96**	DP= **38.39**
		Watershed: Mortandad	**Installed:** 1/15/09



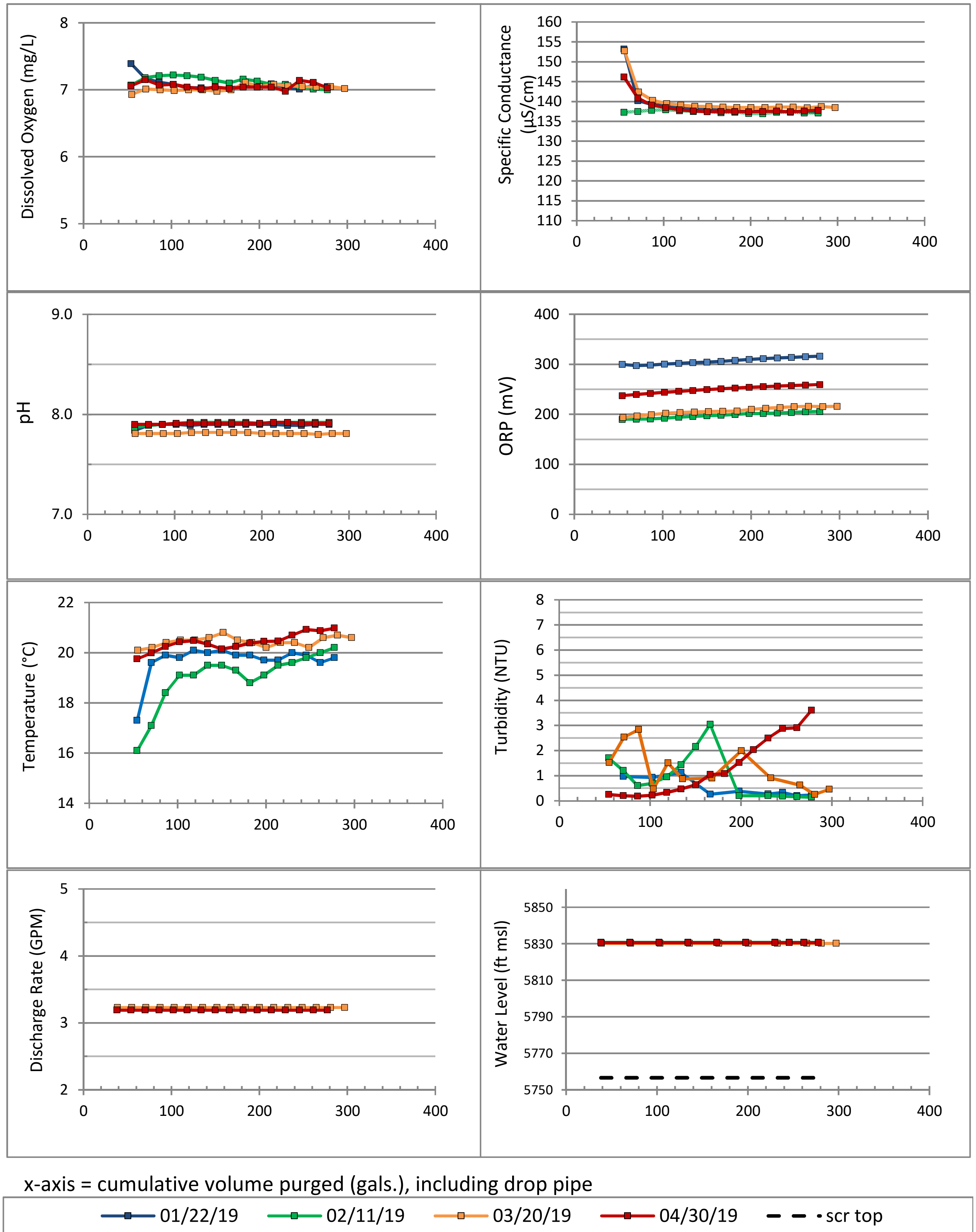
x-axis = cumulative volume purged (gals.), including drop pipe

11/17/20 12/14/20 02/01/21 02/25/21 scr top

*Due to effervescence interference, only HACH data plotted

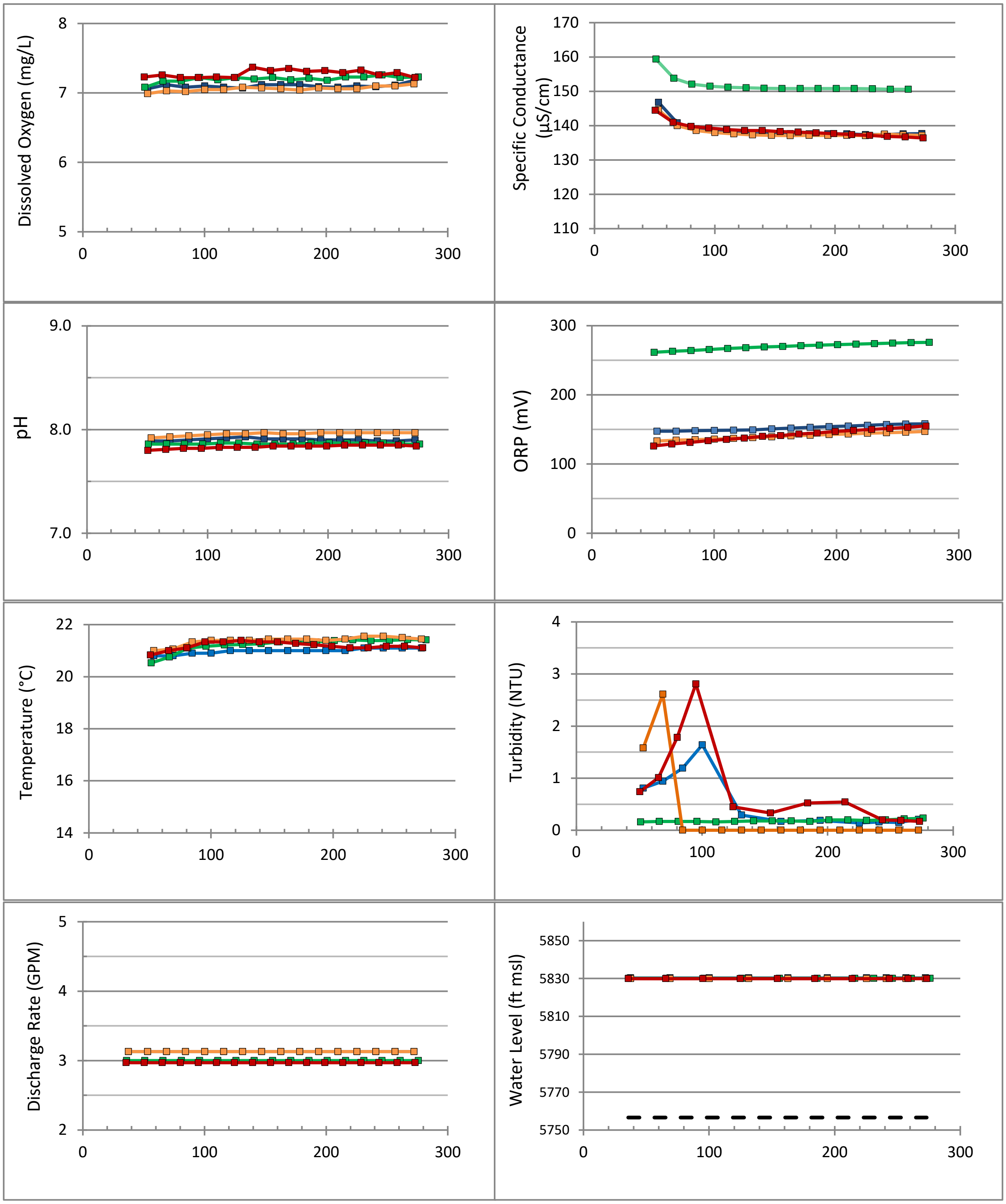
8/2/2019

R-44 S2	Top of Screen: **958.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.86**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/19



x-axis = cumulative volume purged (gals.), including drop pipe

R-44 S2	Top of Screen: **958.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.86**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/19

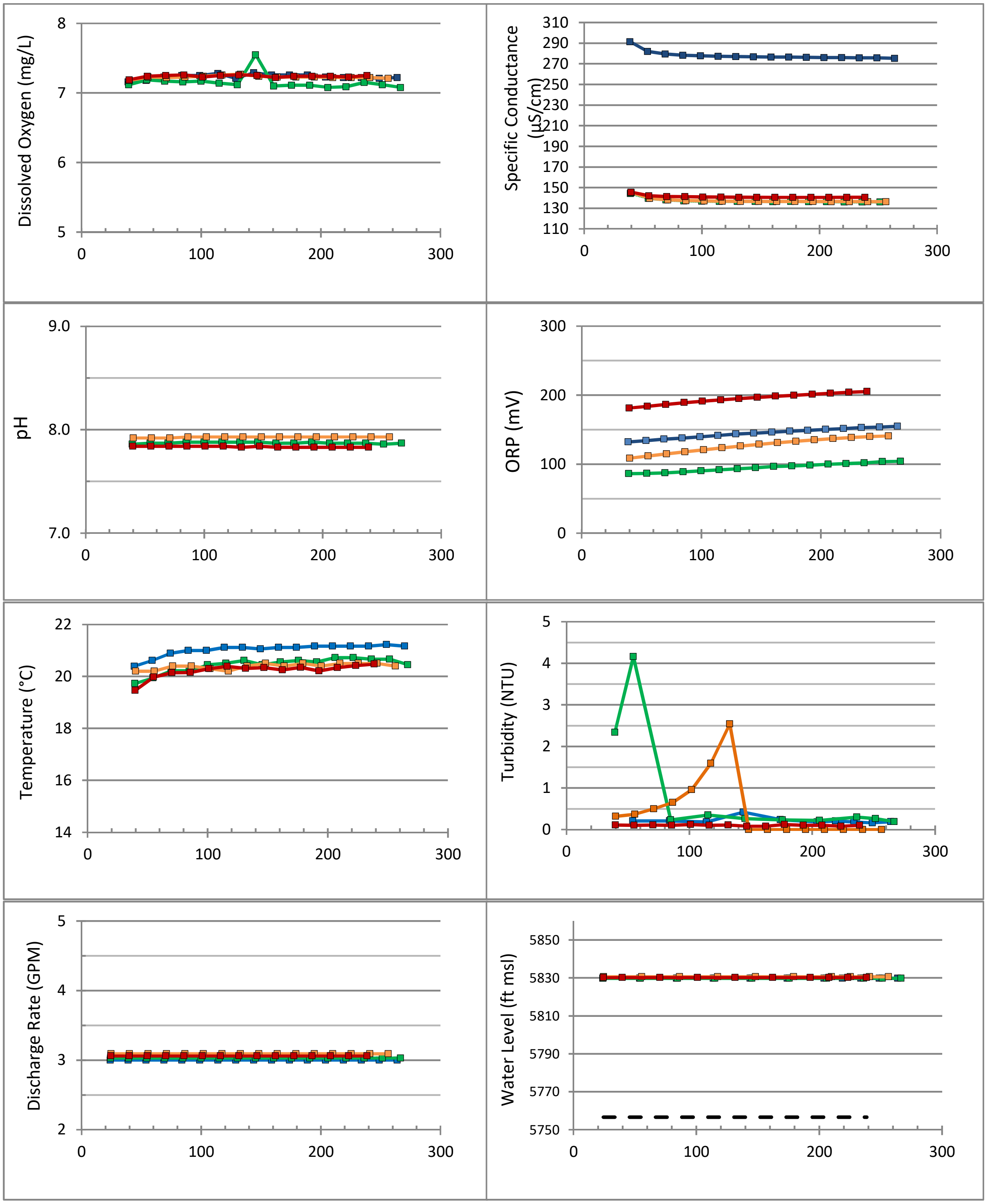


x-axis = cumulative volume purged (gals.), including drop pipe

05/15/19 | 06/18/19 | 07/16/19 | 08/27/19 | scr top

5/21/2020

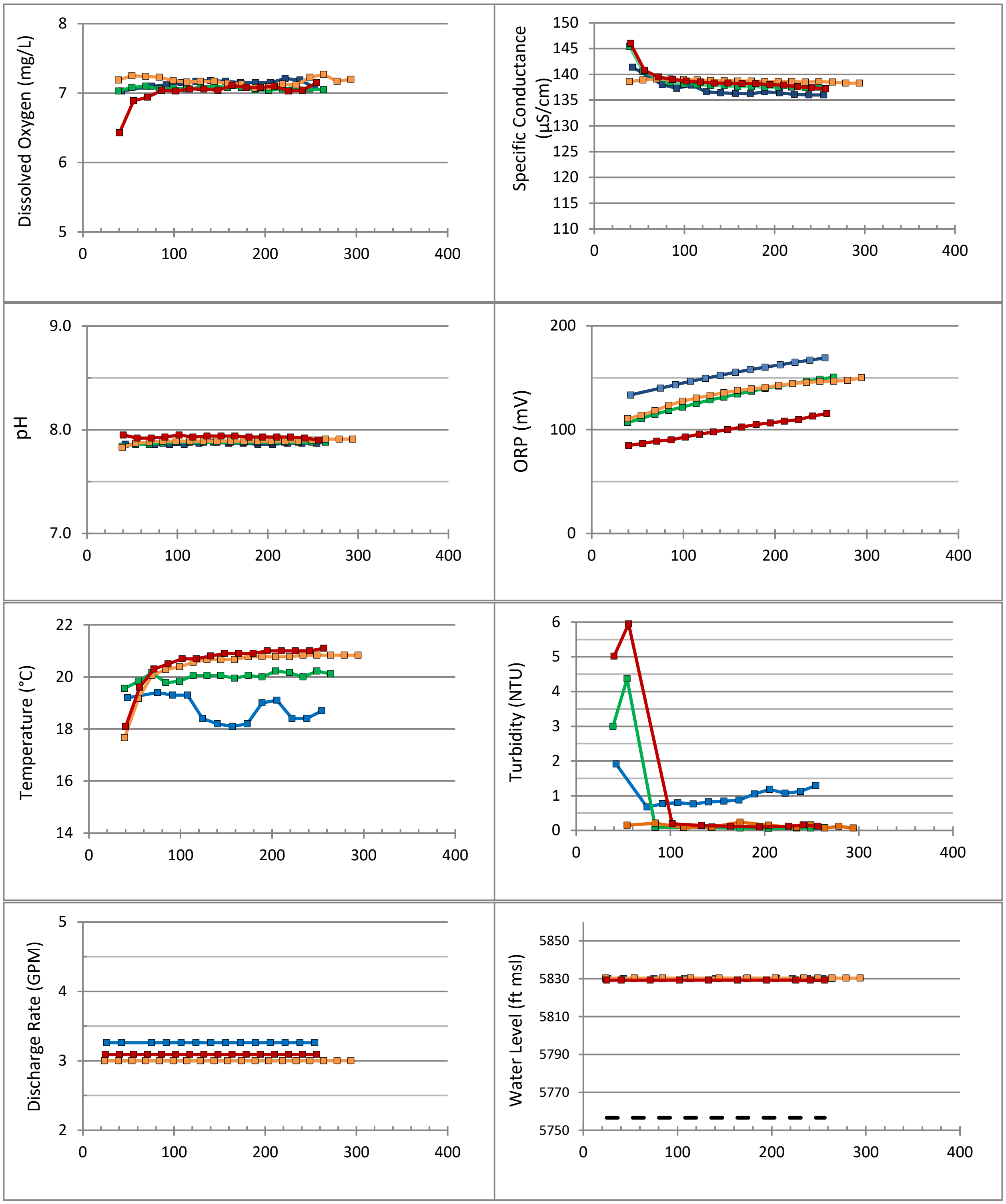
R-44 S2	Top of Screen: **985.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.85**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/19



x-axis = cumulative volume purged (gals.), including drop pipe

09/12/19 10/11/19 11/20/19 12/10/19 scr top

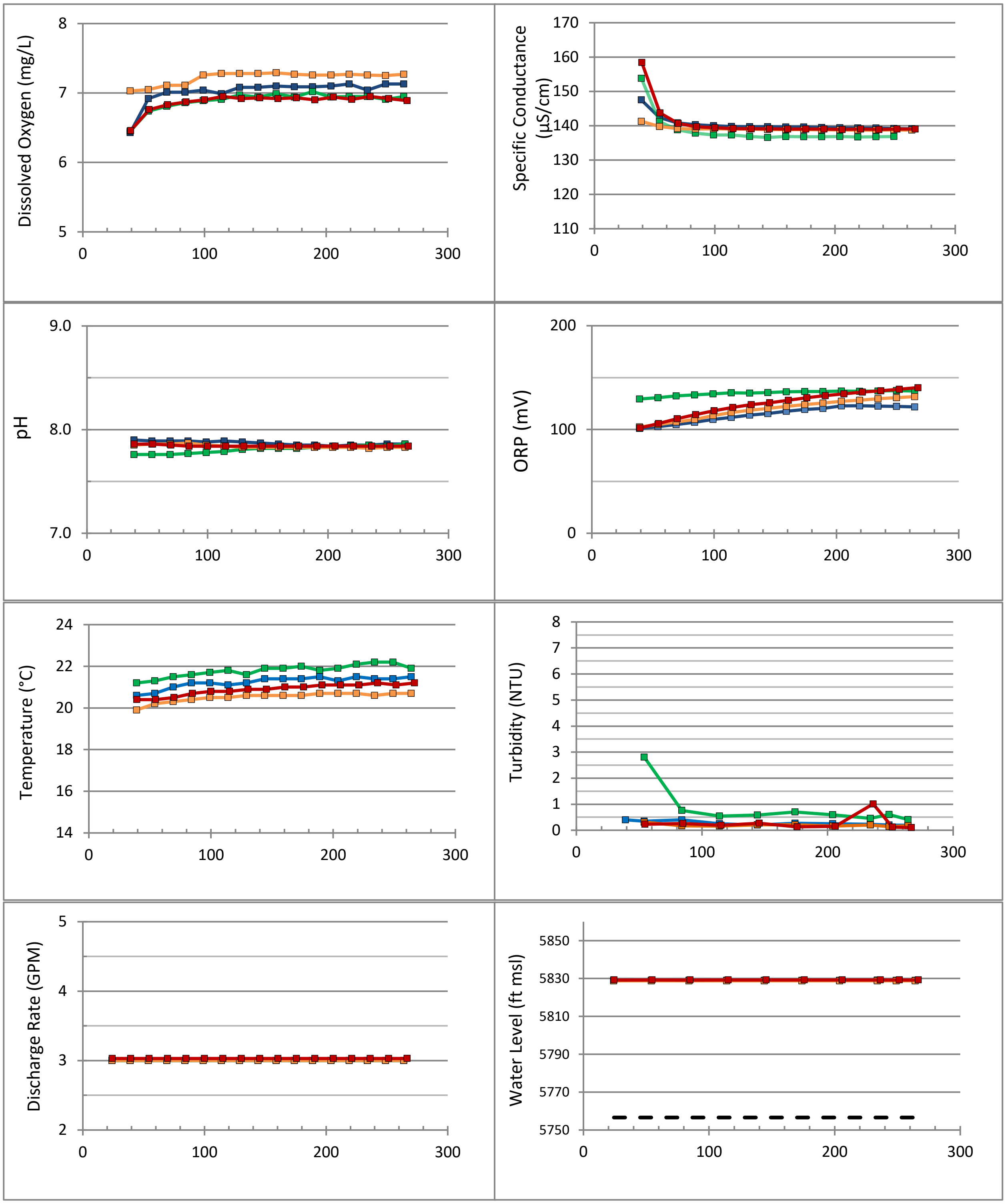
R-44 S2	Top of Screen: **958.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.85**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/19



x-axis = cumulative volume purged (gals.), including drop pipe

01/21/20 02/20/20 03/12/20 06/26/20 scr top

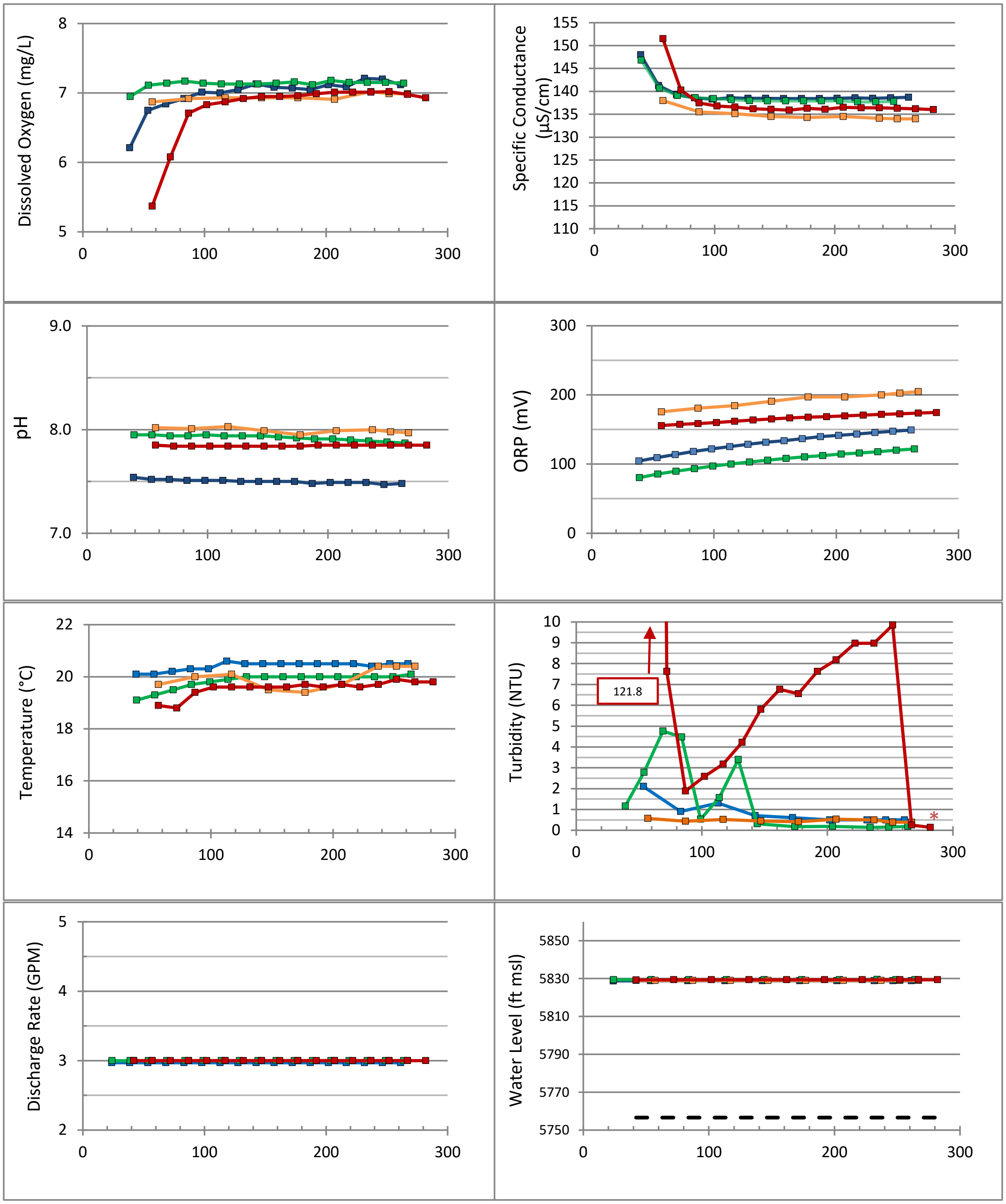
R-44 S2	Top of Screen: **985.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.86**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/09



x-axis = cumulative volume purged (gals.), including drop pipe

07/29/20 08/11/20 09/10/20 10/08/20 scr top

R-44 S2	Top of Screen: **985.3** ft bgs		Formation: **Tpf**
Volumes (gal) (approx):	1CV= **76.4**	3CV+DP= **270.85**	DP= **41.66**
		Watershed: Mortandad	**Installed:** 1/15/09



x-axis = cumulative volume purged (gals.), including drop pipe

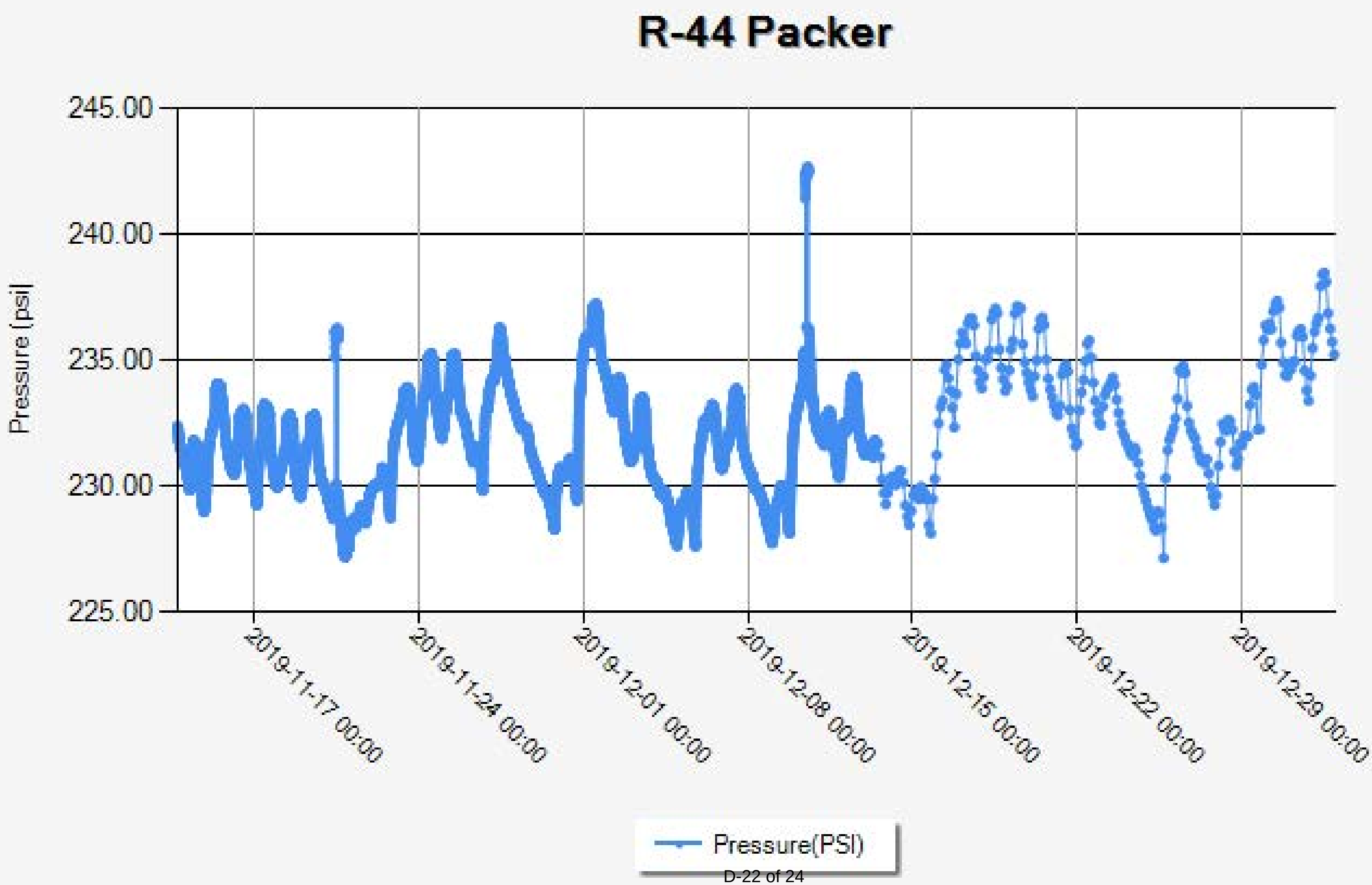
* = Switched to HACH turbidimeter

11/17/20 12/14/20 02/01/21 02/25/21 scr top

R44 Packer 201-04479-3

Range Start: 2019-01-01 Range End: 2020-01-01 Refresh

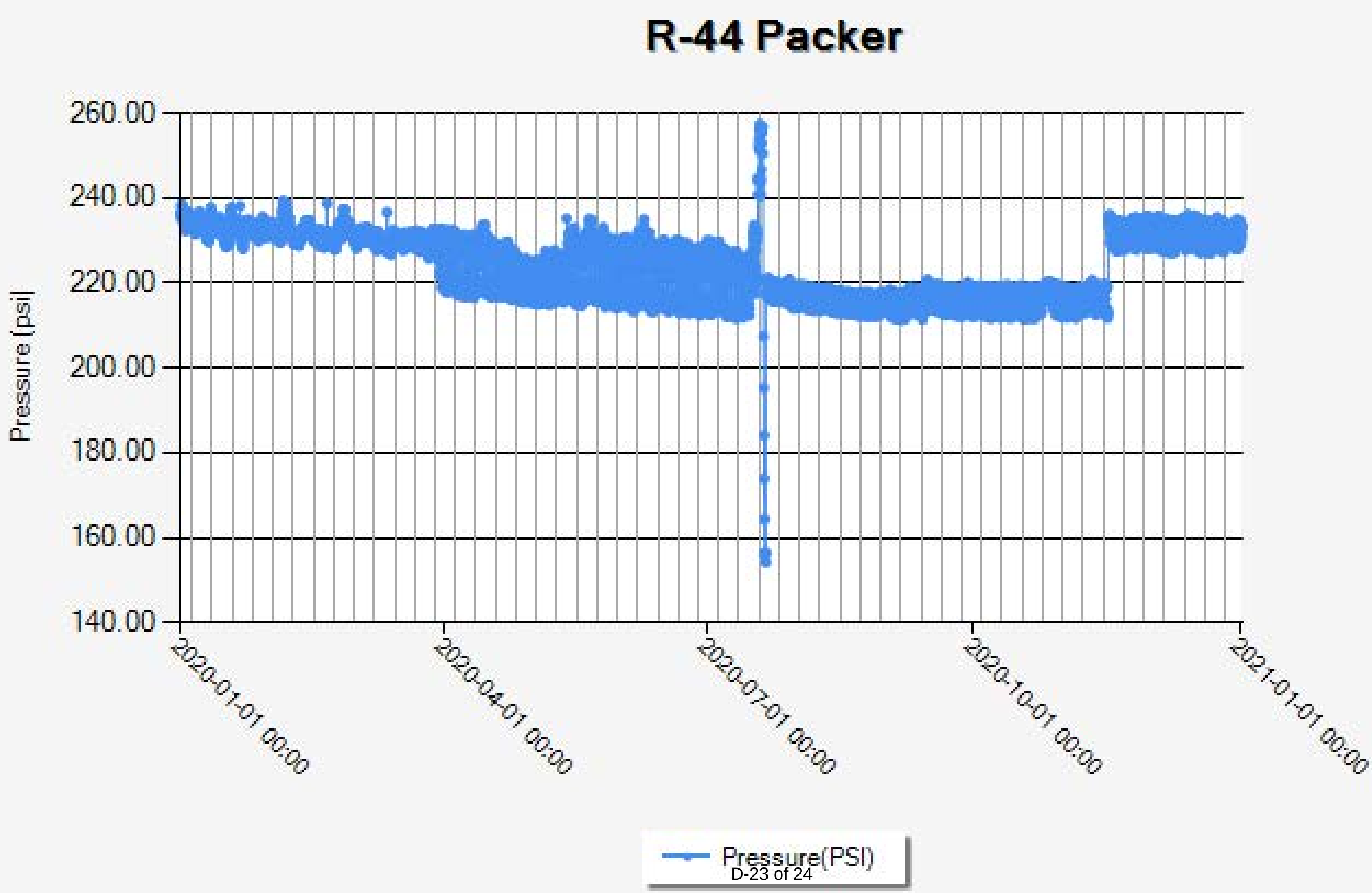
Switch to: Detail Range View Graph View



R44 Packer 201-04479-3

Range Start: 2020-01-01 Range End: 2021-01-01 Refresh

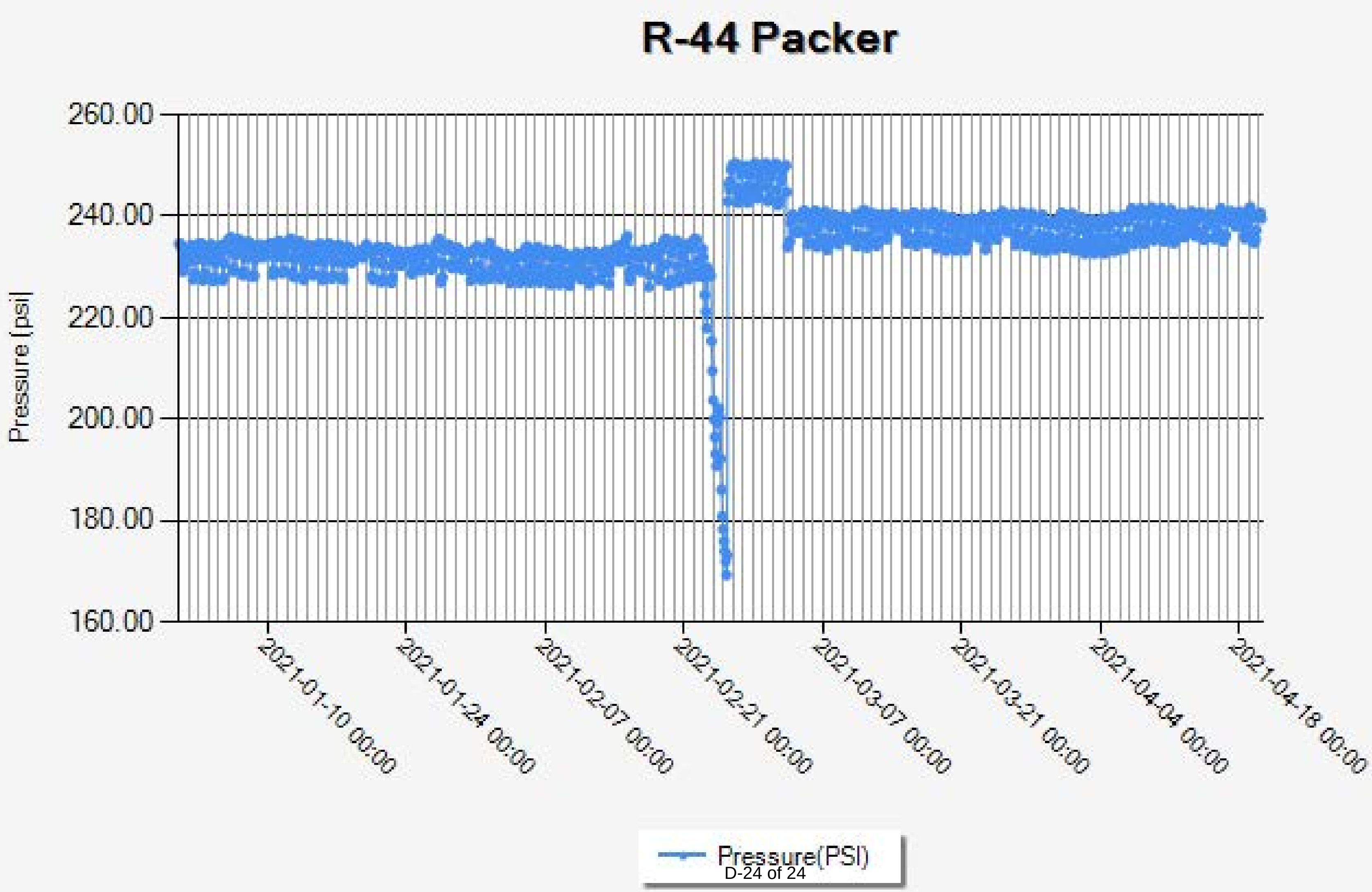
Switch to: Detail Range View Graph View



R44 Packer 201-04479-3

Range Start: 2021-01-01 Range End: 2021-04-20 Refresh

Switch to: **Detail Range View** **Graph View**



Appendix E

Well Completion Report

Los Alamos
NATIONAL LABORATORY
EST. 1943
Environmental Programs
P.O. Box 1663, MS M991
Los Alamos, New Mexico 87545
(505) 606-2337/FAX (505) 665-1812

National Nuclear Security Administration
Los Alamos Site Office, MS A316
Environmental Restoration Program
Los Alamos, New Mexico 87544
(505) 667-4255/FAX (505) 606-2132

Date: May 30, 2009
Refer To: EP2009-0254

James P. Bearzi, Bureau Chief
Hazardous Waste Bureau
New Mexico Environment Department
2905 Rodeo Park Drive East, Building 1
Santa Fe, NM 87505-6303

Subject: Submittal of the Completion Report for Regional Aquifer Well R-44

Dear Mr. Bearzi:

Enclosed please find two hard copies with electronic files of the Completion Report for Regional Aquifer Well R-44.

If you have any questions, please contact Mark Everett at (505) 667-5931 (meverett@lanl.gov) or Nancy Werdel at (505) 665-3619 (nwerdel@doeal.gov).

Sincerely,

Michael J. Graham, Associate Director
Environmental Programs
Los Alamos National Laboratory

Sincerely,

David R. Gregory, Project Director
Environmental Operations
Los Alamos Site Office

MG/DG/PH/ME/SW:sm

Enclosures: Two hard copies with electronic files – Completion Report for Regional Aquifer Well R-44 (LA-UR-09-3066)

Cy: (w/enc.)
Neil Weber, San Ildefonso Pueblo
Nancy Werdel, DOE-LASO, MS A316
Mark Everett, EP-LWSP, MS M992
RPF, MS M707 (with two CDs)
Public Reading Room, MS M992

Cy: (Letter and CD only)
Laurie King, EPA Region 6, Dallas, TX
Steve Yanicak, NMED-OB, White Rock, NM
Steve White, EP-LWSP, MS T005
Kristine Smeltz, EP-WES, MS M992
EP-LWSP File, MS M992

Cy: (w/o enc.)
Tom Skibitski, NMED-OB, Santa Fe, NM
Keyana DeAguero, DOE-LASO (date-stamped letter emailed)
Michael J. Graham, ADEP, MS M991
Alison M. Dorries, EP-WES, MS M992
Paul Huber, EP-LWSP, MS M992
IRM-RMMSO, MS A150 (date-stamped letter emailed)

LA-UR-09-3066
May 2009
EP2009-0254

Completion Report for Regional Aquifer Well R-44

Los Alamos NATIONAL LABORATORY
EST.1943

Prepared by the Environmental Programs Directorate

LA-UR-09-3066
EP2009-0254

Completion Report for Regional Aquifer Well R-44

May 2009

Responsible project leader:

Mark Everett	Mark Everett	Project Leader	Environmental Programs	5-26-09
Printed Name	Signature	Title	Organization	Date

Responsible LANS representative:

Michael Graham	M Graham	Associate Director	Environmental Programs	5/27/09
Printed Name	Signature	Title	Organization	Date

Responsible DOE representative:

David R. Gregory	David R Gregory	Project Director	DOE-LASO	5/28/09
Printed Name	Signature	Title	Organization	Date

EXECUTIVE SUMMARY

This well completion report describes the drilling, installation, development, and aquifer testing of Los Alamos National Laboratory's regional aquifer well R-44, which is located in a tributary of Mortandad Canyon, Technical Area 05 (TA-05) in Los Alamos County, New Mexico. This report was written in accordance with the requirements in Section IV.A.3.e.iv of the March 1, 2005, Compliance Order on Consent. The well was installed at the direction of the New Mexico Environment Department (NMED) to monitor groundwater quality and contaminant movement and to define the southern limit of chromium contamination in the vicinity of well R-28 (which has consistently shown elevated concentrations of chromium in the regional aquifer at the Laboratory). The well will also be used to monitor water levels within the regional aquifer and measure pumping effects from nearby water supply wells.

The R-44 borehole was drilled using dual-rotary air-drilling methods. Fluid additives used included potable water and foam. Foam-assisted drilling was used only in the vadose zone and ceased approximately 100 ft above the regional aquifer; no drilling-fluid additives other than small amounts of potable water were used in the regional aquifer. Additive-free drilling provides minimal impacts to the groundwater and the formation. The R-44 borehole was successfully completed to total depth using dual-rotary casing-advance and open-hole drilling methods.

A retractable 16-in. casing was advanced through the Bandelier Tuff, Guaje Pumice Bed and basaltic volcaniclastic sediments to a depth of 345.8 ft bgs. A 15-in. open borehole was advanced with fluid-assisted air-rotary methods with a downhole hammer bit through the Cerros del Rio basalt and into the Puye Formation to a depth of 765 ft bgs. Then 12-in. casing was advanced with an 11-5/8-in. tricone bit through the remainder of the Puye Formation, through Miocene pumiceous sediments, and through Santa Fe Group Miocene riverine gravels to a total depth of 1094 ft bgs.

Well R-44 was completed as a dual-screen well to evaluate water quality and measure water levels at two discrete depth intervals within the regional aquifer. Well screens will be separated by a packer, as part of the permanent dedicated sampling system, to ensure isolation of each groundwater bearing zone. The upper 10-ft long screened interval has the top of the screen set at 895 ft bgs and the lower 10-ft long screened interval has the top of the screen set at 985.3 ft bgs. Both screen intervals are within the Puye Formation. The composite depth to water after well installation and well development was 879.1 ft bgs.

The well was completed in accordance with an NMED-approved well design and was developed and met target water-quality parameters. Hydrogeologic testing indicated that monitoring well R-44 is highly productive and will perform effectively to meet the planned objectives. Water-level transducers will be placed in the upper and lower well screens in the R-44 well, and groundwater sampling will be performed as part of the facility-wide groundwater-monitoring program.

CONTENTS

Figures

Tables

Appendixes

Acronyms and Abbreviations

µS/cm	microsiemens per centimeter
amsl	above mean sea level
BETCO	barometric and Earth tide correction
bgs	below ground surface
CNL	Compensated Neutron Log
Consent Order	Compliance Order on Consent
cu	capture unit
DO	dissolved oxygen
ECS	Elemental Capture Sonde
EES-14	Earth and Environmental Sciences Group
ENV-MAQ	Environmental Division–Meteorology and Air Quality Group

EP	Environmental Programs
gAPI	American Petroleum Institute gamma ray
GR	gamma ray
HNGS	Hostile Natural Gamma Spectroscopy
IC	ion chromatography
ICPMS	inductively coupled (argon) plasma mass spectrometry
ICPOES	inductively coupled (argon) plasma mass spectrometry
ID	identification
I.D.	inside diameter
ICPMS	inductively coupled (argon) plasma mass spectrometry
ICPOES	inductively coupled (argon) plasma optical emission spectroscopy
LANL	Los Alamos National Laboratory
lbf	pound force
MDA	material disposal area
mV	millivolt
NMED	New Mexico Environment Department
NTU	nephelometric turbidity unit
O.D.	outside diameter
ORP	oxidation-reduction potential
PVC	polyvinyl chloride
RPF	Records Processing Facility
SOP	standard operating procedure
SVOC	semivolatile organic compound
TA	technical area
TD	total depth
TLD	Triple Detector Litho-Density
TOC	total organic carbon
VOC	volatile organic compound
WCSF	waste characterization strategy form
WES-EDA	Waste and Environmental Services Division–Environmental Data and Analysis

1.0 INTRODUCTION

This completion report summarizes the site preparation, drilling, well construction, well development, and aquifer testing for regional aquifer well R-44. The report is written in accordance with the requirements in Section IV.A.3.e.iv of the March 1, 2005, Compliance Order on Consent (the Consent Order). Well R-44 was drilled from November 10, 2008 to December 8, 2008, and the well was completed from December 13, 2009, to January 15, 2009, at Los Alamos National Laboratory (LANL or the Laboratory) for the Environmental Programs (EP) Directorate Water Stewardship Program.

The R-44 project site is located in a small tributary of Mortandad Canyon in the vicinity of regional well R-13 within Technical Area 05 (TA-05), Los Alamos County, New Mexico (Figure 1.0-1). The purposes of the R-44 monitoring well are to monitor potential releases of contaminants from Mortandad and Sandia Canyon sources, assess the conceptual model for contaminant fate and transport of known chromium contamination in the vicinity of well R-28, monitor water levels within the regional aquifer, and measure pumping effects from water-supply well PM-5 and other wells in the vicinity.

The primary objective of the drilling activities at R-44 was to drill and install a dual-screen regional aquifer monitoring well in the uppermost part of the regional groundwater system. The two-screen approach was designed to determine the vertical extent of potential chromium contamination so that pathways and potential future impacts to regional groundwater may be assessed. Water-level transducers will be placed in upper and lower well screens to evaluate hydraulic connections between this monitoring well, other monitoring wells and nearby water-supply well PM-5. Secondary objectives were to collect drill-cutting samples, conduct borehole geophysical logging, and investigate potential perched groundwater zones.

The R-44 borehole was drilled to a total depth (TD) of 1094.0 ft below ground surface (bgs). A monitoring well was installed with two screens. Currently, a temporary packer is being used to isolate the two well screens until the permanent sampling system that is being built by an off-site contractor can be installed. The permanent sampling system will isolate the two screens with a packer when installed in the near future. The upper 10-ft long screened interval is between 895.0 and 905.0 ft bgs and the lower 10-ft long screened interval is between 985.3 and 995.2 ft bgs. The composite depth to water after well installation and well development was 879.1 ft bgs on December 9, 2008. Cuttings samples were collected at 5-ft intervals in the borehole from ground surface to TD. Post installation activities included well development, aquifer testing, surface completion, and a geodetic survey. Future activities include dedicated sampling system installation, site restoration, and waste management.

The information presented in this report was compiled from field reports and daily activity summaries. Records, including field reports, field logs, and survey information, are on file at the Laboratory's Records Processing Facility (RPF). This report contains brief descriptions of activities and supporting figures, tables, and appendixes completed to date associated with the R-44 project.

2.0 PRELIMINARY ACTIVITIES

Preliminary activities included preparing administrative planning documents and preparing the drill pad. All preparatory activities were completed in accordance with Laboratory policies and procedures and regulatory requirements.

2.1 Administrative Preparation

The following documents helped guide the implementation of the scope of work for well R-44: "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45" (TerranearPMC 2008, 105083); "Integrated Work Document for Regional and Intermediate Aquifer Well Drilling" (LANL 2007, 100972); "Storm Water Pollution Prevention Plan Addendum" (LANL 2006, 092600); and "Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling" (LANL 2008, 103916).

2.2 Site Preparation

Site preparation was performed by LANL staff several weeks prior to rig mobilization. Between November 8 and 9, 2008, activities included mobilizing the drill rig, air compressors, trailers, and support vehicles to the drill site and staging alternative drilling tools and construction materials at the Pajarito Road lay down yard.

Office supply trailers, generators, and general field equipment were moved on-site after mobilization of drilling equipment. Potable water was obtained from the Puye Road fire hydrant and a fire hydrant near the Los Alamos County landfill on East Jemez Road. Safety barriers and signs were installed around the borehole-cuttings containment pit and along the perimeter of the work area.

3.0 DRILLING ACTIVITIES

This section describes the drilling strategy and approach and provides a chronological summary of field activities conducted at monitoring well R-44.

3.1 Drilling Approach

The drilling methodology and selection of equipment, including drill casing sizes, for R-44 were designed to retain the ability to case off perched groundwater and ensure reaching TD with a sufficiently sized casing to allow well installation with the required 2-in. minimum annular filter pack thickness for a 5.56-in.-outside diameter (O.D.) well. It was anticipated that if perched groundwater was encountered at R-44, the perched zone would be isolated and sealed off either with casing or by cementing to avoid commingling perched groundwater with the regional aquifer.

Dual-rotary drilling methods using a Foremost DR-24HD drill rig were employed to drill the R-44 borehole. Dual-rotary drilling has the advantage of simultaneously advancing and casing the borehole. The Foremost DR-24HD drill rig was equipped with conventional drilling rods, tricone bits, downhole hammer bits, one deck-mounted 900 ft^3/min air compressor, and general drilling equipment. Auxiliary equipment included two Sullair 1150 ft^3/min trailer-mounted air compressors. Two sizes of A53 grade B flush-welded mild carbon-steel casing (16-in. and 12-in. inside-diameter [I.D.]) were used for the R-44 project. The dual-rotary technique used filtered compressed air and fluid-assisted air to evacuate cuttings from the borehole. Cuttings samples were collected at 5-ft intervals in the borehole from ground surface to TD to characterize the hydrostratigraphy of rock units encountered in the borehole.

Drilling fluids, other than air, used in the vadose zone included municipal water and a mixture of municipal water with Baroid AQF-2 foaming agent. The fluids were used to cool the bit and help lift cuttings from the borehole. Use of foaming agents was terminated at 780 ft bgs, approximately 100 ft above the predicted regional aquifer water table. No additives other than municipal water were used for drilling within the regional aquifer. Total amounts of drilling fluids introduced into the borehole and those recovered are recorded and presented in Table 3.1-1.

3.2 Chronology of Drilling Activities

Mobilization of drilling equipment and supplies to the R-44 site occurred during November 8 and 9, 2008. The borehole was initiated the next day, at 0145 hours using dual-rotary methods with 16-in. casing and a 15-in. tri-cone, long-tooth carbide bit. After drilling and advancing 16-in. casing through the alluvium, Bandelier Tuff, and volcaniclastic sediments overlying the Cerros del Rio basalt, the 16-in. casing was landed at 345.8 ft bgs in the morning of November 12, 2008. In conjunction with preparation to start open-hole drilling the top-head drive developed a minor hydraulic leak which required replacing a seal.

Open-hole drilling resumed using a 15-in. hammer bit at the top of Cerros del Rio basalt at 1807 h on November 13, 2008. Drilling progressed smoothly through the basalt to the contact with underlying Puye Formation sediments, at 707.0 ft bgs (at 0400 h on November 16, 2008). Progress was slowed because of problems with one of the two auxiliary Sulair air compressors. However, open-hole drilling was eventually suspended at 765.0 ft bgs (within the Puye Formation sediments) due to borehole instability.

On November 17, 2008 the 16-in. casing was cut in order to detach the welded drive shoe at 344.0 ft bgs, prior to running the Laboratory's video and geophysical (gamma ray and induction) logging tools in the borehole. The video tool revealed water entering the borehole at 739 ft bgs. Two groundwater samples were collected from this depth using a bailer; however the exact depth of this water was suspect because there were "knots" in the logging wire line. Because of the lack of water at this depth during later monitoring, and based on chemical analysis of these samples, it was decided that the water was most likely drilling water and not perched groundwater.

The drive shoe and 20-ft sections of 12-in. casing were welded and installed in the hole from November 18 through 24, 2008. Several mechanical and hydraulic problems with the top-head drive required ordering replacement parts and servicing, which slowed progress. On December 2, 2008, dual-rotary drilling commenced with the 12-in. casing and an 11-5/8 in. tricone bit. Drilling was unusually slow in the soft Puye Formation sediments, and the bit was pulled on December 3, 2008, for inspection. Several carbide buttons were observed to be absent from the bit cones, but of particular note was the presence of heavy score marks on the bit just above the cones. The score marks were indicative of the bit cutting into the steel drive shoe due to the bit not being able to be advanced outside the bottom of the 12-in. casing. All signs pointed to a misalignment somewhere toward the bottom of the 12-in. casing string. The decision to remove the entire 12-in. casing string for inspection was determined to be imperative.

Once on the surface, the bottom 12-in. casing joint was found to be slightly bent and the drive shoe weld showed some cracking. A new drive shoe was then welded on to a new lead casing joint on December 3, 2008, and the 12-in. casing string was reinstalled in the borehole. Drilling with dual-rotary methods and 12-in. casing recommenced on December 6, 2008. Eleven groundwater samples were collected, by air-lifting, from the 920 to 1094 ft bgs interval on December 7 and 8, 2008. Significant water production in the borehole was noted beginning at about 990 ft bgs and some indication of formation heaving occurred at 1094 ft bgs. A total depth of 1094 ft bgs was reached on December 8, 2008 at 1735 h. The next day the drill string was tripped out of the hole in preparation for geophysical logging by Schlumberger on December 9, 2008. After logging concluded, a stable depth-to-water of 879.1 ft bgs was recorded the same day.

Twelve-in. (74.0 ft casing and shoe) and 16-in. (1.8 ft casing and shoe) drill casing were left in the borehole. The longer length of 12-in. casing was left in place to help control heaving. The 12-in. casing stub was buried in backfill and isolated by the lowermost bentonite seal and the 16 in. casing stub was set in bentonite to avoid unwanted impacts in future.

Before moving the drilling rig off the site, the 12-in. casing was cut on December 10, 2008, at 1020.0 ft bgs. The rig was moved off site early in the morning of December 11, 2008, to the next drilling location (R-46).

The field crews typically worked two 12-h shifts per day (24-h operation) and 7 d/wk. Daily activities progressed without weather delays throughout the duration of drilling. Only minor mechanical delays with the 12-in. casing shoe, top-head drive, and air compressors slowed drilling progress.

4.0 SAMPLING ACTIVITIES

This section describes the cuttings and groundwater sampling activities at well R-44. All sampling activities were conducted in accordance with applicable quality procedures.

4.1 Cuttings Sampling

Cuttings samples were collected from the R-44 borehole at 5-ft intervals from ground surface to the TD of 1094.0 ft bgs. At each interval, approximately 500 mL of bulk cuttings were collected from the discharge hose, placed in resealable plastic bags, labeled, and archived in core boxes. Sieved fractions (>#10 and >#35 mesh) were also collected from ground surface to bottom depth and placed in chip trays along with unsieved (whole rock) cuttings. Radiation control technicians screened cuttings before removal from the site. The core boxes and chip trays were delivered to the Laboratory's archive at the conclusion of drilling activities. All screening measurements were within the range of background values.

Drilling and sample collection methods used at R-44 did not retain a majority of the fine fraction (silt and clay) of the drill cuttings, and much of the fine material throughout the borehole was lost. The volume of compressed air and water required for circulation made catching samples difficult, and fines were selectively lost during sample collection. Site geologists manually collected samples with a wire mesh basket directly from the discharge hose, and discharge velocities commonly forced the fine fraction of sample through the basket. Recovery of the coarser fraction of the cuttings samples was successful in nearly 100% of the borehole. The borehole lithologic log for R-44 stratigraphy is summarized in section 5.1 and detailed in Appendix A.

4.2 Water Sampling

Groundwater-screening samples were collected from the drilling discharge hose at approximate 20-ft intervals starting at 739 ft bgs to evaluate a potential perched zone (see discussion in section 3.2) and continued through the top of the regional aquifer to the borehole's TD of 1094.0 ft bgs. Typically, upon reaching the bottom of a 20-ft run of casing, the driller would stop water circulation (if injecting water) and circulate air, and as the discharge cleared; a water sample was collected directly from the discharge hose. Not all depth intervals below the top of the regional groundwater table could be captured at the end of each casing run. Alternatively, some water samples were collected upon start-up of the next casing run after the borehole equilibrated. Refer to Table 4.2-1 for a summary of screening samples collected at well R-44.

Eleven groundwater-screening samples, from depths of 739.0 to 1094.0 ft bgs, were collected during drilling operations by bailing or air-lifting water samples through the drill string. Two of these samples represented waters collected while drilling through the vadose zone to evaluate the presence or absence of perched groundwater. Drilling screening samples were analyzed for anions and metals, and one sample was analyzed for tritium.

Four regional groundwater-screening samples were collected during well development; two from the upper screen interval (895–905 ft bgs) and two from the lower screen interval (985.3–995.2 ft bgs). Development screening samples were analyzed for anions, metals, and total organic carbon (TOC).

Twelve regional groundwater-screening samples were collected at regular intervals (approximately one sample per 4 h) during aquifer testing. Six of these screening samples were collected from the upper screen interval (895–905 ft bgs), and six samples were collected from the lower screen interval (985.3–995.2 ft bgs). The groundwater samples were collected from a stainless-steel riser pipe that was connected to the surface discharge line from the submersible pump. Aquifer-testing screening samples were analyzed for dissolved anions, metals and TOC.

Groundwater characterization samples were collected from the completed well in accordance with the Consent Order. The samples were analyzed for the full suite of constituents including radioactive elements; anions/cations; general inorganic chemicals; volatile and semi-volatile organic compounds; and stable isotopes of hydrogen, nitrogen, and oxygen. These groundwater analytical results will be reported in the annual update to the “Interim Facility-Wide Groundwater Monitoring Plan.”

5.0 GEOLOGY AND HYDROGEOLOGY

A brief description of the geologic and hydrogeologic features encountered at R-44 is presented below. The Laboratory’s geology task leader and site geologists examined cuttings and geophysical logs to determine geologic contacts and hydrogeologic conditions. Drilling observations, video logging, water-level measurements, and geophysical logs were used to characterize groundwater occurrences encountered at R-44.

5.1 Stratigraphy

The stratigraphy for the R-44 borehole is presented below in order of youngest to oldest geologic units. Lithologic descriptions are based on cuttings samples collected from the discharge hose. Cuttings and borehole geophysical logs were used to identify geologic contacts. Figure 5.1-1 illustrates the stratigraphy at R-44. A detailed lithologic log based on analysis of drill cuttings is presented in Appendix A.

Quaternary Alluvium, Qal (0–47 ft bgs)

Quaternary alluvium, consisting of unconsolidated tuffaceous silty sand to sandy silt with pebble gravels containing pumice and volcanic detritus, occurs from 0 to 47 ft bgs. No evidence of alluvial groundwater was observed.

Unit 1g of the Tshirege Member of the Bandelier Tuff, Qbt 1g (47– 70 ft bgs)

Unit 1g of the Tshirege Member of the Bandelier Tuff was encountered from 47 to 70 ft bgs as interpreted by natural gamma geophysical log analysis. Unit 1g is a poorly welded vitric ash-flow tuff that is pumiceous, generally crystal rich and lithic-poor, with abundant vitric ash matrix. The thin Tshirege Unit 1g section preserved in R-44 contains strongly weathered pumices, minor lithics of diverse volcanic lithologies and abundant quartz and sanidine crystals.

Cerro Toledo Interval, Qct (70–94 ft bgs)

The Cerro Toledo interval, a thin layer of poorly consolidated volcaniclastic sediments that occurs stratigraphically between the Tshirege and Otowi Members of the Bandelier Tuff, is present from 70 to 94 ft bgs based on natural gamma ray geophysical log interpretation. This unit consists of silty fine to medium sands and gravels made up of detrital volcanic materials (dacites, obsidian, rhyodacite), generally weathered pumice fragments, and abundant quartz and sanidine crystal grains.

Otowi Member of the Bandelier Tuff, Qbo (94–296 ft bgs)

The Otowi Member of the Bandelier Tuff is present from 94 to 296 ft bgs as interpreted from natural gamma geophysical log data. The Otowi Member is a poorly welded, pumiceous, locally lithic-rich, ash-flow tuff. Abundant pumice lapilli are white to pale orange, glassy, fibrous-textured and quartz- and sanidine-phyric and are enclosed in a matrix of vitric ash. Locally abundant volcanic lithic fragments, or xenoliths (generally up to 15 mm in diameter), are commonly subangular to subrounded and of intermediate volcanic composition, predominantly gray and light pinkish gray hornblende-and biotite-phyric dacites.

Guaje Pumice Bed of the Otowi Member of the Bandelier Tuff, Qbog (296–313 ft bgs)

The Guaje Pumice Bed occurs from 296 to 313 ft bgs on the basis of natural gamma ray log interpretation. The Guaje is a pumice-rich, lithic- and crystal-poor fall deposit that contains abundant (97%–100% by volume) pristine-appearing vitric, phenocryst-poor pumice fragments and lapilli. Trace volumes of volcanic lithics, quartz and sanidine phenocrysts, and fine ash are present.

Basaltic Volcaniclastic Sediments, Unassigned (313–344 ft bgs)

A thin sedimentary layer of pinkish to orange-tan siltstone to silty fine- to medium-grained sandstone with pebble gravel was intersected from 313 to 344 ft bgs, based on natural gamma log interpretation. Locally abundant subrounded detrital clasts (up to 20 mm in diameter) consist of basalt, basaltic scoria, vitric pumice fragments, dacite, and minor quartzite. These basalt-rich sediments occur at a stratigraphic position regionally occupied by the Puye Formation but have not yet been assigned to a particular unit. Basaltic constituents in these sediments likely were derived from underlying Cerros del Rio basalt lavas.

Cerros del Rio Basalt, Tb4 (344–707 bgs)

The Cerros del Rio basalt, intersected from 344 to 707 ft bgs, is locally a sequence of basalt lava flows with interlayers of cinders and basaltic ejecta, and pumiceous and basaltic sediments, some of which suggest a possible hydromagmatic origin. The upper part of the Cerros del Rio section, from 344 to 505 ft bgs, is made up of three distinct clinopyroxene (cpx)-phyric and olivine-cpx basalt flows, each with a layer of cinders/ejecta at its base. Cuttings suggest that a basaltic tuff layer containing basalt cinders, glassy scoria, dacite, weathered pumice, minor quartzite and fragments of indurated volcaniclastic sandstone, from 505 to 535 ft bgs, may indicate a hydromagmatic event between effusive lava eruptions. A similar sequence of three olivine-bearing basalt flows, with intercalated thin sedimentary deposits containing basalt and pumice detritus, makes up the lower part of the Cerros del Rio section between 535 and 707 ft bgs.

Puye Formation, Tpf (707–1005 ft bgs)

Puye Formation volcaniclastic sediments encountered from 707 to 1005 ft bgs consist of texturally diverse, gray, grayish brown and pinkish tan, poorly sorted, fine to coarse gravels, gravelly sandstones and silty sandstones with gravel. Detrital constituents that make up these sediments are generally subangular to subrounded and represent a range of volcanic lithologies including olivine-basalt (present as detrital clasts mainly at the top of the section), abundant biotite- and hornblende-dacites (present as a major constituent in large volumes throughout the section), rhyodacite, weathered pumice, scoria and dark colored vitrophyre.

Miocene Pumiceous Sediments, Tjfp (1005–1088 ft bgs)

A section of pumice-rich volcaniclastic sediments occur from 1005 to 1088 ft bgs. These deposits are made up of fine- to coarse-grained sandstones with pebble gravels, locally with a silty matrix. White, glassy, phenocryst-poor detrital pumices generally make up a large percent (locally as much as 100% by volume) of granule and pebble-size clasts. Additional constituents include abundant subangular to subrounded dacites, lesser amounts of basalt and andesite, and locally trace occurrences of Precambrian quartzite.

Miocene Riverine Sediments, Tcar (1088–1094 ft bgs)

A brief interval of fine to coarse gravels with fine- to coarse-grained sandstones, representing axial-river deposits, was encountered from 1088 ft bgs to the total borehole TD of 1094 ft bgs. These distinctive sediments are characterized by rounded to well-rounded pebbles and coarser gravel clasts composed of diverse volcanic litholgies (i.e., dark colored fine-grained andesites, varieties of dacite and rhyolite) and locally abundant (up to 30% by volume) Precambrian granites and quartzites.

5.2 Groundwater

Possible groundwater was first encountered at approximately 739 ft bgs in the Puye Formation sediments on January 24, 2009. As discussed in section 6.1, video log interpretation and later water-level measurements suggested that this was water introduced during drilling (see Appendix B). Perched water was not present in R-44. After the well was drilled to final depth of 1094 ft bgs, the water level was measured at approximately 879.1 ft bgs in the borehole.

Groundwater-screening samples were collected during drilling, well development, and aquifer testing as discussed in section 4.2 and presented in Table 4.2-1. Groundwater chemistry and field water-quality parameters are discussed in Appendix B. Aquifer testing data and analysis are discussed in Appendix C.

6.0 BOREHOLE LOGGING

Several video logs and a limited suite of geophysical logs were collected during the R-44 drilling project using Laboratory-owned equipment. An additional suite of cased-hole geophysical logs was collected by Schlumberger Wireline Services. A summary of video and geophysical logging runs is presented in Table 6.0-1.

6.1 Video Logging

A video log was run in the uncased borehole to check for the presence of perched groundwater on November 17, 2008. Water was observed in the video log in the Puye Formation sediments at a depth of 739 ft bgs when the borehole was at 765-ft depth. However interpretation of the log indicated the actual depth of the observed water was uncertain because there was a “knot” in the wire line. The November 17, 2008, video log from the borehole is presented on a digital video disc as part of Appendix D included with this document. Table 6.0-1 provides details about the video logging run.

6.2 Geophysical Logging

A suite of Schlumberger geophysical logs was run inside the drill casing on December 9, 2008. At the time of logging, the terminations of the two casing strings in the borehole were located at the following depths: 16-in. casing at 344 ft bgs and the 12-in. casing at 1094 ft bgs. The geophysical suite included natural gamma ray, Triple Litho-Density (TLD), Elemental Capture Sonde (ECS), and Compensated Neutron Log (CNL). Interpretation and details of the logging are presented on CD as part of Appendix E.

7.0 WELL INSTALLATION

R-44 well was installed between December 13, 2008, and January 15, 2009.

7.1 Well Design

The R-44 well was designed in accordance with the approved Drilling Work Plan. NMED approved the well design before installation. The well was designed with dual-screened intervals to monitor groundwater quality at two depths in the upper part of the regional aquifer within Puye Formation sediments.

7.2 Well Construction

The R-44 monitoring well was constructed of 5.0-in.-I.D./5.56-in.-O.D., type A304 stainless-steel beveled casing fabricated to American Society for Testing and Materials A312 standards. The two screened sections utilized 10-ft lengths of 5.0-in.-I.D. rod-based 0.020-in. wire-wrapped well screen. Welding, using compatible stainless-steel welding rods, was used to join all individual casing and screen sections. All casing and screens were steam and pressure washed on-site before installation. A 2-in. I.D. steel threaded/coupled tremie pipe string (decontaminated prior to use) was utilized for delivery of backfill and annular fill materials during well construction. The placement of annular materials typically had two components: installing materials, and retracting the drill casing and raising the tremie pipe. As each section of drill casing was cut off the string, it was picked up and laid down. During this part of the process, the well casing was hung under full tension on a wireline while the drill casing was supported by a ring and slips.

Two screened intervals were chosen for the R-44 well design, based on monitored water levels and indications of potentially productive full-saturation intervals in the Schlumberger geophysical logs. The lower nominal 10-ft long screened interval had the top of the screen set at 985.3 ft bgs, and the upper nominal 10-ft long screened interval had the top of the screen set at 895 ft bgs. A 20.8-ft stainless-steel sump was placed below the bottom of the lower well screen. Stainless-steel centralizers (four sets of four) were welded to the well casing approximately 2.1 ft above and below each screen. A Pulstar work-over rig was used for well construction activities. Figure 7.2-1 presents an as-built schematic showing construction details for the completed well.

Well construction materials were moved onto the R-44 site starting on December 11, 2008. The Pulstar rig was moved on location and decontamination of the stainless-steel well casing and screens took place the next day. Before running the well casing, 41 ft^3 of 10/20 silica sand was added to the borehole as backfill bringing the borehole bottom to 1024.7 ft bgs, which is roughly 5 ft below the 12-in. casing cut.

On December 13 the well casing was installed. Each joint was welded as it went into the borehole, using careful welding techniques and covering the borehole to avoid slag falling into the annular void. After hanging the well at 1016 ft bgs the process of installing annular materials began. Additional 10/20 silica sand (5.5 ft^3) was added to bring the top of the backfill to 1008.4 ft bgs. A lower bentonite seal composed of ¼-in. pellets (1.3 ft^3) followed by ⅜-in. chips (0.7 ft^3) was placed from 999.8 to 1008.4 ft bgs. The lower screen 10/20 silica sand filter pack was then installed, and surged to promote compaction, from 980.2–1008.4 ft bgs. This was capped by a finer 20/40 silica sand transition from 976.3 to 980.2 ft bgs on December 22, 2008. All fieldwork was suspended that day due to the Laboratory holiday shut-down and R-44 well construction recommenced on January 5, 2009, after the break.

A seal separating the two screened intervals was placed from 910.2–976.3 ft bgs and consisted of ¼-in. bentonite pellets (13.4 ft^3) followed by ⅜-in. bentonite chips (45.2 ft^3). The upper screen filter pack of 10/20 silica sand was then installed (and surged) from 890.3 to 910.2 ft bgs. The upper filter pack was then capped with a transition 20/40 silica sand from 887.6 to 890.3 ft bgs.

The well's upper bentonite seal (⅜-in. chips) was installed from 342.4 to 887.6 ft bgs from January 8 to January 12, 2009. A surface seal (mix of 97–98 wt% Portland cement with 2–3 wt% bentonite) was placed above the upper bentonite seal from 3–342.4 ft bgs; this marked well construction completion on January 15, 2009 (1030 h). Table 7.2-1 details volumes of all materials used during well construction.

8.0 POSTINSTALLATION ACTIVITIES

Following installation, the well was developed and aquifer pumping tests were performed. Total groundwater purged during well development and aquifer testing was 92,929 gal. The wellhead and surface pad was constructed and a geodetic survey performed. A dedicated dual-zone sampling system will be installed after receipt from the manufacturer. Site restoration activities will be completed following the final disposition of contained drill cuttings and groundwater, per the NMED-approved waste-decision trees.

8.1 Well Development

Well development was conducted between January 15 and January 20, 2009. Initially, the screened interval was bailed and swabbed to remove formation fines in the filter pack and well sump. Bailing and swabbing continued until water clarity visibly improved. Final development was accomplished using a submersible pump. The swabbing tool was a 4.5-in.-O.D. 1-in.-thick nylon disc attached to a weighted steel rod. The swabbing tool was lowered by wireline and drawn repeatedly in both directions across each screened interval. After bailing and swabbing, a 10-hp, 4-in.-Grundfos submersible pump was installed in the well for the final stage of well development. Approximately 16,005 gal. of groundwater was purged at R-44 during well development activities.

During the pumping stage of well development, turbidity, temperature, pH, dissolved oxygen (DO), oxygen-reduction potential (ORP), and specific conductance parameters were measured. In addition, water samples for TOC analysis were collected. The required values for TOC and turbidity to determine adequate well development are less than 2.0 ppm and less than 5 nephelometric turbidity units (NTUs), respectively.

A discussion of water removed during well development, field water-quality parameters, and analytical results for samples collected during development is summarized below in section 8.1.1 and detailed in Table B.1.2-1 of Appendix B.

8.1.1 Well Development Field Parameters

Field parameters were measured at well R-44 by collecting aliquots of groundwater from the discharge pipe without the use of a flow-through cell, allowing the samples to be exposed to the atmosphere. Results are provided here and in greater detail in Appendix B. This condition probably resulted in a slight variation of field parameters during well development and during the pumping test, most notably, temperature, pH, and DO.

Measurements of pH varied from 8.22 to 8.30 in the upper screened interval and from 8.19 to 8.29 in the lower screened interval. Measurements of temperature varied from 18.3°C to 18.56°C in the upper screened interval and from 17.47°C to 18.78°C in the lower screened interval. Concentrations of DO varied from 9.70 to 10.66 mg/L in the upper screened interval and from 11.57 to 13.72 mg/L in the lower screened interval. Uncorrected ORP measurements varied from –135.1 to–129.7 millivolts (mV) in the upper screened interval and from –130.8 to –118.9 mV in the lower screened interval. These negative, uncorrected ORP values are not reliable and representative of known relatively oxidizing conditions characteristic of the regional aquifer beneath the Pajarito Plateau. Specific conductance ranged from 142 to 148 microsiemens per centimeter (μS/cm) in the upper screened interval and from 193 to 204 μS/cm in the lower screened interval. Values of turbidity measured at R-44 ranged from 0.0 to 0.1 NTU for the nonfiltered groundwater samples of the upper screen and from 0.0 to 55.8 NTUs for the lower screen samples.

8.2 Aquifer Testing

Aquifer pumping tests were conducted at R-44 from February 14 to February 17, 2009. Several short-duration tests with short-duration recovery periods were performed on the upper and lower screens in the well. A 24-h test followed by a 24-h recovery period completed the testing. The same 10-hp Grundfos pump used during well development was used to perform the aquifer tests. Approximately 76,924 gal. of groundwater was purged during aquifer testing activities.

During aquifer testing, turbidity, temperature, pH, DO, ORP, and specific conductance parameters were measured. In addition, water samples for TOC analysis were collected.

A discussion of water removed during well development, field water-quality parameters, and analytical results for samples collected during development is summarized below in section 8.2.1 and detailed in Table B.1.2-1 of Appendix B. Results of the R-44 aquifer test are presented in Appendix C.

8.2.1 Aquifer Testing Field Parameters

Measurements of pH varied from 7.80 to 8.04 in the upper screened interval and 8.31 to 8.67 in the lower screened interval at R-44. Measurements of temperature varied from 14.99°C to 19.08°C in the upper screened interval and 15.14°C to 20.31°C in the lower screened interval. Concentrations of DO varied from 7.95 to 9.30 mg/L in the upper screened interval and from 8.60 to 11.14 mg/L in the lower screened interval. Uncorrected ORP measurements varied from 117.3 to 204.4 mV in the upper screened interval and from 117.3 to 195.2 mV in the lower screened interval. The uncorrected ORP measurements are in general agreement with the DO values, suggesting that relatively oxidizing conditions were established during the aquifer performance testing at well R-44. Specific conductance ranged from 60 to 140 μS/cm in the upper screened interval and 173 to 154 μS/cm in the lower screened interval. Values of turbidity for the nonfiltered groundwater samples ranged from 0 to 2.8 NTUs in the upper screened interval and 1.6 to 5.9 NTUs in the lower screened interval.

8.3 Dedicated Sampling System Installation

A dedicated sampling system for the R-44 well was custom-designed based on the hydrogeologic data gathered during the aquifer tests. The sampling system is on order from the manufacturer and will be installed upon delivery. The system consists of Baski Inc.-designed stainless-steel plumbing and an inflatable isolation packer. The system will implement a shrouded 4-in, Grundfos submersible pump (environmentally retrofitted with Teflon) with a 4-in., 3-phase, 460-V, viton-fitted Franklin Electric submersible motor. The pump will draw water from discrete intervals via pneumatically actuated access port valves. An inflatable viton-covered packer will be supplied as a component of the dedicated system.

All materials that contact the groundwater will be constructed of stainless steel, Teflon, viton, or polyvinyl chloride (PVC). All components of the pump column will be new. The pump column will be constructed of 1-in. threaded/coupled stainless steel pipe with check valves installed in the pipe string every 200 ft. A weep hole will be installed at the bottom of the uppermost pipe joint to protect the pump column from freezing. To measure water levels in the well, two 1-in. I.D. schedule 80 PVC pipes will be installed to the top of the pump shroud in order to set dedicated transducers below the measured static water levels. The upper PVC transducer tube will be equipped with a 6-in. section of 0.010-in slot screen with a threaded end cap at the bottom of the tube. The lower PVC transducer tube will be equipped with a flexible nylon tube that will extend from a threaded end cap at the bottom of the PVC tube through the isolation packer to measure water levels in the lower screen interval. A weather-resistant pump control box will be installed next to the wellhead.

Post-installation construction and sampling system component installation details for R-44 are presented in Figure 8.3-2a. Figure 8.3-2b presents technical notes.

8.4 Wellhead Completion

A reinforced concrete surface pad, 10 ft × 10 ft × 6 in. thick, was installed at the wellhead. The pad will provide long-term structural integrity for the well. A brass survey pin was embedded in the northwest corner of the pad. A 10-in.-I.D. steel protective casing with a locking lid was installed around the stainless-steel well riser. The concrete pad was slightly elevated above the ground surface and crowned to promote runoff. Base course was graded around the edges of the pad. A total of four bollards, painted yellow for visibility, are set at the outside edges of the pad to protect the well from traffic. All of the four bollards are designed for easy removal to allow access to the well. Details of the wellhead completion are presented in Figure 8.3-1a.

8.5 Geodetic Survey

A New Mexico licensed professional land surveyor conducted a geodetic survey on February 10, 2009 (Table 8.5-1). The survey data collected conforms to Laboratory Information Architecture project standards IA-CB02, “GIS Horizontal Spatial Reference System,” and IA-D802, “Geospatial Positioning Accuracy Standard for A/E/C and Facility Management.” All coordinates are expressed as New Mexico State Plane Coordinate System Central Zone (NAD 83); elevation is expressed in feet above mean sea level (amsl) using the National Geodetic Vertical Datum of 1929. Survey points include ground-surface elevation near the concrete pad, the top of the brass pin in the concrete pad, the top of the well casing, and the top of the protective casing.

8.6 Waste Management and Site Restoration

Waste generated from the R-44 project includes drilling fluids, purged groundwater, decontamination water, drill cuttings, and contact waste. A summary of the waste characterization samples collected from the R-44 well is presented in Table 8.6-1.

All waste streams produced during drilling and development activities were sampled in accordance with “Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling” (LANL 2008, 103916).

Fluids produced during drilling and well development are expected to be land-applied after a review of associated analytical results per the waste characterization strategy form (WCSF) and the EP-Directorate Standard Operating Procedure (SOP) 010.0, Land Application of Groundwater. If it is determined that drilling fluids are nonhazardous but cannot meet the criterion for land application, the drilling fluids will be evaluated for treatment and disposal at one of the Laboratory's six wastewater treatment facilities. If analytical data indicate that the drilling fluids are hazardous/nonradioactive or mixed low-level waste, the drilling fluids will be disposed of at an authorized facility.

Cuttings produced during drilling are anticipated to be land-applied after a review of associated analytical results per the WCSF and ENV-RCRA SOP-011.0, Land Application of Drill Cuttings. If the drill cuttings do not meet the criterion for land application, they will be disposed of at an authorized facility. Decontamination fluid used for cleaning the drill rig and equipment is containerized. The fluid waste was sampled and will be disposed of at an authorized facility. Characterization of contact waste will be based upon acceptable knowledge, pending analyses of the waste samples collected from the drill cuttings, purge water, and decontamination fluid.

Site restoration activities will include removing drilling fluids and cuttings from the pit and managing the fluids and cuttings in accordance with SOP-010.06, removing the polyethylene liner, removing the containment area berms, and backfilling and regrading the containment area, as appropriate.

9.0 DEVIATIONS FROM PLANNED ACTIVITIES

Drilling, sampling, and well construction at R-44 were performed as specified in "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45" (TerranearPMC 2008, 105083).

10.0 ACKNOWLEDGMENTS

Patrick Longmire wrote Appendix B, Groundwater Analytical Results.

Boart Longyear drilled the R-44 borehole and installed the well.

Los Alamos National Laboratory personnel ran downhole video equipment.

Schlumberger Wireline Services performed the final geophysical logging of the borehole.

Right Bit Services and Equipment Repair welded the stainless well screen and casing.

TerranearPMC provided oversight on all preparatory and field-related activities.

11.0 REFERENCES

The following list includes all documents cited in this report. Parenthetical information following each reference provides the author(s), publication date, and ER ID. This information is also included in text citations. ER IDs are assigned by the Environmental Programs Directorate's Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

LANL (Los Alamos National Laboratory), March 2006. "Storm Water Pollution Prevention Plan for SWMUs and AOCs (Sites) and Storm Water Monitoring Plan," Los Alamos National Laboratory document LA-UR-06-1840, Los Alamos, New Mexico. (LANL 2006, 092600)

LANL (Los Alamos National Laboratory), October 4, 2007. "Integrated Work Document for Regional and Intermediate Aquifer Well Drilling (Mobilization, Site Preparation and Setup Stages)," Los Alamos National Laboratory, Los Alamos, New Mexico. (LANL 2007, 100972)

LANL (Los Alamos National Laboratory), November 2007. "Work Plan for Geochemical Characterization and Drilling for Fate and Transport of Contaminants Originating in Sandia Canyon," Los Alamos National Laboratory document LA-UR-07-7579, Los Alamos, New Mexico. (LANL 2007, 099607)

LANL (Los Alamos National Laboratory), October 2008. Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling, Los Alamos, New Mexico. (LANL 2008, 103916)

TerranearPMC, October 2008. "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45," plan prepared for Los Alamos National Laboratory, Los Alamos, New Mexico. (TerranearPMC 2008, 105083)

Map Data Sources for R-42 Completion Report Location Map

Point Feature Locations of the Environmental Restoration Project Database; Los Alamos National Laboratory, Waste and Environmental Services Division, EP2008-0109; February 28, 2008.

Hypsography, 100 and 20 Foot Contour Interval; Los Alamos National Laboratory, ENV Environmental Remediation and Surveillance Program; 1991.

Surface Drainages, 1991; Los Alamos National Laboratory, ENV Environmental Remediation and Surveillance Program, ER2002-0591; 1:24,000 Scale Data; Unknown publication date.

Paved Road Arcs; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Dirt Road Arcs; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Structures; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Technical Area Boundaries; Los Alamos National Laboratory, Site Planning & Project Initiation Group, Infrastructure Planning Division; September 19, 2007.

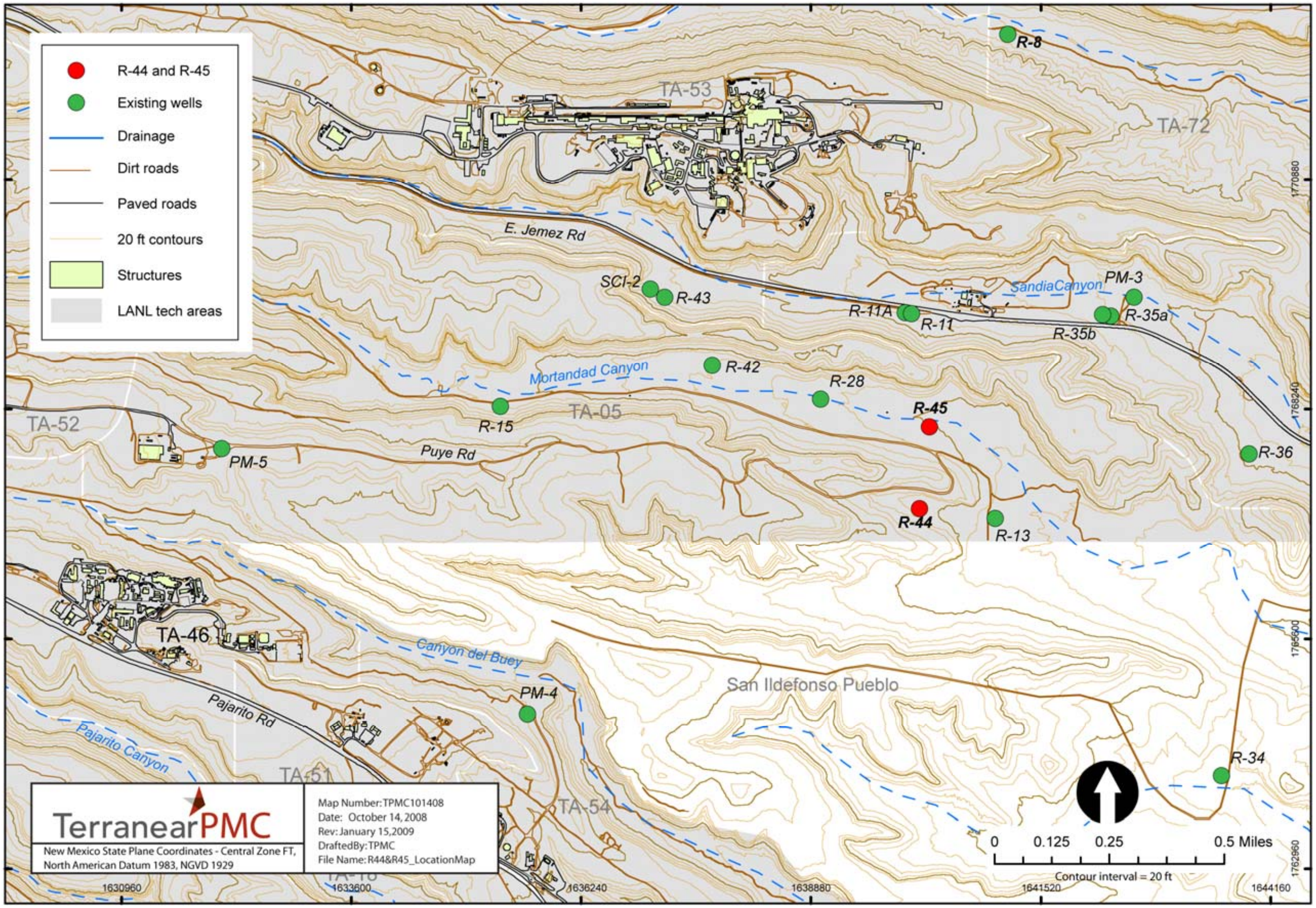
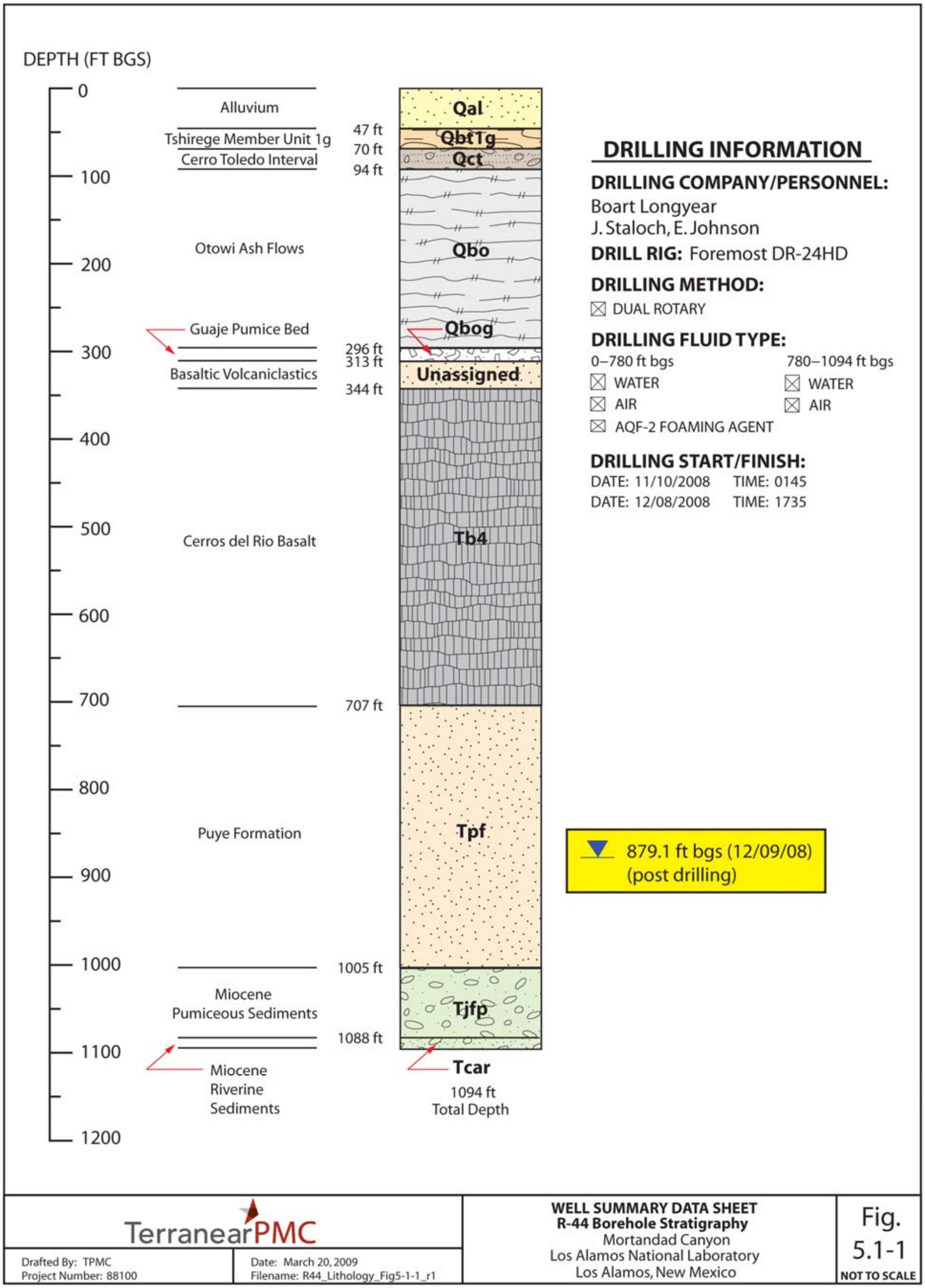


Figure 1.0-1 Regional aquifer well R-44 with respect to surrounding regional wells and PM-5


DEPTH (FT BGS)
0
100
200
300
400
500
600
700
800
900
1000
1100
1200
Alluvium
Tshirege Member Unit 1g
Cerro Toledo Interval
Otowi Ash Flows
Guaje Pumice Bed
Basaltic Volcaniclastics
Cerros del Rio Basalt
Puye Formation
Miocene Pumiceous Sediments
Miocene Riverine Sediments
47 ft
70 ft
94 ft
296 ft
313 ft
344 ft
707 ft
1005 ft
1088 ft
Qal
Qbt1g
Qct
Qbo
Qbog
Unassigned
Tb4
Tpf
Tjfp
Tcar
1094 ft Total Depth
DRILLING INFORMATION
DRILLING COMPANY/PERSONNEL:
Boart Longyear
J. Staloch, E. Johnson
DRILL RIG: Foremost DR-24HD
DRILLING METHOD:
☒ DUAL ROTARY
DRILLING FLUID TYPE:
0–780 ft bgs
☒ WATER
☒ AIR
☒ AQF-2 FOAMING AGENT
780–1094 ft bgs
☒ WATER
☒ AIR
DRILLING START/FINISH:
DATE: 11/10/2008 TIME: 0145
DATE: 12/08/2008 TIME: 1735
879.1 ft bgs (12/09/08) (post drilling)
TerranearPMC
Drafted By: TPMC
Project Number: 88100
Date: March 20, 2009
Filename: R44_Lithology_Fig5-1-1_r1
WELL SUMMARY DATA SHEET
R-44 Borehole Stratigraphy
Mortandad Canyon
Los Alamos National Laboratory
Los Alamos, New Mexico
Fig. 5.1-1
NOT TO SCALE

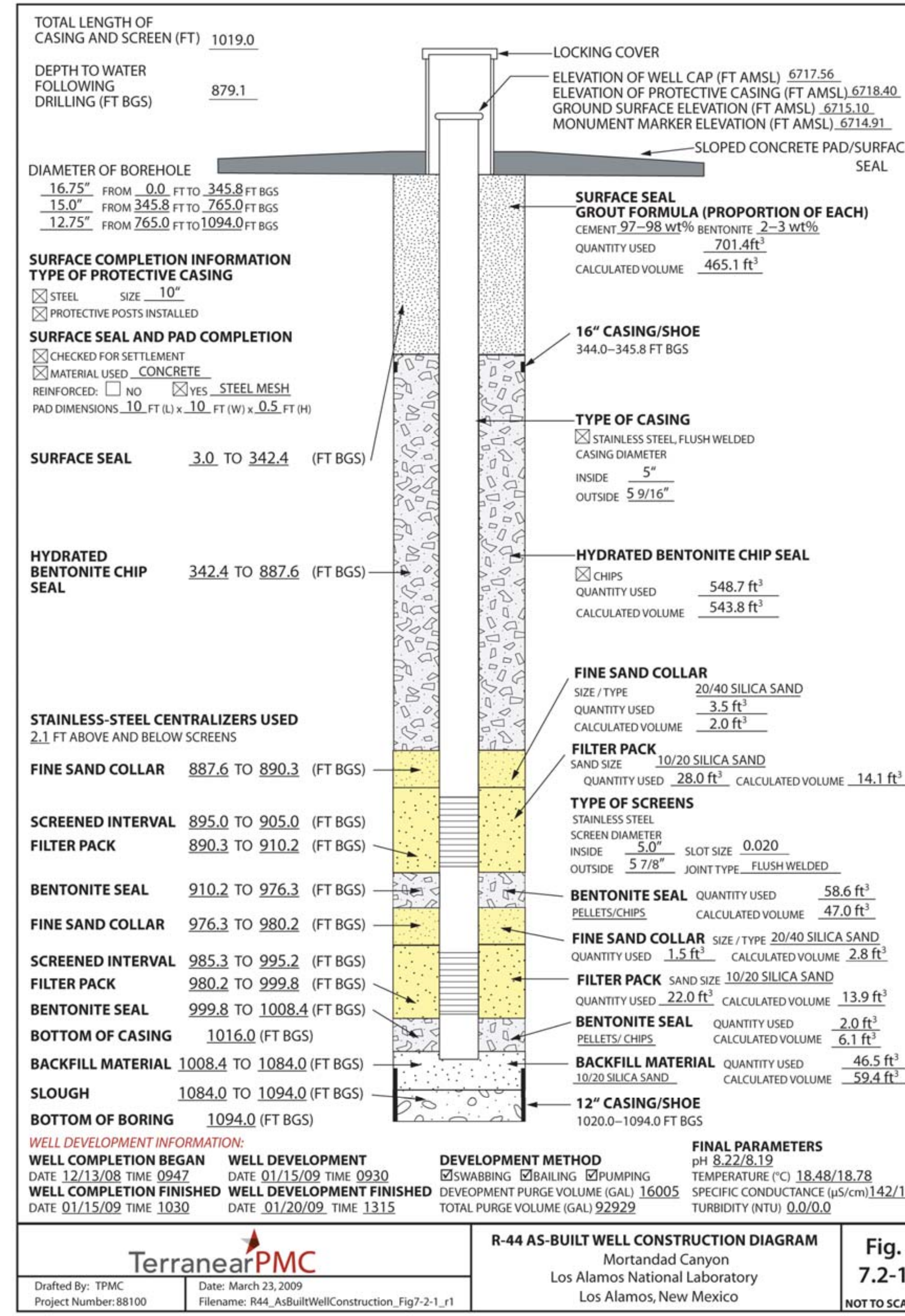


Figure 7.2-1 R-44 as-built well construction diagram

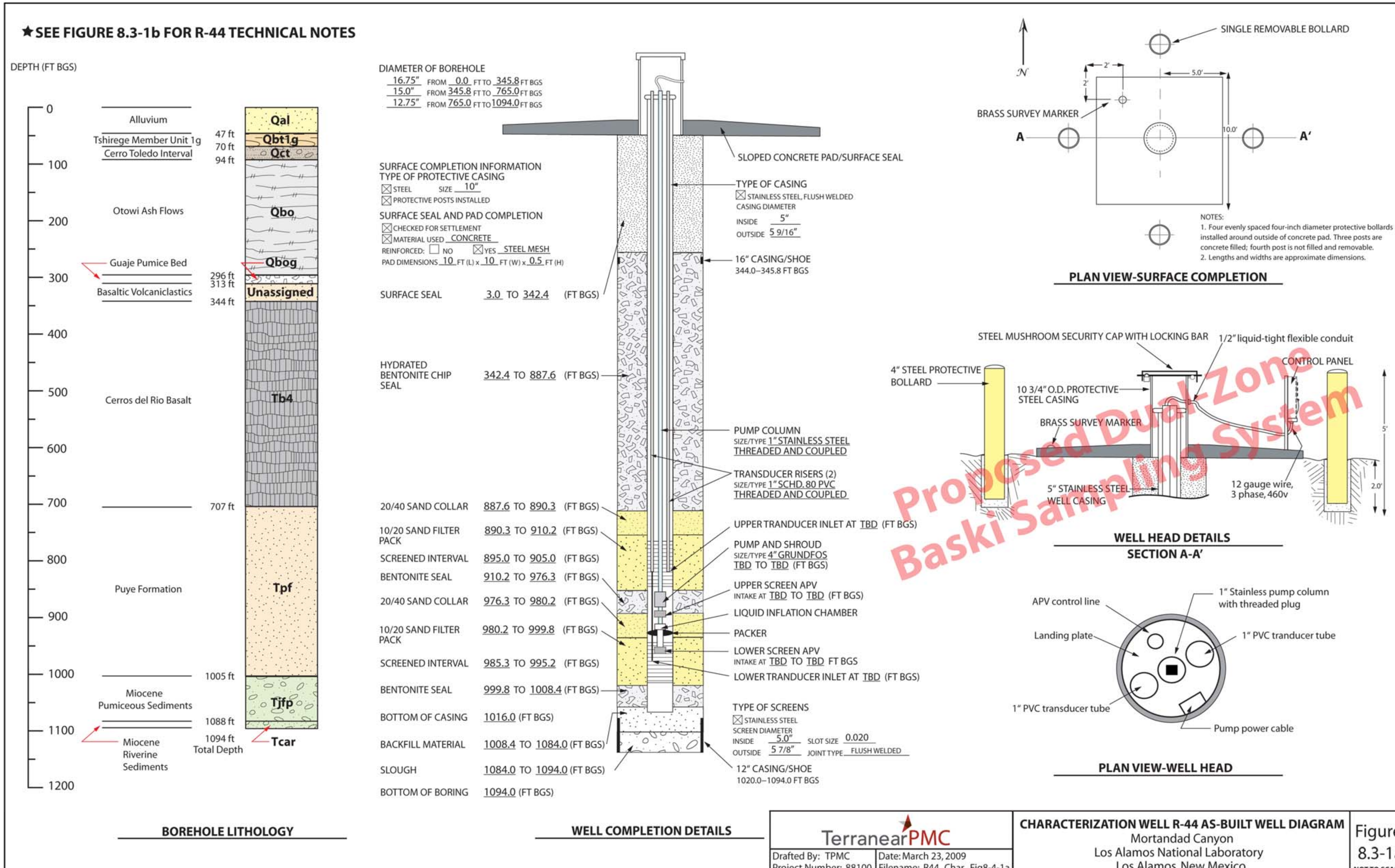


Figure 8.3-1a As-built schematic for regional well R-44

R-44 TECHNICAL NOTES: [1]

SURVEY INFORMATION[2]

Brass Marker

Northing:	1767109.8527 ft
Easting:	1640061.3389 ft
Elevation:	6714.91 ft AMSL

Well Casing (top of stainless steel)

Northing:	1767104.3569 ft
Easting:	1640063.4865 ft
Elevation:	6717.56 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL: natural gamma ray, induction, video
Schlumberger: natural gamma ray, elemental capture (ECS), compensated neutron (CNTG), litho-density (TLD)

DRILLING INFORMATION

Drilling Company
Boart Longyear

Drill Rig
Foremost DR-24HD

Drilling Methods
Dual Rotary
Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids
Air, potable water, AQF-2 Foam

MILESTONE DATES

Drilling

Start:	11/10/2008
Finished:	12/08/2008

Well Completion

Start:	12/13/2008
Finished:	01/15/2009

Well Development

Start:	01/15/2009
Finished:	01/20/2009

WELL DEVELOPMENT

Development Methods
Performed swabbing, bailing, and pumping
Total Volume Purged: 16005 gallons (both screens)

Parameter Measurments (Final, upper screen/lower screen)

pH:	8.22/8.19
Temperature:	18.48/18.78°C
Specific Conductance:	142/193 µS/cm
Turbidity:	0.0/0.0 NTU

AQUIFER TESTING

Step-Tests and Constant Rate Pumping Tests

Upper Screen

Water Produced:	38223 gallons
Average Flow Rate:	24.1 gpm
Performed on:	02/14–17/2009

Lower Screen

Water Produced:	38701 gallons
Average Flow Rate:	23.9 gpm
Performed on:	02/19–22/2009

DEDICATED SAMPLING SYSTEM

Pump
Type: TBD
Model: TBD
TBD U.S. gpm, intake at TBD ft bgs
Environmental Retrofit

Motor
Type: TBD
Model: TBD

Pump Column
TBD

Transducer Tubes
TBD

Transducers
Type: TBD
Model: TBD
S/N: TBD

NOTES:
1) Additional information available in "Final Completion Report, Characterization Well R44 and R45, Los Alamos National Laboratory, Los Alamos, New Mexico, TBD 2009".
2) Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83);
Elevation expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

TerranearPMC

Drafted By: TPMC
Project Number: 86000
Date: March 23, 2009
Filename: R44_TechnicalNotes_Fig8-3-1b_r1

R-44 TECHNICAL NOTES
Mortandad Canyon
Los Alamos National Laboratory
Los Alamos, New Mexico

Figure 8.3-1b
NOT TO SCALE

Figure 8.3-1b As-built technical notes for R-44

Table 3.1-1
Fluid Quantities Used during Drilling and Well Construction

Date	Water (gal.)	Cumulative Water (gal.)	AQF-2 Foam (gal.)	Cumulative AQF-2 Foam (gal.)
Drilling				
11/10/08	1700	1700	3	3
11/11/08	1400	3100	5	8
11/14/08	4500	7600	45	53
11/15/08	6500	14,100	40	93
11/16/08	4000	18,100	0	93
12/02/08	1100	19,200	8	101
12/05/08	200	19,400	0	101
12/06/08	1300	20,700	0	101
12/07/08	900	21,600	0	101
Well Construction				
12/18/08	200	21,800	n/a*	n/a
12/19/08	7000	28,800	n/a	n/a
12/20/08	5700	34,500	n/a	n/a
12/21/08	1000	35,500	n/a	n/a
12/22/08	1000	36,500	n/a	n/a
01/05/09	1500	38,000	n/a	n/a
01/06/09	9800	47,800	n/a	n/a
01/07/09	5000	52,800	n/a	n/a
01/08/09	8700	61,500	n/a	n/a
01/09/09	3500	65,000	n/a	n/a
01/10/09	5500	70,500	n/a	n/a
01/11/09	2500	73,000	n/a	n/a
01/12/09	1600	74,600	n/a	n/a
01/13/09	2150	76,750	n/a	n/a
01/15/09	70	76,820	n/a	n/a
Total Volume (gal.)				
R-44		**76,820**		

Note. Cumulative returns in the pit following drilling and well development are estimated to be approximately 30,000 gal.
*n/a = Not applicable. Foam use and pit use discontinued after drilling activities; therefore, no additional fluids were produced.

Table 4.2-1
Summary of Groundwater-Screening Samples Collected during Drilling, Well Development, and Aquifer Testing of Well R-44

Location ID	Sample ID	Date Collected	Collection Depth (ft bgs)	Sample Type	Analysis
Drilling					
R-44	GW44-09-1292	11/17/08	739.0–739.5	Possible intermediate groundwater	Anions, metals
R-44	GW44-09-1315	11/17/08	739.0–739.5	Possible intermediate groundwater	Tritium
R-44	GW44-09-1293	12/07/08	920	Regional groundwater	Anions, metals
R-44	GW44-09-1294	12/07/08	957	Regional groundwater	Anions, metals
R-44	GW44-09-1295	12/07/08	977	Regional groundwater	Anions, metals
R-44	GW44-09-1296	12/07/08	997	Regional groundwater	Anions, metals
R-44	GW44-09-1297	12/07/08	1017	Regional groundwater	Anions, metals
R-44	GW44-09-1298	12/08/08	1037	Regional groundwater	Anions, metals
R-44	GW44-09-1299	12/08/08	1056	Regional groundwater	Anions, metals
R-44	GW44-09-1301	12/08/08	1076	Regional groundwater	Anions, metals
R-44	GW44-09-1300	12/08/08	1094	Regional groundwater	Anions, metals
Well Development					
R-44	GW44-09-1272	01/18/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1273	01/18/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1274	01/20/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1275	01/20/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
Aquifer Pump Test					
R-44	GW44-09-1276	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1277	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1278	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1279	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1280	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1281	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1282	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1283	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1284	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1285	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1286	02/22/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1287	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC

Note: Tritium was submitted for off-site analysis.

Table 6.0-1
R-44 Video and Geophysical Logging Runs

Date	Depth (ft bgs)	Description
11/17/08	0–<762.0	Run LANL natural gamma-ray, induction, and video tools. Video shows water in borehole at 739 ft bgs. Depths suspect because of "knots" in logging wire line.
12/09/08	0–1094.0	Run Schlumberger suite: cased-hole logs consist of a natural gamma ray, ECS, CNL, and TLD after reaching TD.

Table 7.2-1
R-44 Annular Fill Materials

Material	Volume
Surface seal: cement slurry	701.4 ft^3
Upper seal: bentonite chips	548.7 ft^3
Upper fine sand collar: 20/40 silica sand	3.5 ft^3
Upper filter pack: 10/20 silica sand	28.0 ft^3
Middle seal: bentonite pellets/chips	58.6 (13.4/45.2) ft^3
Lower fine sand collar: 20/40 silica sand	1.5 ft^3
Lower filter pack:	22.0 ft^3
Lower seal: bentonite pellets/chips	2.0 (1.3/0.7) ft^3
Backfill material: 10/20 silica sand	46.5 ft^3
Backfill material: formation slough	8.9 ft^3
Potable water used in the regional aquifer (drilling and well construction)	76,820 gal.

Table 8.5-1
R-44 Survey Coordinates

North	East	Elevation	Identification
1767109.85	1640061.34	6714.91	R-44 brass pin embedded in pad
1767105.66	1640062.81	6715.10	R-44 ground surface near pad
1767104.88	1640063.73	6718.40	R-44 top of 10-in. protective casing
1767104.36	1640063.49	6717.56	R-44 top of stainless-steel well casing

Notes: All coordinates are expressed as New Mexico State Plane Coordinate System Central Zone (NAD 83). Elevation is expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

Table 8.6-1
Summary of Waste Samples Collected during Drilling and Development of R-44

Location ID	Sample ID	Date Collected	Description	Sample Type
R-44	RC05-09-1519	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1520	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1521	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1522	1/26/09	Trip blank	Liquid
R-44	RC05-09-1527	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1528	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1529	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1530	1/26/09	Trip blank	Liquid
R-44	RC05-09-1535	1/26/09	Purge water	Liquid
R-44	RC05-09-1536	1/26/09	Purge water	Liquid
R-44	RC05-09-1537	1/26/09	Purge water	Liquid
R-44	RC05-09-1538	1/26/09	Trip blank	Liquid
R-44	RC05-09-1543	1/26/09	Drill cuttings	Solid
R-44	RC05-09-1544	1/26/09	QC sample of -1543	Solid

Appendix A

Well R-44 Lithologic Log

Los Alamos National Laboratory
Regional Hydrogeologic Characterization Project
Borehole Lithologic Log

COREHOLE IDENTIFICATION (ID): R-44	**TECHNICAL AREA (TA):** 5	**PAGE:** 1 of 16
DRILLING COMPANY: Boart Longyear Company	**START DATE/TIME:** 11/10/2008:0145	**END DATE/TIME:** 12/8/2008: 1735
DRILLING METHOD: Dual Rotary	**MACHINE**: Foremost DR24 HD	**SAMPLING METHOD:** Grab
GROUND ELEVATION:		**TOTAL DEPTH:** 1094 ft below ground surface (bgs)
DRILLERS: J. Staloch, C. Johnson	**SITE GEOLOGISTS:** A. Miller, C. Pigman, J. R. Lawrence	

Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
0–47	ALLUVIUM: Unconsolidated tuffaceous sediments—light grayish tan (7.5YR 7/1) to pale pinkish tan (7.5YR 8/4) silty fine to medium sand with minor pebble gravel; detrital grains/clasts of indurated tuff, quartz and sanidine crystals, pumice and volcanic lithics. 0–5 ft surficial construction fill. 5–20 ft +10F: quartz and sanidine crystal grains, mixed angular volcanic clasts and fragments of siltstone/very fine-grained sandstones. 20–47 ft +10F/+35F: siltstone fragments, quartz and sanidine grains, granules of weathered pumice. 95–108 ft +10F: mixed weathered pumice fragments, subangular granules of various volcanic rocks, fragments of silty very fine-grained sandstone.	Qal	Note: Drill cuttings for microscopic and descriptive analysis were collected at 5-ft intervals from 0 ft bgs to borehole total depth (TD) at 1094 ft bgs. Quaternary alluvial sediments, from 0 to 47 ft bgs, are estimated to be 47 ft thick. The Qal–Qbt 1g contact is estimated to be at 47 ft bgs.
47–70	**UNIT 1g OF THE TSHIREGE MEMBER OF THE BANDELIER TUFF:** Tuff—light grayish tan (7.5YR 7/1), very poorly welded, locally abundant ash matrix. 47—55 ft WR: silty ash matrix. +10F/+35F: 30%–40% fragments tuffaceous fragments with abundant quartz and sanidine grains; 30%–40% subangular lithics (up to 10 mm in diameter) of diverse intermediate volcanic rocks; 10%–20% weathered pumices. 55–60 ft +10F: 60%–70% white and gray subangular dacitic lithics; 20%–30% weathered pumices fragments; 10% fragments of silty sandstone. 60–70 ft +10F: 90-95% subrounded granule-size lithics of various volcanic lithologies (dacites, obsidian, rhyodacite(?); 10% weathered pumices fragments.	Qbt 1g	Unit 1g of the Tshirege Member of the Bandelier Tuff, from 47 to 70 ft bgs, is estimated to be 23 ft thick. The Qbt 1g–Qct contact is estimated to be at 70 ft bgs, based on natural gamma log interpretation.

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 2 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
70–94	**CERRO TOLEDO INTERVAL OF THE BANDELIER TUFF:** Tuffaceous sediments—light grayish tan (7.5YR 7/1) to pale pinkish tan (7.5YR 8/4) poorly consolidated to unconsolidated silty fine- to medium-grained sandstone with local pebble gravel, locally abundant ash matrix. 70–80 ft +10F/+35F: 30%–40% fragments of tuffaceous silty sandstone with abundant quartz and sanidine grains; 30%–40% subangular lithics (up to 10 mm in diameter) of diverse intermediate volcanic lithologies; 10%–20% weathered pumices. 80–94 ft+10F: very little material retained of this size fraction; minor dacite granules. +35F: abundant quartz and sanidine crystal grains, pumices and volcanic lithic fragments.		Qbo	The Cerro Toledo interval, from 70 to 94 ft bgs, is estimated to be 24 ft thick. The Qct–Qbo contact is estimated to be at 94 ft bgs, based on natural gamma log interpretation.
94–110	**OTOWI MEMBER OF THE BANDELIER TUFF:** Tuff—pale orange tan (7.5YR 8/6) to pinkish white (7.5YR 8/2), poorly welded, lithic-bearing, locally abundant ash and silt matrix; this interval likely represents the weathered upper part of Qbo. 94–105 ft +10F: mixed weathered pumice fragments, various subangular volcanic lithics. 105–110 ftWR: abundant silt and ash matrix. +10F: 75% weathered to glassy pumices (up to 20 mm in diameter); 15%–25% dacite fragments.		Qbo	Otowi Member ash-flow tuff, encountered from 94 to 296 ft bgs, is estimated to be 202 ft thick.
110–120	Tuff—white (7.5YR 8/2), poorly welded, crystal-rich, lithic-bearing, abundant ash matrix. 110—120 ft +10F: 100% pumice fragments (up to 22 mm in diameter), mostly glassy to locally devitrified, quartz- and sanidine-phyric. +35F: 30%–35% quartz and sanidine crystals, 40%–45% pumice fragments (mostly glassy).		Qbo	
120–135	Tuff—white (10YR 8/1), poorly welded, pumice-rich, crystal-rich, lithic-bearing, abundant ash matrix. 120–135 ft +10F: 80%–85% vitric pumice fragments (up to 14 mm in diameter), quartz- and sanidine-phyric. +35F: 15%–20% broken volcanic lithic fragments (up to 15 mm) composed of light gray and pinkish biotite-dacites. 35F: 30%–40% quartz and sanidine crystals, 40%–50% glassy pumice fragments; 15%–20% lithic fragments.		Qbo	125—135 ft +10F contains large dacite fragments up to 25 mm in diameter.
135–145	Tuff—white (10YR 8/1), poorly welded, pumice-rich, crystal-rich, lithic-poor, abundant fine volcanic ash matrix. 135—145 ft +10F: 100% vitric pumice fragments (up to 20 mm in diameter), quartz- and sanidine-phyric; trace volcanic lithics. +35F: 40%–50% quartz and sanidine crystals, 30%–40% 10%–15% volcanic lithic grains.		Qbo	

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 3 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
145–155	Tuff—white (10YR 8/1), poorly welded, pumiceous, crystal-rich, lithic-bearing to lithic-rich, moderate volcanic ash matrix. 145—155 ft +10F: 40%–50% white vitric quartz-sanidine-phyric pumice fragments; 40%–50% broken, angular and subangular gray and pinkish dacites. +35F: 50%–60% quartz and sanidine crystals, 30%–40% glassy pumice fragments, 10%–15% volcanic lithic fragments. 125–135 ft 10%–20% light gray dacite fragments (i.e., xenoliths) up to 25 mm in diameter.		Qbo	
155–170	Tuff—pale yellowish tan (10YR 7/4), poorly welded, pumiceous, lithic-rich, crystal-rich, abundant ash matrix. 155—160 ft +10F: 40%–50% angular lithics (up to 9 mm) of intermediate volcanic compositions (porphyritic dacites are common); 40%–50% white to pink glassy pumices, quartz-sanidine-phyric. 160–170 ft contains 65%–70% angular, porphyritic biotite-phyric dacite lithic fragments (up to 18 mm in diameter).		Qbo	
170–175	Tuff—white (10YR 8/1), poorly welded, pumiceous, lithic-bearing, crystal-rich, abundant volcanic ash matrix. 170—175 ft +10F: 100% white glassy pumices, quartz- and sanidine-phyric. +35F: 20%–30% glassy pumice fragments, 30%–40% quartz and sanidine crystals, 30%–40% dacitic grains.		Unit 1g, Qbt	
175-185	Tuffaceous/volcaniclastic sediments (?? Tuff ?? It would be unusual to find tuffaceous sediments in the middle of the Qbo ash-flow tuffs.) —pale pinkish tan (2.5YR 8/4) to white (10YR 8/1) poorly consolidated, silty fine to medium sand and pebble gravels. 175–180 ft+10F: pale pinkish tan (2.5YR 8/4), 40%–50% broken/angular volcanic lithic fragments/clasts (up to 12 mm in diameter) composed of dacite and flow-banded rhyolite; 40%–50% vitric pumices grains, quartz- and sanidine-phyric. 180-185 ft +10F: white (10YR 8/1), 90%–95% broken/angular volcanic lithic fragments/clasts (up to 11 mm in diameter) of diverse composition: pink and gray dacites, flow-banded rhyolites, dark brown andesite(?), minor vesicular basalt; 5%–10% vitric white and pinkish pumices, quartz- and sanidine-phyric.		Qbo	

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 4 of 16	
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
185-220	Tuff—pale orange tan (5YR 7/4), poorly welded, pumiceous, lithic-poor, crystal-rich. 185—195 ft WR/+10F: 97%–99% white to pale orange (i.e., limonite-stained) vitric quartz-sanidine-phyric pumice fragments (up to 18 mm in diameter), commonly with black specks of secondary Fe-oxide; 1%–3% dacite lithic fragments. +35F: 20%–30% glassy pumice fragments, 50%–60% quartz and sanidine crystals, 20%–30% volcanic lithic grains. 195'–200 ft WR: more abundant pale orange volcanic ash matrix. +10F: contains 25%–35% biotite-phyric dacite lithics. 200–215 ft+10F: similar to 185—190 ft. 215–220 ft +10F: 85-90% white to pale orange vitric quartz-sanidine-phyric pumice fragments (up to 13 mm in diameter); 10%–15% volcanic lithic fragments.	Qbo	
220–240	Tuff—pale orange tan (7.5YR 7/6), poorly welded, pumiceous, lithic-rich, crystal-rich, locally abundant ash matrix. 220–230 ft WR: abundant pale orange volcanic ash matrix. +10F: 75%–80% white to pale orange vitric quartz-sanidine-phyric pumice fragments (up to 14 mm in diameter), frequently exhibiting black specks of secondary Fe-oxide; 20%–25% pinkish to light gray biotite-dacite lithic fragments. +35F: 40%–50% glassy pumice fragments, 30%–40% quartz and sanidine crystals, 5%–10% volcanic lithic grains. 230—235 ft +10F: 60%–70% white and pale orange glassy pumice, quartz- and sanidine phyric, commonly with abundant dark Fe-oxide specks; 30%–40% subangular volcanic lithics (up to 8 mm in diameter) composed of various volcanic lithologies: pink and gray dacites, banded dacites, dark porphyritic vitrophyre. 235—240 ft +10F: similar to 230—235 ft.	Qbo	
240–250	Tuff—very pale orange tan (7.5YR 8/4), poorly welded, pumiceous, lithic-poor, crystal-rich. 240–250 ftWR: locally with abundant pale orange volcanic ash matrix. +10F: 99%–100% white to pale orange-pink vitric quartz-sanidine-phyric pumice fragments (up to 25 mm in diameter) with locally abundant specks of dark secondary Fe-oxide; <1% dacite lithics. +35F: 50%–60% quartz and sanidine crystals; 20%–30% dacite grains; 20%–30% pumice fragments.	Qbo	

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 5 of 16	
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
250–265	Tuff—pinkish white (7.5YR 8/2), poorly welded, pumiceous, lithic-bearing to lithic rich, crystal-rich. 250—265 ft +10F: 65%–75% white to pale pink glassy quartz-sanidine-phyric pumice fragments (up to 14 mm in diameter) with locally abundant specks of dark secondary Fe-oxide; 35%–25% angular fragments (up to 10 mm in diameter) composed of dark and light gray dacites. +35F: 30%–40% quartz and sanidine crystals; 25%–35% dacite lithic grains; 30%–40% pumice fragments.	Qbo	
265–285	Tuff—pale pinkish white (7.5YR 8/2), poorly welded, pumice-rich, lithic-bearing, crystal-rich. 265—270 ft +10F: 95%–98% white to pale pinkish white, fibrous, vitric, quartz-sanidine-phyric pumice fragments (up to 23 mm in diameter); 2%–5% gray and light pink dacite lithics (up to 6 mm in diameter). +35F: 50%–60% quartz and sanidine crystals; 15%–25% dacitic lithic fragments with minor basalt, black vitrophyre; 15%–25% pumice grains. 270–275 ft+10F: note increased abundances and varieties of lithics–80% vitric pumices; 20% mixed volcanic (dacite, basalt, rhyolite) fragments (up to 7 mm in diameter). 275–285 ft no cuttings available for description.	Qbo	
285–296	Tuff—pale pinkish white (7.5YR 8/2), poorly welded, pumice-rich, lithic-poor, crystal-poor. 285–296 ft WR: silty matrix. +10F: 95%–98% white fibrous, vitric, quartz-sanidine-phyric pumice fragments (up to 22 mm in diameter); 2%–5% dacite lithics. +35F: Note poor representation of this sample size fraction, 50%–60% pumice fragments; 10%–15% quartz and sanidine crystals; 20%–25% volcanic lithic fragments (dacite, black vitrophyre).	Qbo	The Qbo-Qbog contact is placed at 296 ft bgs, based on interpretation of natural gamma geophysical log data.
296–313	**GUAJE PUMICE BED:** Tuff— white (5YR 8/1) to very pale orange (7.5YR 7/6), pumice-rich, lithic-poor, crystal-poor, no apparent volcanic ash matrix. 296—305 ft WR/+10F: 97%–98% white to locally yellowish (i.e., weak limonite-staining) vitric pumices (up to 22 mm in diameter); 2%–3% dacitic lithic fragments (up to 20 mm in diameter). 305—315 ft WR/+10F: 100% white and locally pinkish vitric pumices (up to 22 mm in diameter); phenocryst-poor, having pristine, very fresh appearance. 310—315 ft +35F: no returns of this sample size fraction.	Qbog	The Guaje Pumice Bed, from 296 to 313 ft bgs, is estimated to be 17 ft thick. The contact between Qbog and underlying basalt-rich sediments is placed at 313 ft bgs, based on interpretation of natural gamma geophysical log.

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5		**Page:** 6 of 16
Depth (ft bgs)	**Lithology**	**Lithologic Symbol**	**Notes**
313–340	**BASALT-RICH SEDIMENTS:** Pale orange tan (7.5YR 7/6) siltstone to silty fine- to medium-grained sandstone with pebble gravel. 313–325' ft +10F: 60% orange-tan fragments of indurated siltstone with fine basalt grains; 20% fragments of vitric pumice; 20% broken to subangular clasts of hornblende-dacite and minor black basalt scoria. 325—330 ft +10F: 100% large pebbles (up to 20 mm in diameter) subrounded clasts of black basalt scoria with adhered rinds of orange-tan siltstone. +35F: 80% siltstone fragments, 10% pumice fragments plus quartz and sanidine crystals, 10% basalt and dacitic grains. 330–340 ft +10F: No sample recovery.	N/S	Unassigned basalt-rich volcaniclastic sediments, encountered from 313 to 344 ft bgs, are estimated to be 31 ft thick.
340–344	Pale pinkish tan (7.5YR 8/4) siltstone and fine-grained sandstone with subordinate chips of basalt-bearing siltstone similar to 340–345 ft. Coarse- to medium-grained sand with pebble gravel. 340–345 ft WR: 100% siltstone fragments with basalt pebbles. +10F: 40% silt-coated basalt pebbles (up to 15mm), 60% siltstone and fine-grained sandstone fragments with basalt, dacite, and quartzite granules.	N/S	Estimated contact between basalt-rich sediments and underlying Tb4 is placed at 344 ft bgs, based on natural gamma log interpretation.
344–375	**CERROS DEL RIO BASALT:** Basalt lava–medium gray (GLEY1 6/0) strongly vesicular, porphyritic with aphanitic groundmass, clinopyroxene (cpx), plagioclase and minor olivine present as phenocrysts. 344–355 ft +10F/+35F: 100% basalt chips, phenocrysts 2%–4% by volume, anhedral (up to 3 mm in diameter) dark brown to opaque cpx and minor small (up to 1 mm in diameter) green olivine; olivine commonly intergrown with cpx. 355–375 ft +10F/+35F: 100% basalt chips, compositionally similar to 344–355 ft; degree of vesicularity diminishing rapidly downward.	Tb4	The Cerros del Rio basalt section, encountered from 344 to 707 ft bgs, is estimated to be 363 ft thick. 344–355 ft represents the strongly vesicular top of cpx-basalt flow.
375–392	Basaltic cinder deposits–dark reddish brown (2.5YR 4/4) to orange brown (2.5YR 5/6) scoriaceous basalt. 375–395 ft WR/+10F: 95%–97% scoriaceous basalt chips and orange-brown ferruginous cinders (up to 20 mm in diameter); 3%–5% vesicular crystal-poor cpx-basalt chips, trace locally abundant white amygdaloidal zeolite (?) and zeolite-encrusted scoria.	Tb4	

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5		**Page:** 7 of 16
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
392–435	Basalt lava–light gray (GLEY1 7/0), weakly vesicular to massive, phenocryst-poor, aphanitic groundmass, clinopyroxene-bearing basalt, moderately altered groundmass. 392–435 ft+10F/+35F: 99% basalt chips of altered cpx-phyric basalt, minor basalt scoria; phenocrysts (2%–4% by volume) of anhedral clinopyroxene (up to 2 mm in diameter) and minor small (up to 1 mm in diameter) green olivine; olivine and cpx commonly intergrown.	Tb4	392–435 ft characteristic of this lava is the strong recrystallization of groundmass feldspars yielding bleached coloration and webs/tiny veinlets of clay; dusty appearance and rounding (i.e., apparent milling because of the drilling process) of chips.
435–468	Basalt lava–light gray (GLEY1 7/1) massive to weakly vesicular, weakly porphyritic with aphanitic GM, groundmass feldspar strongly altered. 435–455 ft +10F: 100% basalt chips that are commonly rounded or milled by drilling process, sparse phenocrysts 2%–3% by volume of anhedral dark brown clinopyroxene (up to 1 mm in diameter) and lesser small green translucent olivine (<1 mm in diameter); cpx and olivine are commonly intergrown. Groundmass is distinctive in that the felty feldspars are bleached and recrystallized. 455–460 ft WR/+10F: 100% basalt chips, edges milled during drilling process. Olivine becoming more abundant and large (up to 2 mm in diameter) as euhedral phenocrysts downward in section. 460–468 ft: Similar to 435–455 ft.	Tb4	
468–471	Basaltic lava and cinder deposits–varicolored light gray (GLEY1 7/1) and reddish brown (10YR 5/6) mixed basalts with altered GM and scoriaceous basalt. WR: finely milled basalt chips with abundant white powder produced from altered feldspars. 468–471 ft: +10F: 70% angular to rounded (milled) basalt chips, massive to weakly vesicular, phenocrysts 3%–5% by volume anhedral olivine (up to 3 mm in diameter) and lesser small (1 mm in diameter) cpx; black cpx commonly occurs as rinds/overgrowths of olivine. GM strongly bleached; 30% angular chips of brick-red scoriaceous basalt.	Tb4	

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 8 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
471–490	Basalt lava–light gray (GLEY1 6/1) basalt chips, massive, altered groundmass; minor reddish (10YR 5/6) basalt scoria. 471–475 ft WR/+10F: 75%–80% abraded/milled chips olivine+cpx-basalt, groundmass strongly altered. 20%–25% broken chips of ferruginous basalt scoria. 475–485 ft WR: abundant finely ground white powder; basalt chips abraded/milled. +10F/+35F: 95%–97% chips of olivine-cpx porphyritic basalt, phenocrysts of anhedral pale green olivine (up to 2 mm in diameter) and small (up to 1 mm in diameter) black cpx, groundmass feldspars are bleached/recrystallized. 3%–5% chips of hematite-stained basalt scoria. 485–490 ft+10F: Locally more abundant (15%–20% by volume) basalt scoria chips.		Tb4	
490–505	Basalt lava–light gray (GLEY1 6/1) moderately altered massive to weakly vesicular olivine-cpx basalt, with moderately to slightly altered groundmass. 490–505 ft WR/+10F: 99%–100%% massive basalt chips, phenocrysts (2%–4% by volume) of small (1 mm in diameter) olivine and cpx; groundmass feldspar altered/recrystallized/bleached; up to 1% reddish brown scoria chips.		Tb4	
505–530	Basaltic cinders and volcaniclastic sediments–varicolored light gray (GLEY 6/1) and brick-red (2.5 YR 5/8), mixed massive basaltic lava and scoria/cinders. 505–515 ft WR/+10F: 50% light gray chips of cpx- and minor olivine-phyric basalt; groundmass feldspar recrystallized/bleached. 50% hematitic vesicular basalt and glassy scoriaceous cinders (up to 15 mm in diameter). +35F: moderately abundant fragments of volcaniclastic sediments. 515–530 ft +10F/+35F: Mixed basalt fragments of brick-red glassy scoria, minor dacitic detritus and fragments of fine-grained volcaniclastic sediments.		Tb4	505–530 interval possibly of hydromagmatic origin.
530–535	Pumiceous volcaniclastic sediments–varicolored light gray (GLEY1 6/1), reddish brown (10YR 4/8) and white (10YR 8/1) fragments of basalt, pumice and brown scoria. 530–535 ft WR +35F/+10F: 55%–65% chips and partly subrounded detrital grains of weakly vesicular cpx-bearing basalt. 20%–25% fragments of weathered quartz- and sanidine-phyric pumice, detrital grains of dacite.		Tb4	530–535 ft some evidence of reworked volcanic materials indicated by local subrounding of basalt, pumice, and dacite fragments.

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 9 of 16	
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
535–560	Basalt lava–reddish gray (10YR 5/1) massive to weakly vesicular cpx-basalt, porphyritic with aphanitc groundmass, groundmass feldspars weakly altered and bleached. 535–540 ft WR/+10F: 100% angular basalt chips, phenocrysts (3%–5% by volume) of anhedral opaque black cpx (up to 3 mm in diameter); groundmass weakly bleached; minor local white calcite on fractured surfaces.	Tb4	
560–575	Basalt lava–light gray (GLEY1 7/1) massive to weakly vesicular cpx-basalt, porphyritic with aphanitc groundmass that is moderately altered. 560–565 ft +10F/+35F: 95-97% angular chips of cpx-phyric basalt, cpx-phenocrysts (2%–4% by volume); groundmass moderately recrystallized/bleached. 3%–5% subangular detrital granules/grains of pumice and quartzite. 565–575 ft+10F/+35F: minor to trace fragments of fine-grained volcaniclastic sandstone.	Tb4	
575–590	Basalt lava–light gray (GLEY1 7/1) massive cpx-basalt, porphyritic with strongly altered aphanitic groundmass. 575–580 ft +10F/+35F: 80%–85% subrounded (i.e., milled because of drilling) chips of cpx-basalt; 15%–20% detrital grains of pumice and quartz crystal, also fragments of pale tan clay and fine-grained sandstone. 580–590 ft+10F/+35F: trace abundances of pale orange clay.	Tb4	
590–650	Basalt lava–light gray (GLEY1 7/1) massive to weakly vesicular cpx- and ol-phyric basalt, porphyritic with moderately strongly altered aphanitic groundmass. 590'–610 ft +10F/+35F: 100% subrounded (i.e., milled because of drilling) chips of basalt, phenocrysts (3%–5% by volume) of subhedral cpx (up to 1 mm in diameter) and lesser green olivine (up to 4 mm in diameter) that are commonly rimmed by black cpx; groundmass altered and bleached; trace pale tan clay fragments. 610–620 ft +10F/+35F: 99%–100% cpx-basalt chips with strongly altered groundmass; <1% detrital grains of ferruginous scoria, quartzite, and tan clay fragments. 620–630 ft +10F: olivine phenocrysts frequently exhibit cpx overgrowths. 630–650 ft +10F/35F: 98%–99% milled cpx-olivine basalt chips exhibiting strongly altered groundmass; olivine phenocrysts frequently have cpx overgrowths 1%–2% detrital grains of pumice, quartz crystal and fragments of very fine-grained silty sandstone.	Tb4	647–648 ft possible thin sedimentary interlayer containing pumice fragments and basalt granules.

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 10 of 16	
Depth (ft bgs)	**Lithology**	**Lithologic Symbol**	**Notes**
650–665	Basalt lava–light gray (GLEY1 7/1) massive olivine-phyric basalt, weakly porphyritic with altered aphanitic groundmass. 650–665 ft WR/+10F: 100% subrounded (i.e., milled because of drilling) chips of ol-basalt, phenocrysts (1%–3% by volume) of small anhedral green olivine and trace cpx; groundmass moderately to strongly altered and bleached; minor white clay on fracture surfaces; trace fragments of light pink claystone.	Tb4	
665–685	Basaltic volcaniclastic sediments–varicolored white (2.5YR 8/1), medium gray (GLEY16/1) and reddish brown (2.5YR 4/6), mixed detrital clasts/grains of pumice and basalt 665–670 ft +10F: 100% subangular to subrounded detrital pebbles/clasts (up to 17 mm in diameter) composed mostly of glassy quartz- and sanidine-phyric pumices with subordinate amounts of gray massive and reddish scoriaceous basalt. 670%–685 ft +10F: 40%–50% gray basaltic detrital clasts (locally rounded) and chips; 40%–50% pale pinkish porphyritic, vitric pumices and fragments of welded tuff (crystal-rich, lithic-bearing, pumiceous).	Tb4	665–685 ft apparent sedimentary interlayer between Tb4 basalt flows.
685–698	Basalt lava—medium gray (GLEY1 6/1) massive basalt, olivine-phyric, phenocryst-poor, moderate very fine-grained alteration of groundmass feldspars. 685–698 ft WR/+10F: 99%–100% angular basalt chips, phenocrysts (1%–2% by volume) of pale green anhedral olivine (up to 1mm in diameter); groundmass feldspars moderately recrystallized and bleached; minor fragments of white clay or claystone.	Tb4	
698–707	Basalt lava and volcaniclastic sediments—varicolored medium gray (GLEY1 6/1) to light pinkish tan (2.5YR 8/3), mostly chips/detritus of olivine-basalt and lesser volcaniclastic detritus. 698–707 ftWR/+10F: 80%–90% angular chips and subrounded detrital granules of olivine-phyric basalt; 10%–20% subangular to subrounded detrital volcanic clasts (up to 7 mm in diameter) including gray dacite, white pumices, red scoriaceous cinders; note trace white clay adhered to detrital basaltic grains.	Tb4	698–707 ft apparent rubbly base of basaltic flow with intercalated thins volcaniclastic sedimentary layer. Estimated Tb4-Tpf contact placed at 707 ft bgs.
707–715	**PUYE FORMATION:** Volcaniclastic sediments—varicolored light gray (GLEY1 7/0) and pale tan (5YR 8/3) coarse to medium gravels with fine-grained sand to silty sand matrix, subangular to rounded clasts of diverse volcanic compositions 707–715 ft WR/+10F: 40% subrounded light and dark gray olivine-basalt granules (up to 10 mm in diameter); 60% subangular to subrounded pebbles and broken clasts (up to 22 mm in diameter) composed of dacites, minor pinkish pumice and fragments of indurated dacitic silty sandstone.	Tpf	Puye volcaniclastic sediments, encountered from 707 to 1005 ft bgs, are estimated to be 298 ft thick.

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 11 of 16
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
715–735	Volcaniclastic sediments—varicolored pale pinkish gray (5YR 7/2) to light gray (GLEY1 7/1) coarse to medium gravels with fine-grained sandy to silty matrix, subangular to rounded clasts of dacite and minor basalt. 715–735 ft WR/+10F: 90%–95% broken and subrounded to rounded clasts (up to 25 mm in diameter) light gray bt- and hbn-phyric dacites; 5%–10% fragments to indurated fine-grained sandstone. +35F: subangular to subrounded grains: 75%–80% dacites; 20%–25% basalt; 3%–5% quartz and sanidine crystals; trace red scoria.	Tpf	
735–750	Volcaniclastic sediments—light gray (GLEY1 7/1) coarse gravels with fine- to medium-grained sandstone, subangular to subrounded clasts dominantly of dacites. 10F: broken and subrounded clasts (up to 19 mm in diameter) mostly porphyritic dacites; 10%–15% reddish vesicular basalt, minor weathered pumices.	Tpf	
750–760	Volcaniclastic sediments—light gray (GLEY1 7/1) to pinkish gray (5YR 7/2) medium- to coarse-grained sandstone with minor pebble gravel, detritus composed dominantly of porphyritic hbn- and bt-dacites. 750—760 ft WR/+10F: broken and subrounded clasts (up to 13 mm in diameter), 70% white and grayish bt-phyric dacites 30% fragments of silty fine-grained volcanic sandstone.	Tpf	
760–785	Volcaniclastic sediments—varicolored light gray (GLEY1 7/1) to pinkish gray (5YR 7/2) coarse gravels and medium- to coarse-grained sandstones, detrital clasts composed of various volcanic lithologies, predominantly dacites. 760–770 ft WR/+10F: subangular to subrounded clasts (up to 17 mm in diameter) light gray to pinkish porphyritic dacites, minor dark brown andesite. 770–785 ft +10F: very coarse gravels indicated by abundantly large (up to 20 mm in diameter) broken chips of porphyritic dacites, minor dark gray vitrophyre, and minor fragments of indurated sandstone.	Tpf	
785–795	Volcaniclastic sediments—light pinkish gray (7.5YR 7/1) very coarse- to medium-grained sandstones with small pebbles, predominantly dacitic detritus. 785—795 ft WR/+10F: subangular to subrounded granules (up to 4 mm in diameter) almost exclusively of gray porphyritic dacites.	Tpf	

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 12 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
795–815	Volcaniclastic sediments—light pinkish gray (7.5YR 7/1) very coarse- to medium-grained sandstones with silt, composition of grains predominantly dacitic. 795—815 ft WR/+10F: subangular to subrounded grains and granules mostly of gray to pinkish gray dacites, lesser abundances other volcanics.		Tpf	795–815 ft silt percentage of silt increasing downward in this interval.
815–840	Volcaniclastic sediments—varicolored light pinkish gray (GLEY1 7/0) to light pinkish gray (7.5YR 7/1) coarse gravels and medium- to coarse-grained sandstones; clast composition predominantly dacitic. 815–820 ft WR/+10F: subangular to subrounded clasts (up to 18 mm) mostly gray porphyritic dacites, minor orange and dark gray porphyritic vitrophyre, trace cpx-phyric basalt. 820–830' ftWR/+10F: clast composition more diverse: gray dacites, white bt-phyric dacite, gray and white dacitic(?) vitrophyre. 830–840 ft WR/+10F: subangular to subrounded clasts (up to 13 mm) almost exclusively light gray dacites, trace white bt-dacite.		Tpf	
840-860	Volcaniclastic sediments—pale pinkish tan (7.5YR 7/3) fine to coarse gravels and fine- to medium-grained sandstone with silt; clast composition predominantly dacitic. 840–850 ft WR: silt-rich matrix. +10F: broken and subangular to subrounded granules and small pebbles (up to 12 mm) mostly of light gray porphyritic dacites, minor white bt-phyric dacite. 850–860 ft texturally similar to 840–850 ft; contains also 2%–3% fragments of medium- to coarse-grained silty sandstone, also minor porphyritic vitrophyre.		Tpf	
860–870	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/2) fine to medium gravel and medium- to coarse-grained sandstone, predominantly dacitic detritus. 860–870 ft WR/+10F: broken and subangular to subrounded clasts (up to 11 mm in diameter) composed almost exclusively of light gray porphyritic dacites, minor white bt -phyric dacite matrix. +10F		Tpf	
870–875	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/2) fine-grained to very coarse-grained sandstone to silty sandstone with gravel, clasts predominantly dacitic. 870–875 ft WR: moderately silty matrix. +10F: broken and subangular to subrounded clasts (up to 16 mm in diameter) of gray porphyritic dacite, white bt-bearing dacite, and minor chips of indurated sandstone.		Tpf	

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 13 of 16	
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
875–890	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/2) fine to coarse gravel and medium- to coarse-grained sandstone, clasts predominantly dacitic. 875–885 ft WR/+10F: broken and subangular clasts (up to 10 mm in diameter) of gray porphyritic dacites, pink dacites, white bt-phyric dacite(?); minor fragments of indurated medium-grained sandstone. 885–890 ftWR/+10F: broken to subrounded detrital clasts (up to 18 mm in diameter) of light gray dacites and reddish brown hornblende (hbn)-biotite (bt)-bearing dacites.	Tpf	
890–900	Volcaniclastic sediments—very pale pinkish gray (7.5YR 7/1) fine- to coarse-grained sandstones with gravel to silty sand with gravel, clasts predominantly dacitic. 890–900 ftWR: silty matrix. +10F: broken and subangular clasts (up to 15 mm in diameter) composed of porphyritic hbn-dacite and minor bt-bearing dacite.	Tpf	
900–905	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/1) to very light gray (GLEY1 7/0) fine- to coarse- grained sandstones, dacitic detritus. 900–905 ft +10F: broken and subrounded granules (up to 5 mm in diameter) of light gray hbn-dacite and minor white bt-phyric dacite(?).	Tpf	
905–920	Volcaniclastic sediments—light gray (GLEY1 7/0) to pale pinkish gray (7.5YR 7/1) coarse gravels with medium- to coarse-grained, clasts, predominantly dacitic. 905–920 ft WR/+10F: broken and subangular to subrounded clasts (up to 17 mm in diameter) composed mainly of light gray porphyritic hbn-dacites, minor bt-bearing dacite.	Tpf	
920–940	Volcaniclastic sediments—very pale pinkish gray (7.5YR 7/1) medium to coarse gravels and silty medium- to coarse-grained sandstones, dacite-rich detritus. . 920–935 ft WR: moderately silty matrix. +10F: broken and subangular clasts (up to 10 mm in diameter) composed predominantly of light gray hbn-dacites and minor white bt-bearing dacite. 935–940 ft WR: silty fine sand with gravel. +10F: compositionally similar to 920—935 ft. +35F: abundant fragments very fine-grained silty sandstone.	Tpf	

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 14 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
940–960	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/1) medium to coarse gravels and coarse-grained sandstones, dacite-rich detritus. 940–950 ft +10F: broken and subangular clasts (up to 12 mm in diameter) composed predominantly of light gray porphyritic hbn-dacites and minor bt-bearing dacite. 950–960 ft +10F: broken and subrounded clasts (up to 13 mm in diameter) of hbn- and bt-phyric dacites, minor orange pink rhyodacite(?).		Tpf	
960–985	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/1) fine gravels and fine- to medium-grained sandstone with silt, predominantly dacitic detritus. 960–970 ft WR: moderately silty matrix. +10F: broken and subangular to subrounded clasts (up to 6 mm in diameter) composed predominantly of hbn-dacites and minor bt-bearing dacite, trace vesicular rhyodacite(?), and indurated sandstone fragments. 970–975 ft WR: fine to coarse sand with pebble gravel, silty matrix. +10F: broken and subangular clasts (up to 8 mm in diameter) hbn- and tt-bearing dacites. 975–980 ft WR: fine gravel and fine- to medium-grained sandstone. +10F: subangular to subrounded clasts (up to 10 mm in diameter) composed of light gray, pinkish and white hbn- and bt-bearing dacites.		Tpf	
985–990	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/1) coarse gravels and medium to very coarse-grained sandstone, dacitic detritus. 985–990 ft +10F: subangular to subrounded clasts (up to 18 mm in diameter) composed of light gray hbn-dacites and minor white bt-bearing dacite.		Tpf	
990–1005	Volcaniclastic sediments—pale pinkish gray (7.5YR 7/1) fine- to very coarse grained sandstone with some pebble gravel, dacitic detritus. 990–1005 ft +10F: broken and subangular clasts (up to 15 mm in diameter) composed predominantly of coarsely porphyritic light gray hbn-dacites and lesssr bt-bearing dacite.		Tpf	Estimated contact between Puye volcaniclastic sediments and underlying Miocene pumiceous sediments is placed at 1005 ft bgs.

Borehole Lithologic Log (continued)

Borehole ID: R-44		**TA:** 5	**Page:** 15 of 16	
Depth (ft bgs)	Lithology		Lithologic Symbol	Notes
1005–1010	**MIOCENE PUMICEOUS SEDIMENTS:** Pumiceous volcaniclastic sediments—varicolored light gray (GLEY1 7/0) to white (5YR 8/1) fine to very coarse sand with granules, detritus of mixed pumice, and dacite. 1005–110 ft WR/ +10F: broken and subangular granule-size clasts (up to 7 mm in diameter)composed of 60% pumice fragments (glassy, phenocryst-poor), 40% light gray dacite with minor aphyric rhyolite.		Tjfp	Miocene pumice-rich volcaniclastic sediments, encountered from 1005 to 1088 ft bgs, are estimated to be 83 ft thick.
1010–1020	Pumiceous volcaniclastic sediments—pinkish white (5YR 8/2) fine to coarse sand with pebble gravel and silt, mixed pumice, and dacitic detritus. 1010–1015 ft WR: moderately silty matrix. +10F: 100% pumice fragments that are vitric and phenocryst-poor. +35F: 80%–85% pumices; 15%–20% gray dacite grains. 1015–1020 ft +10F: broken and subangular clasts, 70%–75% white glassy phenocryst-poor pumices (up to 10 mm in diameter); 25%–30% light gray dacite and white rhyolites.		Tifp	
1020–1045	Pumiceous volcaniclastic sediments—varicolored pinkish white (5YR 8/2) to very light gray (GLEY1 7/0) fine (i.e., pebble-size clasts) gravel and very coarse sand, mixed detritus of pumice, and dacite. 1020–1025 ft WR/+10F: 60%–70% white glassy, phenocryst-poor pumice fragments; 10%–20% subangular to subrounded dacite and lesser rhyodacite clasts (up to 13 mm in diameter); 7%–10% fragments of fine- to medium-grained pumiceous sandstone. 1025–1030 ft WR/+10F: 65%–70% white glassy pumice fragments; 15%–20% pink tan pumiceous sandstone fragments; 10%–15% dacite clasts. 1030–1035 ft WR/+10F: 85-95% white glassy phenocryst-poor (rare quartz, biotite) pumice fragments; 5%–10% light pinkish tan pumiceous sandstone fragments; 3%–5% dacitic detritus. 1035–1045 ft WR/+10f: 60%–70% white glassy pumices; 15%–20% pale pink tan pumiceous sandstone fragments, 1%–20% subangular clasts (up to 12 mm in diameter) mixed volcanic detritus (dacite, andesite, basalt).		Tifp	

Borehole Lithologic Log (continued)

Borehole ID: R-44	**TA:** 5	**Page:** 16 of 16	
Depth (ft bgs)	Lithology	Lithologic Symbol	Notes
1045–1075	Pumiceous volcaniclastic sediments—varicolored pinkish white (5YR 8/2) to medium gray (GLEY1 5/0) fine to coarse sand with pebble gravel mixed detritus composed of pumice and a variety of volcanic lithologies. 1045–1055 ft WR/+10F: 50%–60% white vitric pumices (both phenocryst-poor and biotite-bearing varieties present); 30%–40% subangular clasts (up to 10 mm in diameter) mixed andesite and dacite; 3%–5% pink tan fragments pumiceous sandstone. 1055–1065 ft WR/+10F: 40%–50% white vitric pumices; 40%–50% mixed volcanic detritus (dacite, andesite, basalt). 1065–1070 ft WR/+10F: 50-60% white pumices; 30%–40% pinkish tan fine-grained sandstone with abundant pumice grains; 10–15% mixed volcanic detritus. 1070–1075 ft WR: silty matrix. +10F: 50%–60% white pumices; 25%–35% mixed volcanic detritus; 10%–15% pumiceous sandstone fragments.	Tifp	
1075–1088	Pumiceous volcaniclastic sediments—varicolored, white (%YR 8/2), pale pinkish tan (5YR 7/3) and medium gray (GLEY1 5/0) fine to medium gravels with fine to coarse sand, detritus predominantly of dacite and lesser pumices. 1075–1088 ft WR/+10F: 50%–60% broken and subangular clasts (up to 15 mm in diameter) of dacite and minor andesite; 25%–30% white vitric pumice fragments; 10%–20% indurated pumiceous sandstone fragments.	Tifp	Estimated contact between Miocene pumiceous sediments and underlying Miocene riverine sediments is placed at 1088 ft bgs.
1088–1094	**MIOCENE RIVERINE SEDIMENTS:** Axial-river gravel deposits—varicolored medium gray (GLEY1 5/0) to pink tan (5YR 7/4) fine to coarse gravels with fine to coarse sand, commonly rounded detrital clasts composed of diverse volcanic and Precambrian quartzo-feldspathic lithologies. +10F: 20%–30% well-rounded quartzite and granitic pebbles (up to 22 mm in diameter); 70%–80% broken and well rounded volcanic clasts (up to 13 mm in diameter) composed of dark gray fine-grained andesite and varieties of dacite.	Tcar	Miocene riverine gravel deposits were encountered at the bottom of the R-44 borehole through the 6-ft interval, from 1088 to 1094 ft bgs (TD). Note: R-44 borehole drilling was concluded at a TD of 1094 ft bgs.

Borehole Lithologic Log (continued)

ABBREVIATIONS

5YR 8/4 = Munsell rock color notation where hue (e.g., 5YR), value (e.g., 8), and chroma (e.g. 4) are expressed. Hue indicates soil color's relation to red, yellow, green, blue, and purple. Value indicates soil color's lightness. Chroma indicates soil color's strength.

% = estimated per cent by volume of a given sample constituent

bgs = below ground surface

bt = biotite

cpx = clinopyroxene

ft = feet

GM = groundmass

hbn = hornblende

N/S = no assigned symbol for geologic unit

ol = olivine

Qal = Quaternary Alluvium

Qbt 1g = vitric unit 1g of the Tshirege member of Bandelier Tuff

Qct = Cerro Toledo Interval

Qbo = Otowi Member of Bandelier Tuff

Qbog = Guaje Pumice Bed

Tb4 = Cerros del Rio Basalt

Tpf = Puye Formation

Y = Yellow

YR = Yellow red

+10F = plus No. 10 sieve sample fraction

+35F = plus No. 35 sieve sample fraction

Appendix B

Groundwater Analytical Results

B-1.0 SAMPLING AND ANALYSIS OF GROUNDWATER AT R-44

A total of 15 groundwater samples were collected at the regional aquifer well R-44; 11 samples during drilling and 4 samples during well development. Two groundwater samples potentially were collected from the vadose zone and 9 from the regional aquifer during drilling. The two vadose zone samples most likely consist of municipal water used during drilling, based on very small volumes of water produced from the borehole. In addition, low concentrations of key contaminants, including chloride, chromium, nitrate, and sulfate measured in the borehole samples, were not consistent with those measured at wells MCOI-4, MCOI-5, MCOI-6, SCI-1, SCI-2, R-28, and R-42. Perched intermediate-depth groundwater was not encountered during drilling at R-42 and R-28. The two vadose zone water samples were not analyzed for tritium, another key contaminant found in groundwater in Mortandad Canyon. The lack of tritium analysis on the two water samples places some small uncertainty on the occurrence of perched intermediate groundwater within the deep vadose zone at well R-44. During aquifer performance (pumping) testing, six groundwater samples were collected from screen 1 between a depth interval ranging from 895 to 905 ft below ground surface (bgs), and six groundwater samples were collected from screen 2 between a depth interval of 985 and 995 ft bgs. All of the groundwater samples were collected within the Puye Formation. The filtered samples were analyzed for cations, anions, perchlorate, and metals. A total of 16,005 gal. of groundwater was pumped from well R-44 during development before the aquifer tests. During the pumping tests conducted at well R-44, a total of 76,924 gal. of groundwater was pumped from screens 1 and 2.

B-1.1 Field Preparation and Analytical Techniques

Chemical analyses of groundwater-screening samples collected from well R-44 were performed at Los Alamos National Laboratory's (LANL's, or the Laboratory's) Earth and Environmental Sciences Group 14 (EES-14). Groundwater samples were filtered (0.45-µm membranes) before preservation and chemical analyses. Samples were acidified at the EES-14 wet chemistry laboratory with analytical grade nitric acid to a pH of 2.0 or less for metal and major cation analyses.

Groundwater samples were analyzed using techniques specified in the U.S. Environmental Protection Agency SW-846 manual. Ion chromatography (IC) was the analytical method for bromide, chloride, fluoride, nitrate, nitrite, oxalate, perchlorate, phosphate, and sulfate. The instrument detection limits for perchlorate were 0.002 and 0.005 ppm, depending on the sample type (borehole water versus developed well water) and analyte interferences due to the presence of drilling fluid (AQF-2) used during drilling. Inductively coupled (argon) plasma optical emission spectroscopy (ICPOES) was used for analyses of dissolved aluminum, barium, boron, calcium, total chromium, iron, lithium, magnesium, manganese, potassium, silica, sodium, strontium, titanium, and zinc. Dissolved aluminum, antimony, arsenic, barium, beryllium, boron, cadmium, cesium, chromium, cobalt, copper, iron, lead, lithium, manganese, mercury, molybdenum, nickel, rubidium, selenium, silver, thallium, thorium, tin, vanadium, uranium, and zinc were analyzed by inductively coupled (argon) plasma mass spectrometry (ICPMS). The precision limits (analytical error) for major ions and trace elements were generally less than ±7% using ICPOES and ICPMS. Concentrations of total organic carbon (TOC) in nonfiltered groundwater samples collected during well development and aquifer performance testing were determined by using an organic carbon analyzer. Charge balance errors for total cations and anions were generally less than ±10% for complete analyses of the above inorganic chemicals. The negative cation-anion charge balance values indicate excess anions for the filtered samples. Total carbonate alkalinity was measured using standard titration techniques.

B-1.2 Field Parameters

B-1.2.1 Well Development

Water samples were drawn from the pump flow line into sealed containers, and field parameters were measured using a YSI multimeter. Results of field parameters, consisting of pH, temperature, dissolved oxygen (DO), oxidation-reduction potential (ORP), specific conductance, and turbidity measured during well development at R-44, are provided in Table B-1.2.1-1. Seven measurements of pH and temperature varied from 8.22 to 8.30 and from 18.30°C to 18.56°C, respectively, in groundwater pumped from well R-44 screen 1 during development. Concentrations of DO ranged from 9.70 to 10.66 mg/L, and these anomalously high DO measurements suggest that the groundwater was aerated during field parameter measurements. Uncorrected ORP values varied from –135.1 to –129.7 millivolts (mV) during well development of R-44 screen 1 (Table B-1.2.1-1). These ORP measurements taken during well development are not considered to be reliable and representative of the known relatively oxidizing conditions characteristic of the regional aquifer beneath the Pajarito Plateau, based on analytical results for redox-sensitive solutes, including detectable chromium, nitrate, sulfate, and uranium provided in Table B-1.3.1-1. Measurable concentrations of these solutes are consistent with overall oxidizing conditions encountered at the well. Specific conductance ranged from 142 to 148 microsiemens per centimeter (μS/cm), and turbidity ranged from 0 to 0.1 nephelometric turbidity unit (NTU) during well development of R-44 screen 1 (Table B-1.2.1-1).

Thirteen measurements of pH and temperature varied slightly from 8.19 to 8.30 and from 17.47°C to 18.78°C, respectively, in groundwater pumped from well R-44 screen 2 during development (Table B-1.2.1-1). Concentrations of DO varied from 11.57 to 13.72 mg/L, and these anomalously high DO measurements suggest that the groundwater was aerated during field parameter measurements. Uncorrected ORP values varied from –130.8 to –118.9 mV (Table B-1.2.1-1) during well development of R-44 screen 2, which also are not consistent with analytical results for several of the redox-sensitive solutes listed above. The regional aquifer is relatively oxidizing beneath the Pajarito Plateau and positive, uncorrected ORP measurements are typically recorded at adjacent regional aquifer wells, including R-1, R-13, R-15, and R-28. Specific conductance ranged from 193 to 204 μS/cm in groundwater pumped from R-44 screen 2 during well development, and turbidity decreased from 55.8 to 0 NTUs. Eight of the 13 measurements had turbidity greater than 5 NTUs during well development of R-44 screen 2.

B-1.2.2 Aquifer Performance Testing

During aquifer performance testing, 29 measurements of pH and temperature varied from 7.80 to 8.04 and from 14.99°C to 19.08°C, respectively, at well R-44 screen 1 (Table B-1.2.1-1). Concentrations of DO varied from 7.95 to 9.30 mg/L and positive, uncorrected ORP values varied from 117.3 to 204.4 mV during aquifer performance testing of R-44 screen 1. The uncorrected ORP values are generally consistent with both the DO measurements and analytical results for redox-sensitive solutes listed above and are provided in Table B-1.3-1. Specific conductance ranged from 60 to 140 μS/cm, and turbidity varied from 0 to 2.8 NTUs for groundwater pumped from R-44 screen 1 during aquifer performance testing.

Twenty-three measurements of pH and temperature varied from 8.31 to 8.67 and from 15.14°C to 20.31°C, respectively, during aquifer performance testing conducted at well R-44 screen 2. Concentrations of DO ranged from 8.60 to 11.14 mg/L. The anomalously high DO concentrations (greater than 9 mg/L) suggest that the water samples were aerated during parameter measurement. Positive, uncorrected ORP values varied from 144.9 to 212.1 mV during aquifer performance testing of R-44 screen 2. Specific conductance decreased from 173 to 154 μS/cm for the R-44 screen 2 samples measured during aquifer performance testing. Turbidity varied from 1.6 to 5.9 NTUs with one turbidity value greater than 5 NTUs.

B-1.3 Analytical Results for R-44 Groundwater-Screening Samples

B-1.3.1 Well Development

Analytical results for groundwater-screening samples collected at well R-44 during drilling, well development, and aquifer performance testing are provided in Table B-1.3.1-1. Four groundwater samples were collected from R-44 screens 1 and 2 during well development, and selected analytical results for these samples are combined in the following discussion. Calcium and sodium are the dominant cations in regional aquifer groundwater pumped from well R-44. During well development, dissolved concentrations of calcium and sodium ranged from 12.29 to 13.01 ppm (12.29 to 13.01 mg/L) and from 11.61 to 30.05 ppm, respectively. Dissolved concentrations of chloride and fluoride varied from 4.84 to 8.13 ppm and from 0.39 to 0.42 ppm, respectively, during development conducted at well R-44 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) and sulfate ranged from 0.57 to 1.01 ppm and from 5.83 to 13.8 ppm, respectively, during development at well R-44. Dissolved concentrations of chloride, nitrate(N), and sulfate exceeded Laboratory median background for regional aquifer groundwater (LANL 2007, 095817). Median background concentrations for dissolved chloride, nitrate plus nitrite(N), and sulfate in the regional aquifer are 2.17 mg/L, 0.31 mg/L, and 2.83 mg/L, respectively (LANL 2007, 095817). Concentrations of TOC ranged from 0.55 to 0.71 mgC/L in groundwater-screening samples collected during development conducted at well R-44 (Table B-1.3.1-1). The median background concentration of TOC is 0.34 mgC/L for regional aquifer groundwater (LANL 2007, 095817). Concentrations of perchlorate were less than analytical detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 during development (Table B-1.3.1-1).

During well development conducted at R-44, dissolved concentrations of iron ranged from 0.180 to 0.430 ppm (180 to 430 μg/L or 180 to 430 ppb) using ICPOES (Table B-1.3.1-1), which exceeded the maximum background value of 147 μg/L for regional aquifer groundwater (LANL 2007, 095817). Dissolved concentrations of manganese ranged from 0.011 to 0.019 ppm (Table B-1.3.1-1), which exceeded the median background value of 1.0 μg/L for regional aquifer groundwater (LANL 2007, 095817). A carbon-steel discharge pipe was used during well development at R-44, which contributed iron and manganese in the form of colloidal rust to the filtered groundwater samples. Dissolved concentrations of boron ranged from 0.006 to 0.023 ppm (Table B-1.3.1-1) at well R-44, which is below the maximum background value of 51.6 μg/L for the regional aquifer (LANL 2007, 095817). Dissolved concentrations of nickel were less than analytical detection (0.001 ppm, ICPMS method) (Table B-1.3.1-1) in four groundwater-screening samples collected during well development conducted at R-44. Dissolved concentrations of zinc ranged from 0.003 to 0.007 ppm in groundwater-screening samples collected at well R-44 during development (Table B-1.3.1-1). The background median concentration of zinc in filtered samples is 1.45 μg/L for the regional aquifer (LANL 2007, 095817). Total dissolved concentrations of chromium ranged from 0.004 to 0.008 ppm (4 to 8 μg/L) at well R-44 during well development, with the higher concentrations of this metal measured in groundwater samples collected from screen 1 (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 μg/L, 3.05 μg/L, and 7.20 μg/L, respectively, for the regional aquifer (LANL 2007, 095817).

B-1.3-2 Aquifer Performance Testing

Dissolved concentrations of calcium and sodium ranged from 11.54 to 12.0 ppm and from 8.65 to 9.74 ppm, respectively, during aquifer performance testing conducted at R-44 screen 1 (Table B-1.3.1-1). Dissolved concentrations of chloride and fluoride varied from 3.29 to 3.44 ppm and from 0.36 to 0.37 ppm, respectively, during this phase of testing conducted at well R-44 screen 1 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) and sulfate varied slightly from 1.12 to 1.14 ppm and from 4.20 to 4.44 ppm, respectively, during aquifer performance testing performed at well R-44 screen 1. Dissolved concentrations of chloride, nitrate(N), and sulfate in groundwater-screening samples collected from R-44 screen 1 exceeded Laboratory median background within regional aquifer groundwater (LANL 2007, 095817). Median background concentrations for dissolved chloride, nitrate plus nitrite(N), and sulfate in the regional aquifer are 2.17 mg/L, 0.31 mg/L, and 2.83 mg/L, respectively (LANL 2007, 095817). Elevated above-background concentrations of chloride, nitrate(N), and sulfate at well R-44 screen 1 suggest the presence of a contaminant plume(s) consisting, in part, of treated sewage effluent most likely released from Technical Area 03 (TA-03) discharges and possibly from other sewage/industrial waste streams released within Mortandad Canyon. Concentrations of TOC measured in groundwater-screening samples were 0.50 mgC/L during aquifer performance testing conducted at well R-44 screen 1 (Table B-1.3.1-1). Concentrations of perchlorate were less than detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 screen 1 during aquifer performance testing (Table B-1.3.1-1).

During aquifer performance testing at R-44 screen 1, dissolved concentrations of iron were generally less than analytical detection (0.010 ppm) using ICPOES (Table B.1-3-1). A stainless-steel discharge pipe was used during aquifer performance testing conducted at R-44 screens 1 and 2, which is much less corrodible than the carbon steel used during development. Dissolved concentrations of manganese varied slightly from 0.002 to 0.003 ppm (Table B-1.3.1-1 at well R-44 screen 1 during this phase of testing. Dissolved concentrations of boron ranged from 0.013 to 0.018 ppm (Table B-1.3.1-1) in groundwater-screening samples collected from well R-44 screen 1, which is below the maximum background value of 51.6 μg/L for the regional aquifer (LANL 2007, 095817). Dissolved concentrations of nickel were less than analytical detection (0.001 ppm, ICPMS method) (Table B-1.3.1-1) in six groundwater-screening samples collected from R-44 screen 1 during aquifer performance testing. Dissolved concentrations of zinc ranged from 0.005 to 0.0013 ppm in groundwater-screening samples collected from R-44 screen 1 during this phase of testing (Table B-1.3.1-1). The background median concentration of zinc in filtered samples is 1.45 μg/L for the regional aquifer (LANL 2007, 095817). Total dissolved concentrations of chromium were 0.014 ppm (14 μg/L) in six groundwater-screening samples collected from R-44 screen 1 during aquifer performance testing (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 μg/L, 3.05 μg/L, and 7.20 μg/L, respectively, for the regional aquifer (LANL 2007, 095817). The most likely source of dissolved chromium measured in groundwater samples collected from well R-44 screen 1 is from past releases associated with the TA-03 cooling towers, in which potassium dichromate was used as a corrosion inhibitor from 1956 to 1972. Chromate (CrO_4^{2-}) is mobile in groundwater under oxidizing and basic pH conditions characteristic of most perched intermediate saturated zones and the regional aquifer at Los Alamos.

During aquifer performance testing of R-44 screen 2, dissolved concentrations of calcium and sodium ranged from 12.82 to 13.49 ppm and from 11.46 to 15.27 ppm, respectively, which are slightly higher than those measured in groundwater-screening samples collected from R-44 screen 1. Dissolved concentrations of chloride and fluoride varied slightly from 3.62 to 4.31 ppm and from 0.40 to 0.42 ppm, respectively, during aquifer performance testing conducted at well R-44 screen 2 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) varied slightly from 0.60 to 0.62 ppm, which are less than dissolved concentrations of nitrate(N) measured in groundwater-screening samples collected from R-44 screen 1. Dissolved concentrations of sulfate decreased from 7.38 to 4.71 ppm during aquifer performance testing conducted at well R-44 screen 2, which are higher than those measured in groundwater-screening samples collected from R-44 screen 1. Dissolved concentrations of chloride, nitrate(N), and sulfate at well R-44 exceeded Laboratory median background within regional aquifer groundwater (LANL 2007, 095817). Concentrations of TOC were 0.50 mgC/L during aquifer performance testing conducted at well R-44 screen 2 (Table B-1.3.1-1). Concentrations of perchlorate were less than detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 screen 2 during aquifer performance testing (Table B-1.3.1-1).

During aquifer performance testing conducted at R-44 screen 2, dissolved concentrations of iron were generally less than analytical detection (0.010 ppm) using ICPOES (Table B-1.3.1-1). Dissolved concentrations of manganese varied slightly from 0.007 to 0.008 ppm (Table B-1.3.1-1) at well R-44 screen 2. Dissolved concentrations of boron ranged from 0.014 to 0.020 ppm (Table B-1.3.1-1) at well R-44 screen 2, which is below the maximum background value of 51.6 μg/L for the regional aquifer (LANL 2007, 095817). Dissolved concentrations of boron are similar in groundwater-screening samples collected from both screens at R-44 (Table B-1.3.1-1). Detectable dissolved concentrations of nickel were 0.002 ppm in groundwater-screening samples collected from R-44 screen 2 during aquifer performance testing (Table B-1.3.1-1). Dissolved concentrations of zinc varied slightly from 0.005 to 0.006 ppm in groundwater-screening samples collected from R-44 screen 2 during aquifer performance testing (Table B-1.3.1-1). Total dissolved concentrations of chromium ranged from 0.004 to 0.006 ppm (4 to 6 μg/L) at well R-44 screen 2 (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 μg/L, 3.05 μg/L, and 7.20 μg/L, respectively, for the regional aquifer (LANL 2007, 095817). Total dissolved concentrations of chromium are lower in groundwater-screening samples collected from screen 2 compared with those pumped from screen 1 at well R-44.

B-2.0 REFERENCES

The following list includes all documents cited in this appendix. Parenthetical information following each reference provides the author(s), publication date, and ER ID number. This information is also included in text citations. ER ID numbers are assigned by the Environmental Programs Directorate's Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

LANL (Los Alamos National Laboratory), May 2007. "Groundwater Background Investigation Report, Revision 3," Los Alamos National Laboratory document LA-UR-07-2853, Los Alamos, New Mexico. (LANL 2007, 095817)

Table B-1.2.1-1
Well Development Volumes, Aquifer Pump Test Volumes, and Associated Field Water-Quality Parameters for R-44

Date	pH	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
Well Development								
01/15/09	n/r*, bailing						500	500
01/16/09	n/r, bailing						300	800
01/17/09	n/r, pumping						3000	3800
01/18/09	n/r, pumping						1785	5585
01/18/09 (upper screen)	8.30	18.30	10.66	–133.6	148	0.1	1150	6735
	8.27	18.33	9.88	–133.7	145	0.0	235	6970
	8.26	18.35	10.12	–133.0	144	0.0	235	7205
	8.26	18.42	11.02	–132.9	144	0.0	235	7440
	8.25	18.56	9.77	–135.1	144	0.0	235	7675
	8.23	18.47	9.70	–133.3	144	0.0	235	7910
	8.22	18.48	10.30	–129.7	142	0.0	235	8145
01/19/09	n/r, pumping						2340	10,485
01/20/09	n/r, pumping						2610	13,095
01/20/09 (lower screen)	8.29	17.47	13.65	–125.2	204	55.8	664	13,759
	8.29	17.49	13.72	–118.9	204	43.2	246	14,005
	8.26	17.55	12.96	–121.4	201	35.9	248	14,253
	8.26	17.63	12.84	–120.8	199	15.4	248	14,501
	8.25	17.67	12.91	–120.2	198	22.2	248	14,749
	8.22	17.75	12.50	–122.5	195	14.8	248	14,997
	8.20	18.03	11.94	–129.9	195	7.2	144	15,141
	8.20	18.45	12.07	–130.6	196	7.1	144	15,285
	8.22	18.58	12.00	–130.5	195	4.7	144	15,429
	8.23	18.65	11.64	–130.8	195	1.2	144	15,573
	8.22	18.68	12.03	–130.4	194	0.0	144	15,717
	8.20	18.75	11.57	–130.2	193	0.0	144	15,861
	8.19	18.78	12.48	–129.9	193	0.0	144	16,005

Table B-1.2.1-1 (Continued)

Date	pH	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
Aquifer Pumping Test Volumes								
02/14/09	n/r, pumping, step-test upper screen						3312	3312
02/15/09	n/r, pumping, step-test upper screen						145	3457
02/16–17/09 (upper screen)	7.80	17.94	8.59	121.0	140	0.7	724	4181
	8.04	16.50	8.78	117.3	130	0.2	724	4905
	7.97	16.67	9.00	158.9	130	0.0	724	5629
	7.96	17.27	8.81	156.7	130	0.1	725	6354
	7.89	18.55	8.96	162.6	70	0.3	1448	7802
	7.98	18.72	8.67	170.8	130	0.5	1449	9251
	7.97	17.18	8.71	168.7	130	0.4	1448	10,699
	8.00	17.65	8.90	171.5	130	0.4	1449	12,148
	8.01	16.42	9.18	189.7	130	0.5	1448	13,596
	7.94	15.65	8.81	178.9	130	0.1	1449	15,045
	7.96	14.99	9.07	183.6	130	0.2	1448	16,493
	8.00	17.25	8.59	189.0	130	0.1	1449	17,942
	7.98	17.18	9.30	184.1	130	1.1	1448	19,390
	7.97	17.01	8.35	204.4	130	0.1	5794	25,184
	7.98	17.12	9.29	199.6	130	0.2	1448	26,632
	7.99	16.85	8.25	194.4	60	0.2	724	27,356
	8.03	17.69	8.17	195.2	120	2.8	724	28,080
	7.99	16.15	8.56	192.9	60	0.3	1448	29,528
	7.98	17.21	8.32	193.7	110	0.2	1449	30,977
	8.01	17.16	8.26	192.6	130	0.1	1448	32,425
	7.98	18.37	8.16	151.3	120	0.4	1449	33,874
	8.02	18.86	8.03	161.9	130	0.5	1448	35,322
	8.01	18.41	8.01	169.7	120	0.4	1449	36,771
	8.02	18.79	8.18	179.4	67	0.1	242	37013
	8.02	18.68	8.14	184.9	130	0.2	242	37,255
	8.01	18.65	7.98	188.4	130	0.2	242	37,497
	8.03	18.70	7.95	188.9	130	0.1	242	37,739
	8.03	18.55	8.04	189.9	130	0.1	242	37,981
	8.01	19.08	8.16	189.4	130	0.2	242	38,223
02/19/09	n/r, pumping, step-tests lower screen						4275	42,498
02/21–22/09 (lower screen)	8.31	17.47	8.72	145.7	173	5.0	956	43,454
	8.35	15.14	10.64	157.6	167	4.3	478	43,932
	8.67	18.72	8.90	144.9	173	3.1	1434	45,366
	8.66	19.50	10.38	148.7	171	4.0	1434	46,800
	8.65	19.41	8.79	149.0	168	5.9	1434	48,234

Table B-1.2.1-1 (Continued)

Date	pH	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
	8.62	19.60	9.05	180.7	167	4.7	1434	49,668
	8.62	19.84	9.06	165.6	166	3.0	1434	51,102
	8.63	20.31	8.60	166.8	164	3.1	1434	52,536
	8.61	19.96	9.70	167.1	163	3.5	1434	53,970
	8.60	19.06	9.34	163.2	161	4.7	1434	55,404
	8.67	16.59	9.26	176.2	156	2.4	1434	56,838
	8.63	17.12	9.12	179.9	157	1.8	1434	58,272
	8.64	17.81	11.14	182.4	152	4.0	4302	62,574
	8.62	17.82	9.55	187.4	158	4.0	1434	64,008
	8.60	17.66	10.45	192.4	157	3.8	1434	65,442
	8.57	18.96	10.47	161.2	156	2.5	1434	66,876
	8.58	18.56	10.77	189.3	157	3.5	1434	68,310
	8.56	n/r	9.18	191.2	156	2.2	1434	69,744
	8.60	n/r	10.11	192.9	155	2.1	1434	71,178
	8.62	n/r	10.32	192.9	157	3.0	1434	72,612
	8.63	n/r	8.73	194.8	154	1.7	1434	74,046
	8.61	19.39	8.47	212.1	155	1.6	1434	75,480
	8.62	19.27	8.62	197.8	155	1.6	1195	76,675
	n/r						249	76,924

Note: Cumulative purge volumes for pump test calculated using average pump discharge rate of 24.1gal./min in the upper screen and 23.9 1 gal./min in the lower screen.

* n/r = Not recorded.

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	ER/RRES-WQH	Screen	Depth (ft)	Ag rslt (ppm)	stdev (Ag)	Al rslt (ppm)	stdev (Al)	As rslt (ppm)	stdev (As)	B rslt (ppm)	stdev (B)	Ba rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	09-329	Not applicable	739	0.001	U	0.21	0.00	0.0011	0.0000	0.038	0.001	0.015
GW44-09-1293	12/8/2008	Not applicable	Borehole	09-453	Not applicable	920	0.001	U	0.43	0.00	0.0005	0.0000	0.079	0.001	0.456
GW44-09-1294	12/8/2008	Not applicable	Borehole	09-453	Not applicable	937	0.001	U	0.42	0.00	0.0012	0.0000	0.088	0.001	0.453
GW44-09-1295	12/8/2008	Not applicable	Borehole	09-453	Not applicable	977	0.001	U	0.30	0.00	0.0003	0.0000	0.093	0.001	0.383
GW44-09-1296	12/8/2008	Not applicable	Borehole	09-453	Not applicable	997	0.001	U	1.39	0.01	0.0018	0.0002	0.076	0.001	0.418
GW44-09-1297	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1017	0.001	U	1.54	0.06	0.0011	0.0002	0.118	0.004	0.509
GW44-09-1298	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1037	0.001	U	0.98	0.02	0.0007	0.0000	0.076	0.002	0.386
GW44-09-1299	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1056	0.001	U	0.93	0.00	0.0009	0.0000	0.073	0.001	0.271
GW44-09-1300	12/9/2008	Not applicable	Borehole	09-473	Not applicable	1094	0.001	U	0.02	0.00	0.0011	0.0000	0.070	0.001	0.434
GW44-09-1301	12/9/2008	Not applicable	Borehole	09-473	Not applicable	1076	0.001	U	0.29	0.01	0.0004	0.0000	0.147	0.001	0.536
GW44-09-1272	1/20/2009	Not applicable	Well, development	09-658	1	895-905	0.001	U	0.004	0.000	0.0008	0.0000	0.009	0.000	0.031
GW44-09-1273	1/20/2009	Not applicable	Well, development	09-658	1	895-905	0.001	U	0.004	0.000	0.0007	0.0000	0.006	0.001	0.027
GW44-09-1274	1/20/2009	Not applicable	Well, development	09-679	2	985-995	0.001	U	0.022	0.000	0.0016	0.0000	0.023	0.001	0.056
GW44-09-1275	1/20/2009	Not applicable	Well, development	09-679	2	985-995	0.001	U	0.006	0.000	0.0013	0.0000	0.017	0.000	0.048
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0008	0.0000	0.014	0.000	0.025
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.004	0.000	0.0007	0.0000	0.018	0.001	0.024
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.007	0.000	0.0007	0.0000	0.016	0.001	0.025
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0008	0.0000	0.014	0.000	0.024
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.006	0.000	0.0008	0.0000	0.014	0.000	0.023
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0007	0.0000	0.013	0.001	0.022
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0010	0.0000	0.020	0.001	0.031
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.011	0.000	0.0009	0.0000	0.018	0.000	0.029
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.016	0.000	0.028
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.015	0.001	0.028
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.006	0.000	0.0009	0.0000	0.014	0.000	0.027
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.014	0.000	0.027

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-density polyethylene containers.

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Ba)	Be rslt (ppm)	stdev (Be)	Br(-) ppm	TOC rslt (ppm)	TOC (U)	Ca rslt (ppm)	stdev (Ca)	Cd rslt (ppm)	stdev (Cd)	Cl(-) ppm	ClO4(-) ppm	ClO4(-) (U)	Co rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.000	0.001	U	0.15	Not analyzed		13.77	0.08	0.001	U	7.07	0.005	U	0.001
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.05	Not analyzed		14.46	0.10	0.001	U	7.89	0.005	U	0.001
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.006	0.001	U	0.04	Not analyzed		14.79	0.17	0.001	U	7.24	0.005	U	0.001
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.003	0.001	U	0.05	Not analyzed		10.59	0.05	0.001	U	5.30	0.005	U	0.001
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.05	Not analyzed		11.64	0.01	0.001	U	7.87	0.005	U	0.001
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.001	0.001	U	0.03	Not analyzed		11.83	0.04	0.001	U	4.62	0.002	U	0.001
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		11.62	0.09	0.001	U	3.65	0.002	U	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.001	0.001	U	0.04	Not analyzed		11.57	0.05	0.001	U	3.76	0.002	U	0.001
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		21.75	0.07	0.001	U	3.94	0.005	U	0.001
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		9.13	0.04	0.001	U	4.62	0.005	U	0.001
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.55		13.01	0.04	0.001	U	5.05	0.002	U	0.001
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.66		12.95	0.02	0.001	U	4.84	0.002	U	0.001
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.70		12.29	0.04	0.001	U	8.13	0.002	U	0.001
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	0.001	U	0.06	0.71		12.84	0.14	0.001	U	7.39	0.002	U	0.001
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	11.78	0.04	0.001	U	3.44	0.002	U	0.001
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.000	0.001	U	0.04	0.50	U	11.54	0.03	0.001	U	3.35	0.002	U	0.001
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	11.82	0.06	0.001	U	3.34	0.002	U	0.001
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	12.00	0.13	0.001	U	3.33	0.002	U	0.001
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	0.001	U	0.03	0.50	U	11.71	0.10	0.001	U	3.29	0.002	U	0.001
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.000	0.001	U	0.02	0.50	U	11.69	0.05	0.001	U	3.30	0.002	U	0.001
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	12.82	0.11	0.001	U	4.31	0.002	U	0.001
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.11	0.08	0.001	U	4.09	0.002	U	0.001
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.24	0.02	0.001	U	3.91	0.002	U	0.001
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.33	0.10	0.001	U	3.83	0.002	U	0.001
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.40	0.05	0.001	U	3.62	0.002	U	0.001
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.49	0.09	0.001	U	3.65	0.002	U	0.001

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Co)	Alk-CO3 rslt (ppm)	ALK-CO3 (U)	Cr rslt (ppm)	stdev (Cr)	Cs rslt (ppm)	stdev (Cs)	Cu rslt (ppm)	stdev (Cu)	F(-) ppm	Fe rslt (ppm)	stdev (Fe)
GW44-09-1292	11/17/2008	Not applicable	Borehole	U	0.8	U	0.005	0.000	0.001	U	0.003	0.000	0.3	0.54	0.01
GW44-09-1293	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.002	0.000	0.79	0.51	0.00
GW44-09-1294	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.001	0.000	1.08	0.23	0.00
GW44-09-1295	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.001	U	0.66	0.18	0.00
GW44-09-1296	12/8/2008	Not applicable	Borehole	U	0.8	U	0.003	0.001	0.001	U	0.003	0.001	0.81	0.83	0.03
GW44-09-1297	12/8/2008	Not applicable	Borehole	U	0.8	U	0.002	0.000	0.001	U	0.002	0.000	0.81	0.68	0.23
GW44-09-1298	12/8/2008	Not applicable	Borehole	U	0.8	U	0.005	0.001	0.001	U	0.002	0.000	0.44	2.59	0.03
GW44-09-1299	12/8/2008	Not applicable	Borehole	U	0.8	U	0.002	0.000	0.001	U	0.002	0.000	0.63	1.13	0.02
GW44-09-1300	12/9/2008	Not applicable	Borehole	U	6.89		0.009	0.001	0.001	U	0.001	U	0.67	0.02	0.00
GW44-09-1301	12/9/2008	Not applicable	Borehole	U	0.8	U	0.006	0.000	0.001	U	0.001	U	0.69	0.20	0.00
GW44-09-1272	1/20/2009	Not applicable	Well, development	U	0.8	U	0.008	0.000	0.001	U	0.001	U	0.39	0.24	0.00
GW44-09-1273	1/20/2009	Not applicable	Well, development	U	0.8	U	0.008	0.000	0.001	U	0.001	U	0.39	0.23	0.00
GW44-09-1274	1/20/2009	Not applicable	Well, development	U	0.8	U	0.004	0.001	0.001	U	0.001	U	0.42	0.18	0.00
GW44-09-1275	1/20/2009	Not applicable	Well, development	U	0.8	U	0.004	0.001	0.001	U	0.001	U	0.41	0.43	0.01
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	0.00
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.37	0.01	U
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.001	0.001	U	0.001	U	0.37	0.01	U
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.40	0.01	U
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.41	0.03	0.00
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	U	0.8	U	0.004	0.000	0.001	U	0.001	U	0.42	0.01	U
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	U	0.8	U	0.005	0.001	0.001	U	0.001	U	0.41	0.01	U
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	U	0.8	U	0.006	0.000	0.001	U	0.001	U	0.40	0.01	U
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.41	0.01	0.00

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	Alk-CO3+HCO3 rslt (ppm)	Hg rslt (ppm)	stdev (Hg)	K rslt (ppm)	stdev (K)	Li rslt (ppm)	stdev (Li)	Mg rslt (ppm)	stdev (Mg)	Mn rslt (ppm)	stdev (Mn)
GW44-09-1292	11/17/2008	Not applicable	Borehole	128	0.00005	U	2.12	0.03	0.030	0.001	4.38	0.06	0.059	0.000
GW44-09-1293	12/8/2008	Not applicable	Borehole	85	0.00108	0.00004	2.71	0.01	0.037	0.000	4.58	0.01	0.198	0.001
GW44-09-1294	12/8/2008	Not applicable	Borehole	99	0.00210	0.00004	3.05	0.06	0.036	0.001	4.51	0.06	0.080	0.006
GW44-09-1295	12/8/2008	Not applicable	Borehole	68	0.00017	0.00000	2.06	0.02	0.035	0.000	3.31	0.03	0.075	0.002
GW44-09-1296	12/8/2008	Not applicable	Borehole	85	0.00062	0.00003	2.35	0.02	0.032	0.000	4.72	0.04	0.044	0.007
GW44-09-1297	12/8/2008	Not applicable	Borehole	88	0.00299	0.00012	2.31	0.01	0.048	0.000	4.28	0.03	0.033	0.004
GW44-09-1298	12/8/2008	Not applicable	Borehole	81	0.00012	0.00001	1.66	0.01	0.028	0.000	4.04	0.03	0.090	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	91	0.00078	0.00002	3.51	0.02	0.051	0.000	4.53	0.02	0.067	0.002
GW44-09-1300	12/9/2008	Not applicable	Borehole	138	0.00148	0.00002	2.11	0.01	0.048	0.003	6.32	0.05	0.008	0.001
GW44-09-1301	12/9/2008	Not applicable	Borehole	83	0.00173	0.00003	1.59	0.00	0.039	0.002	3.27	0.01	0.028	0.001
GW44-09-1272	1/20/2009	Not applicable	Well, development	84	0.00005	U	1.20	0.02	0.023	0.000	3.62	0.08	0.013	0.000
GW44-09-1273	1/20/2009	Not applicable	Well, development	83	0.00005	U	1.08	0.02	0.021	0.000	3.36	0.04	0.011	0.000
GW44-09-1274	1/20/2009	Not applicable	Well, development	113	0.00005	U	1.68	0.01	0.030	0.000	4.23	0.02	0.018	0.000
GW44-09-1275	1/20/2009	Not applicable	Well, development	107	0.00005	U	1.56	0.03	0.028	0.001	4.25	0.08	0.019	0.000
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	82	0.00005	U	1.07	0.01	0.021	0.000	3.37	0.02	0.003	0.000
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	78	0.00005	U	1.07	0.01	0.021	0.000	3.31	0.03	0.002	0.000
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	77	0.00005	U	1.07	0.01	0.021	0.000	3.38	0.02	0.002	0.000
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	78	0.00005	U	1.09	0.01	0.022	0.000	3.45	0.02	0.002	0.000
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	81	0.00005	U	1.05	0.01	0.021	0.000	3.32	0.01	0.001	0.000
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	79	0.00005	U	1.02	0.00	0.020	0.000	3.23	0.02	0.001	0.000
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	91	0.00005	U	1.35	0.00	0.022	0.000	3.66	0.02	0.008	0.000
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	88	0.00005	U	1.35	0.01	0.022	0.000	3.80	0.03	0.008	0.000
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	87	0.00005	U	1.33	0.01	0.022	0.000	3.85	0.02	0.007	0.000
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	87	0.00005	U	1.33	0.01	0.022	0.000	3.89	0.02	0.007	0.000
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	86	0.00005	U	1.29	0.01	0.022	0.000	3.87	0.02	0.007	0.000
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	85	0.00005	U	1.30	0.01	0.022	0.000	3.91	0.00	0.007	0.000

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	Mo rslt (ppm)	stdev (Mo)	Na rslt (ppm)	stdev (Na)	Ni rslt (ppm)	stdev (Ni)	NO2(ppm)	NO2-N rslt	NO2-N (U)	NO3 ppm	NO3-N rslt	C2O4 rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.069	0.000	26.96	0.27	0.003	0.000	0.04	0.01	0.001	0.46	0.10	0.08
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.238	0.002	16.53	0.07	0.002	0.000	0.01	0.00	U	6.36	1.44	0.58
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.243	0.002	17.32	0.20	0.001	0.000	0.01	0.00	U	3.64	0.82	0.52
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.145	0.001	13.78	0.06	0.001	0.000	0.01	0.00	U	7.22	1.63	0.44
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.052	0.002	13.33	0.06	0.002	0.000	0.01	0.00	U	6.38	1.44	0.61
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.049	0.001	14.85	0.07	0.001	0.000	0.01	0.00	U	2.73	0.62	0.26
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.050	0.001	12.27	0.03	0.003	0.000	0.01	0.00	U	2.04	0.46	0.14
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.065	0.000	14.70	0.03	0.002	0.000	0.01	0.00	U	2.18	0.49	0.02
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.069	0.000	14.85	0.09	0.001	U	0.01	0.00	U	1.93	0.44	0.27
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.097	0.000	15.48	0.07	0.001	U	0.01	0.00	U	2.00	0.45	0.31
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.001	U	13.04	0.22	0.001	U	0.01	0.00	U	4.38	0.99	0.01
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.001	U	11.61	0.15	0.001	U	0.01	0.00	U	4.48	1.01	0.01
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.001	U	30.05	0.09	0.001	U	0.01	0.00	U	2.51	0.57	0.01
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	U	23.86	0.29	0.001	U	0.01	0.00	U	2.54	0.57	0.01
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.001	U	9.74	0.05	0.001	U	0.01	0.00	U	4.99	1.13	0.01
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.001	U	9.43	0.07	0.001	U	0.01	0.00	U	4.97	1.12	0.01
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.001	U	9.39	0.08	0.001	U	0.01	0.00	U	5.04	1.14	0.01
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.001	U	9.45	0.11	0.001	U	0.01	0.00	U	5.04	1.14	0.01
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	U	9.04	0.02	0.001	U	0.01	0.00	U	5.00	1.13	0.01
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.001	U	8.65	0.05	0.001	U	0.01	0.00	U	5.08	1.15	0.01
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.001	0.000	15.27	0.02	0.002	0.000	0.01	0.00	U	2.67	0.60	0.01
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.001	0.000	13.99	0.18	0.002	0.000	0.01	0.00	U	2.69	0.61	0.01
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.001	U	12.63	0.05	0.001	U	0.01	0.00	U	2.69	0.61	0.01
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.001	U	12.42	0.14	0.001	U	0.01	0.00	U	2.70	0.61	0.01
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.001	U	11.62	0.12	0.001	U	0.01	0.00	U	2.66	0.60	0.01
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.001	U	11.46	0.04	0.002	0.000	0.01	0.00	U	2.73	0.62	0.01

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	C2O4 (U)	Pb rslt (ppm)	stdev (Pb)	Lab pH	PO4(-3) rslt (ppm)	Rb rslt (ppm)	stdev (Rb)	Sb rslt (ppm)	stdev (Sb)	Se rslt (ppm)	stdev (Se)	Si rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole		0.0002	U	7.08	0.01, U	0.003	0.000	0.001	U	0.001	U	38.0
GW44-09-1293	12/8/2008	Not applicable	Borehole		0.0002	0.0000	7.90	0.03	0.002	0.000	0.001	U	0.001	U	22.9
GW44-09-1294	12/8/2008	Not applicable	Borehole		0.0002	U	8.03	0.03	0.002	0.000	0.001	U	0.001	U	21.7
GW44-09-1295	12/8/2008	Not applicable	Borehole		0.0002	U	7.66	0.07	0.001	U	0.001	U	0.001	U	12.0
GW44-09-1296	12/8/2008	Not applicable	Borehole		0.0009	0.0001	7.92	0.03	0.004	0.001	0.001	U	0.001	U	35.4
GW44-09-1297	12/8/2008	Not applicable	Borehole		0.0012	0.0002	7.93	0.07	0.003	0.000	0.001	U	0.001	U	32.4
GW44-09-1298	12/8/2008	Not applicable	Borehole		0.0007	0.0002	7.70	0.08	0.002	0.000	0.001	U	0.001	U	28.3
GW44-09-1299	12/8/2008	Not applicable	Borehole		0.0016	0.0000	7.70	0.05	0.006	0.000	0.001	U	0.001	U	30.1
GW44-09-1300	12/9/2008	Not applicable	Borehole		0.0002	U	8.25	0.03	0.001	U	0.001	U	0.001	U	19.8
GW44-09-1301	12/9/2008	Not applicable	Borehole		0.0002	U	7.95	0.17	0.001	U	0.001	U	0.001	U	20.4
GW44-09-1272	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.57	0.01, U	0.002	0.000	0.001	U	0.001	U	32.5
GW44-09-1273	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.56	0.01, U	0.002	0.000	0.001	U	0.001	U	30.2
GW44-09-1274	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.83	0.08	0.002	0.000	0.001	U	0.001	U	37.1
GW44-09-1275	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.82	0.01, U	0.002	0.000	0.001	U	0.001	U	35.9
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	U	0.0002	U	7.78	0.07	0.002	0.000	0.001	U	0.001	U	33.1
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	U	0.0002	U	7.62	0.08	0.002	0.000	0.001	U	0.001	U	32.8
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	U	0.0002	U	7.66	0.06	0.002	0.000	0.001	U	0.001	U	33.3
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	U	0.0002	U	7.75	0.08	0.002	0.000	0.001	U	0.001	U	33.9
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	U	0.0002	U	7.76	0.08	0.002	0.000	0.001	U	0.001	U	32.8
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	U	0.0002	U	7.78	0.09	0.002	0.000	0.001	U	0.001	U	32.0
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	U	0.0002	U	7.75	0.04	0.002	0.000	0.001	U	0.001	U	33.8
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	U	0.0002	U	7.75	0.06	0.002	0.000	0.001	U	0.001	U	34.9
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	U	0.0002	U	7.79	0.07	0.002	0.000	0.001	U	0.001	U	34.6
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	U	0.0002	U	7.80	0.06	0.002	0.000	0.001	U	0.001	U	35.0
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	U	0.0002	U	7.79	0.07	0.002	0.000	0.001	U	0.001	U	34.4
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	U	0.0002	U	7.81	0.06	0.002	0.000	0.001	U	0.001	U	34.8

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Si)	SiO2 rslt (ppm)	stdev (SiO2)	Sn rslt (ppm)	stdev (Sn)	SO4(-2) rslt (ppm)	Sr rslt (ppm)	stdev (Sr)	Th rslt (ppm)	stdev (Th)	Ti rslt (ppm)	stdev (Ti)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.4	81.3	0.9	0.001	U	4.93	0.062	0.001	0.001	U	0.016	0.000
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.1	49.0	0.3	0.001	U	6.63	0.067	0.001	0.001	U	0.032	0.000
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.4	46.5	0.8	0.001	U	7.66	0.061	0.001	0.001	U	0.014	0.001
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.1	25.6	0.2	0.001	U	4.46	0.042	0.000	0.001	U	0.009	0.000
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.0	75.7	0.1	0.001	U	6.65	0.052	0.000	0.001	U	0.038	0.000
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.1	69.3	0.3	0.001	U	3.60	0.048	0.000	0.001	U	0.052	0.000
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.2	60.6	0.4	0.001	U	2.88	0.044	0.000	0.001	U	0.031	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.6	64.4	1.2	0.001	U	3.70	0.053	0.000	0.001	U	0.056	0.000
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.1	42.4	0.3	0.001	U	4.90	0.081	0.001	0.001	U	0.002	U
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.2	43.7	0.4	0.001	U	3.28	0.036	0.000	0.001	U	0.007	0.000
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.4	69.6	0.9	0.001	U	6.07	0.058	0.001	0.001	U	0.002	U
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.5	64.6	1.1	0.001	U	5.83	0.053	0.001	0.001	U	0.002	U
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.4	79.4	0.8	0.001	U	13.8	0.089	0.000	0.001	U	0.003	0.000
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.6	76.7	1.2	0.001	U	11.5	0.083	0.001	0.001	U	0.002	U
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.4	70.8	0.8	0.001	U	4.44	0.052	0.000	0.001	U	0.002	U
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.2	70.3	0.3	0.001	U	4.31	0.050	0.000	0.001	U	0.002	U
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.2	71.2	0.5	0.001	U	4.33	0.051	0.001	0.001	U	0.002	U
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.3	72.6	0.7	0.001	U	4.45	0.052	0.001	0.001	U	0.002	U
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.1	70.2	0.2	0.001	U	4.20	0.050	0.000	0.001	U	0.002	U
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.2	68.4	0.4	0.001	U	4.25	0.048	0.000	0.001	U	0.002	U
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.1	72.4	0.3	0.001	U	7.38	0.065	0.001	0.001	U	0.002	U
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.2	74.8	0.5	0.001	U	6.32	0.063	0.000	0.001	U	0.002	U
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.2	74.0	0.4	0.001	U	5.74	0.062	0.000	0.001	U	0.002	U
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.4	74.9	0.8	0.001	U	5.25	0.061	0.000	0.001	U	0.002	U
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.3	73.5	0.7	0.001	U	4.79	0.059	0.001	0.001	U	0.002	U
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.1	74.6	0.3	0.001	U	4.71	0.058	0.000	0.001	U	0.002	U

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Table B-1.3.1-1
Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	Tl rslt (ppm)	stdev (Tl)	U rslt (ppm)	stdev (U)	V rslt (ppm)	stdev (V)	Zn rslt (ppm)	stdev (Zn)	TDS (ppm)	Cations	Anions	Balance
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.001	U	0.0003	0.0000	0.004	0.000	0.003	0.000	272.0	2.28	2.50	-0.05
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.001	U	0.0011	0.0001	0.002	0.000	0.046	0.000	196.7	1.91	1.93	-0.01
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.001	U	0.0013	0.0000	0.002	0.000	0.026	0.000	207.8	1.96	2.15	-0.05
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.001	U	0.0003	0.0000	0.002	0.000	0.037	0.000	143.1	1.47	1.54	-0.03
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.001	U	0.0024	0.0003	0.006	0.001	0.041	0.002	218.0	1.62	1.95	-0.09
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.001	U	0.0014	0.0002	0.005	0.001	0.047	0.002	206.7	1.67	1.80	-0.04
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.001	U	0.0005	0.0001	0.004	0.001	0.052	0.001	185.1	1.50	1.59	-0.03
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.001	U	0.0010	0.0000	0.003	0.000	0.017	0.002	203.3	1.69	1.79	-0.03
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.001	U	0.0034	0.0001	0.006	0.000	0.020	0.000	244.7	2.32	2.78	-0.09
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.001	U	0.0007	0.0000	0.002	0.000	0.009	0.000	168.6	1.45	1.66	-0.07
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.001	U	0.0008	0.0000	0.005	0.000	0.004	0.001	202.0	1.55	1.77	-0.07
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.001	U	0.0008	0.0000	0.004	0.000	0.003	0.001	193.0	1.46	1.74	-0.09
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.001	U	0.0019	0.0000	0.007	0.000	0.006	0.001	267.0	2.32	2.46	-0.03
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	U	0.0016	0.0000	0.007	0.000	0.007	0.002	250.0	2.08	2.29	-0.05
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.013	0.001	193.0	1.32	1.66	-0.11
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.010	0.001	188.0	1.29	1.59	-0.10
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.009	0.001	188.0	1.31	1.58	-0.10
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.006	0.000	0.008	0.002	190.0	1.33	1.59	-0.09
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.007	0.002	190.0	1.28	1.63	-0.12
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.005	0.000	186.0	1.26	1.60	-0.12
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.001	U	0.0009	0.0000	0.007	0.000	0.006	0.001	212.0	1.65	1.85	-0.06
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.001	U	0.0008	0.0000	0.006	0.000	0.006	0.000	210.0	1.62	1.79	-0.05
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.001	U	0.0008	0.0000	0.006	0.000	0.005	0.001	205.0	1.57	1.74	-0.05
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.001	U	0.0008	0.0000	0.007	0.000	0.006	0.001	206.0	1.57	1.73	-0.05
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.001	U	0.0007	0.0000	0.007	0.000	0.005	0.002	202.0	1.53	1.70	-0.05
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.001	U	0.0007	0.0000	0.006	0.000	0.006	0.001	203.0	1.53	1.69	-0.05

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

Appendix C

Aquifer Testing Report

C-1.0 INTRODUCTION

This appendix describes the hydraulic analysis of pumping tests at well R-44 screens 1 and 2 located in Mortandad Canyon near the edge of the existing chromium plume beneath the canyon. The tests were conducted in conjunction with testing of nearby well R-45 screens 1 and 2. The primary objective of the analysis was to determine the hydraulic properties of the zones screened in R-44, as well as the intervening sediments between the two screen zones. A secondary objective was to look for cross-connection between R-44 and surrounding wells R-45, R-11, R-13, and R-28.

Testing consisted primarily of constant-rate pumping tests conducted on R-44 screens 1 and 2. During the tests on each screen, water levels were monitored in the nonpumped screen zone in R-44 to examine the properties of the intervening sediments, and in R-45 screens 1 and 2 to monitor cross-connection between the wells. In addition, water levels were monitored in adjacent wells R-11, R-13, and R-28.

Consistent with most of the R-well pumping tests conducted on the plateau, an inflatable packer system was used in R-44 to isolate the screens and eliminate the effects of casing storage on the test data.

Conceptual Hydrogeology

R-44 is a dual-screen well completed in the Puye Formation just above the Miocene pumiceous deposits, with 10 ft of screen from 895.0 to 905.0 ft below ground surface (bgs) (screen 1)] and 9.9 ft of screen from 985.3 to 995.2 ft bgs (screen 2); the screens are separated by 80.3 ft of intervening sediments. The composite static water level measured on February 13 at the onset of testing was 878.86 ft bgs. When the zones were isolated with inflatable packers, the water level in screen 1 rose 0.06 ft to 878.80 ft bgs, while the level in screen 2 dropped 0.14 ft to 879.00 ft bgs. Thus, the initial water level in screen 1 was 0.2 ft higher than that in screen 2, implying a downward gradient. The head difference between the two screen zones in R-44 was modest (0.0022 ft downward gradient from the center of screen 1 to the center of screen 2) compared with differences measured at other multiscreen wells on the plateau, which show head differences of feet or tens of feet in most cases. The brass cap elevation at R-44 is 6714.91 ft above mean sea level (amsl), making the approximate static water-level elevations in screens 1 and 2 5836 ft.

Well R-45, also a dual-screen well, is located about 1000 ft north of R-44 and is completed at the top of the regional aquifer with the upper screen in the Puye Formation and the lower screen in the Miocene pumiceous sediments. Screen 1 is 10 ft long, set between 880 and 890 ft bgs. Screen 2 is 20 ft long, extending from 974.9 to 994.9 ft bgs. The composite water level in R-45 measured at the outset of testing R-44 and R-45 was 868.27 ft bgs. When the zones were isolated with inflatable packers, the water level in screen 1 rose from 0.04 to 868.23 ft bgs, while the level in screen 2 dropped 0.07 to 868.34 ft bgs. Thus, the initial water level in screen 1 was just 0.11 ft above that in screen 2. The brass cap elevation at R-44 is 6704.02 ft amsl, making the approximate static water-level elevations 5836 ft in screens 1 and 2.

R-44 Screen 1 Testing

R-44 screen 1 was tested from February 14 to February 18, 2009. Testing consisted of brief trial pumping on February 14, background data collection, and a 24-h constant-rate pumping test that was begun on February 16.

Two trial tests were conducted on February 14. Trial 1 was conducted at an average discharge rate of 19.2 gpm for 60 min from 8:00 to 9:00 a.m. (all times Mountain Standard Time) and was followed by 60 min of recovery until 10:00 a.m. Trial 2 was conducted for 120 min from 10:00 a.m. to 12:00 p.m. at 20.0 gpm. Following shutdown, recovery/background was monitored for 44 h until 8:00 a.m. on February 16.

During the trial tests, the generator supplying power to the submersible pump operated erratically with fluctuating voltage and alternating current frequency as well as substandard current frequency. This caused undesirable fluctuations in the discharge rate and limited the maximum rate that could be obtained. On February 15, a replacement generator was installed and run for about 10 mi from 11:34 to 11:44 a.m. to verify operation and rotation direction on the pump.

At 8:00 a.m. on February 16, the 24-h pumping test was begun at a rate of 24.2 gpm. Pumping continued until 8:00 a.m. on February 17. Following shutdown, recovery measurements were recorded for 24 h until 8:00 a.m. on February 18.

R-44 Screen 2 Testing

R-44 screen 2 was tested from February 19 to February 23, 2009. Testing consisted of brief trial pumping on February 19, background data collection, and a 24-h constant-rate pumping test that was begun on February 21.

Two trial tests were conducted on February 19. Trial 1 was conducted at a discharge rate of 23.9 gpm for 60 min from 8:00 to 9:00 a.m. and was followed by 60 min of recovery until 10:00 a.m. Trial 2 was conducted for 120 min from 10:00 a.m. to 12:00 p.m. at 24.0 gpm. Following shutdown, recovery/ background was monitored for 44 h until 8:00 a.m. on February 21.

At 8:00 a.m. on February 21, the 24-h pumping test was begun at a rate of 23.9 gpm. Pumping continued until 8:00 a.m. on February 22. Following shutdown, recovery measurements were recorded for 24 h until 8:00 a.m. on February 23.

Leaky Drop Pipe Joints

During the R-44 testing, there was leakage through the threaded joints on the 1 ½-in. stainless-steel drop pipe (1.90-in. outside diameter [O.D.] × 1.61-in. inside diameter [I.D.]), creating downhole voids inside the drop pipe beneath the check valves. This allowed initial pump operation against reduced head until the voids were refilled. The result was an elevated pumping rate for a brief period at the beginning of most of the tests. This effect corrupted the early startup data and added uncertainty to the analyses of the early drawdown data. The leaks were caused by either worn or improperly manufactured threads, as well as the need to avoid wrenching the pipe extremely as a precaution against galling the stainless-steel threads.

C-2.0 BACKGROUND DATA

The background water-level data collected in conjunction with running the pumping tests allow the analyst to see what water-level fluctuations occur naturally in the aquifer and help distinguish between water-level changes caused by conducting the pumping test and changes associated with other causes.

Background water-level fluctuations have several causes, among them barometric pressure changes, operation of other wells in the aquifer, Earth tides, and long-term trends related to weather patterns. The background data hydrographs from the monitored wells were compared with barometric pressure data from the area to determine if a correlation existed.

Previous pumping tests on the plateau have demonstrated a barometric efficiency for most wells of between 90% and 100%. Barometric efficiency is defined as the ratio of water-level change divided by barometric pressure change, expressed as a percentage. In the initial pumping tests conducted on the early R-wells, downhole pressure was monitored using a vented pressure transducer. This equipment measures the difference between the total pressure applied to the transducer and the barometric pressure, this difference being the true height of water above the transducer.

Subsequent pumping tests, including R-44, have utilized nonvented transducers. These devices simply record the total pressure on the transducer, that is, the sum of the water height plus the barometric pressure. This results in an attenuated “apparent” hydrograph in a barometrically efficient well. Take as an example a 90% barometrically efficient well. When monitored using a vented transducer, an increase in barometric pressure of 1 unit causes a decrease in recorded downhole pressure of 0.9 unit because the water level is forced downward 0.9 unit by the barometric pressure change. However, using a nonvented transducer, the total measured pressure increases by 0.1 unit (the combination of the barometric pressure increase and the water-level decrease). Thus, the resulting apparent hydrograph changes by a factor of 100 minus the barometric efficiency, and in the same direction as the barometric pressure change, rather than in the opposite direction.

Barometric pressure data were obtained from Technical Area 54 (TA-54) tower site from the Waste and Environmental Services Division-Environmental Data and Analysis (WES-EDA). The TA-54 measurement location is at an elevation of 6548 ft amsl, whereas the wellhead elevation is approximately 6715 ft amsl. The static water levels of the two zones were about 879 ft below land surface, making the water-table elevation roughly 5836 ft amsl. Therefore, the measured barometric pressure data from TA-54 had to be adjusted to reflect the pressure at the elevation of the water table within R-44.

The following formula was used to adjust the measured barometric pressure data:

$$P_{WT} = P_{TA54} \exp\left[-\frac{g}{3.281R}\left(\frac{E_{R44} - E_{TA54}}{T_{TA54}} + \frac{E_{WT} - E_{R44}}{T_{WELL}}\right)\right]$$

Equation C-1

Where, P_{WT} = barometric pressure at the water table inside R-44

P_{TA54} = barometric pressure measured at TA-54

g = acceleration of gravity, in m/sec^2 (9.80665 m/sec^2)

R = gas constant, in J/Kg/degree Kelvin (287.04 J/Kg/degree Kelvin)

E_{R44} = land surface elevation at R-44 site, in feet (6715 ft)

E_{TA54} = elevation of barometric pressure measuring point at TA-54, in feet (6548 ft)

E_{WT} = elevation of the water level in R-44, in feet (approximately 5836 ft)

T_{TA54} = air temperature near TA-54, in degrees Kelvin (assigned a value of 34.2 degrees Fahrenheit, or 284.4 degrees Kelvin)

T_{WELL} = air temperature inside R-44, in degrees Kelvin (assigned a value of 62.1 degrees Fahrenheit, or 289.9 degrees Kelvin)

This formula is an adaptation of an equation WES-EDA provided. It can be derived from the ideal gas law and standard physics principles. An inherent assumption in the derivation of the equation is that the air temperature between TA-54 and the well is temporally and spatially constant, and that the temperature of the air column in the well is similarly constant.

The corrected barometric pressure data reflecting pressure conditions at the water table were compared with the water-level hydrographs to discern the correlation between the two.

C-3.0 IMPORTANCE OF EARLY DATA

When pumping or recovery first begins, the vertical extent of the cone of depression is limited to approximately the well screen length, the filter pack length or, the aquifer thickness in relatively thin permeable strata. For many pumping tests on the plateau, the early pumping period is the only time that the effective height of the cone of depression is known with certainty. Thus, the early data often offer the best opportunity to obtain hydraulic conductivity information because conductivity would equal the earliest-time transmissivity divided by the well screen length.

Unfortunately, in many pumping tests, casing-storage effects dominate the early-time data, hindering the effort to determine the transmissivity of the screened interval. The duration of casing-storage effects can be estimated using the following equation (Schafer 1978, 098240).

$$t_c = \frac{0.6\left(D^2 - d^2\right)}{\frac{Q}{s}}$$

Equation C-2

Where, t_c = duration of casing storage effect, in minutes

D = inside diameter of well casing, in inches

d = outside diameter of column pipe, in inches

Q = discharge rate, in gallons per minute

s = drawdown observed in pumped well at time t_c, in feet

In some instances, it is possible to eliminate casing storage effects by setting an inflatable packer above the tested screen interval before conducting the test. Therefore, this option has been implemented for the R-well testing program, including the R-44 pumping tests.

C-4.0 TIME-DRAWDOWN METHODS

Time-drawdown data can be analyzed using a variety of methods. Among them is the Theis method (1934-1935, 098241). The Theis equation describes drawdown around a well as follows:

$$s = \frac{114.6Q}{T} W(u)$$

Equation C-3

Where,

$$W(u) = \int_u^\infty \frac{e^{-x}}{x} dx$$

Equation C-4

and

$$u = \frac{1.87 r^2 S}{Tt}$$

Equation C-5

and where, s = drawdown, in feet

Q = discharge rate, in gallons per minute

T = transmissivity, in gallons per day per foot

S = storage coefficient (dimensionless)

t = pumping time, in days

r = distance from center of pumpage, in feet

To use the Theis method of analysis, the time-drawdown data are plotted on log-log graph paper. Then, Theis curve matching is performed using the Theis type curve—a plot of the Theis well function W(u) versus 1/u. Curve matching is accomplished by overlaying the type curve on the data plot and, while keeping the coordinate axes of the two plots parallel, shifting the data plot to align with the type curve, effecting a match position. An arbitrary point, referred to as the match point, is selected from the overlapping parts of the plots. Match-point coordinates are recorded from the two graphs, yielding four values:W(u): 1/u, s, and t. Using these match-point values, transmissivity and storage coefficient are computed as follows:

$$T = \frac{114.6Q}{s} W(u)$$

Equation C-6

$$S = \frac{Tut}{2693r^2}$$

Equation C-7

Where, *T* = transmissivity, in gallons per day per foot

S = storage coefficient

Q = discharge rate, in gallons per minute

W(u) = match-point value

s = match-point value, in feet

u = match-point value

t = match-point value, in minutes

An alternative solution method applicable to time-drawdown data is the Cooper–Jacob method (1946, 098236), a simplification of the Theis equation that is mathematically equivalent to the Theis equation for most pumped well data. The Cooper–Jacob equation describes drawdown around a pumping well as follows:

$$s = \frac{264Q}{T} \log \frac{0.3Tt}{r^2 S}$$

Equation C-8

The Cooper–Jacob equation is a simplified approximation of the Theis equation and is valid whenever the u value is less than about 0.05. For small radius values (e.g., corresponding to borehole radii), u is less than 0.05 at very early pumping times and therefore is less than 0.05 for most or all measured drawdown values. Thus, for the pumped well, the Cooper–Jacob equation usually can be considered a valid approximation of the Theis equation.

According to the Cooper–Jacob method, the time-drawdown data are plotted on a semilog graph, with time plotted on the logarithmic scale. Then a straight line of best fit is constructed through the data points and transmissivity is calculated using:

$$T = \frac{264Q}{\Delta s}$$

Equation C-9

Where, T = transmissivity, in gallons per day per foot

Q = discharge rate, in gallons per minute

Δs = change in head over one log cycle of the graph, in feet

Because the R-wells are severely partially penetrating, an alternate solution considered for assessing aquifer conditions is the Hantush equation for partially penetrating wells (Hantush 1961, 098237; Hantush 1961, 106003). The Hantush equation is as follows:

Equation C-10

$$s = \frac{Q}{4\pi T}\left[W(u) + \frac{2b^2}{\pi^2 (l-d)(l'-d')}\sum_{n=1}^{\infty}\frac{1}{n^2}\left(\sin\frac{n\pi l}{b} - \sin\frac{n\pi d}{b}\right)\left(\sin\frac{n\pi l'}{b} - \sin\frac{n\pi d'}{b}\right)W\left(u, \sqrt{\frac{K_z}{K_r}}\frac{n\pi r}{b}\right)\right]$$

Where, in consistent units, s, Q, T, t, r, S, and u are as previously defined and

b = aquifer thickness

d = distance from top of aquifer to top of well screen in pumped well

l = distance from top of aquifer to bottom of well screen in pumped well

d' = distance from top of aquifer to top of well screen in observation well

l' = distance from top of aquifer to bottom of well screen in observation well

K_z = vertical hydraulic conductivity

K_r = horizontal hydraulic conductivity

In this equation, $W(u)$ is the Theis well function and $W(u,\beta)$ is the Hantush well function for leaky aquifers where:

$$\beta = \sqrt{\frac{K_z}{K_r}}\frac{n\pi r}{b}$$

Equation C-11

Note that for single-well tests, $d = d'$ and $l = l'$.

C-5.0 RECOVERY METHODS

Recovery data were analyzed using the Theis recovery method. This is a semilog analysis method similar to the Cooper–Jacob procedure.

In this method, residual drawdown is plotted on a semilog graph versus the ratio *t/t'*, where *t* is the time since pumping began and *t'* is the time since pumping stopped. A straight line of best fit is constructed through the data points and *T* is calculated from the slope of the line as follows:

$$T = \frac{264Q}{\Delta s} \qquad \textbf{Equation C-12}$$

The recovery data are particularly useful compared with time-drawdown data. Because the pump is not running, spurious data responses associated with dynamic discharge rate fluctuations are eliminated. The result is that the data set is generally “smoother” and easier to analyze. This was of paramount importance in the R-44 pumping tests because of the entrained air induced discharge rate fluctuations.

C-6.0 SPECIFIC CAPACITY METHOD

The specific capacity of the pumped well can be used to obtain a lower-bound value of hydraulic conductivity. The hydraulic conductivity is computed using formulas that are based on the assumption that the pumped well is 100% efficient. The resulting hydraulic conductivity is the value required to sustain the observed specific capacity. If the actual well is less than 100% efficient, it follows that the actual hydraulic conductivity would have to be greater than calculated to compensate for well inefficiency. Thus, because the efficiency is unknown, the computed hydraulic conductivity value represents a lower bound. The actual conductivity is known to be greater than or equal to the computed value.

For fully penetrating wells, the Cooper–Jacob equation can be iterated to solve for the lower-bound hydraulic conductivity. However, the Cooper–Jacob equation (assuming full penetration) ignores the contribution to well yield from permeable sediments above and below the screened interval. To account for this contribution, it is necessary to use a computation algorithm that includes the effects of partial penetration. One such approach was introduced by Brons and Marting (1961, 098235) and augmented by Bradbury and Rothchild (1985, 098234).

Brons and Marting introduced a dimensionless drawdown correction factor, s_P, approximated by Bradbury and Rothschild as follows:

$$s_P = \frac{1-\frac{L}{b}}{\frac{L}{b}}\left[\ln\frac{b}{r_w} - 2.948 + 7.363\frac{L}{b} - 11.447\left(\frac{L}{b}\right)^2 + 4.675\left(\frac{L}{b}\right)^3\right] \qquad \textbf{Equation C-13}$$

In this equation, L is the well screen length, in ft. Incorporating the dimensionless drawdown parameter, the conductivity is obtained by iterating the following formula:

$$K = \frac{264Q}{sb}\left(\log\frac{0.3Tt}{r_w^2 S} + \frac{2s_P}{\ln 10}\right) \qquad \textbf{Equation C-14}$$

To apply this procedure, a storage coefficient value must be assigned. Unconfined conditions were assumed for screen 1, while confined to leaky-confined conditions were applied to screen 2. Storage coefficient values for confined conditions can be expected to range from about 10^{-5} to 10^{-3}, depending on aquifer thickness, while those for unconfined conditions can be expected to range from about 0.01 to 0.25 (Driscoll 1986, 104226). The calculation result is not particularly sensitive to the choice of storage coefficient value, so a rough estimate of the storage coefficient is generally adequate to support the calculations. An assumed value of 0.1 was used in the calculations for screen 1, while values of 10^{-3} and $10–^{2}$ were used for screen 2. For screen 2, a storage coefficient value of 10^{-3} was deemed appropriate for the assumption of confined conditions (with perhaps very minor leakage from above), while 10^{-2} was used to simulate leaky-confined conditions.

The analysis also requires assigning a value for the saturated aquifer thickness, b. For calculation purposes, the screen 1 zone was assumed to extend from the water table, at 879 ft bgs, to the midpoint of the blank pipe section between the two screens, at approximately 945 ft bgs. This resulted in an assigned aquifer thickness of 67 ft for screen 1. This was equivalent to assuming that the resistive zone between screens 1 and 2 was at the midpoint of the intervening blank section, even though the actual location of the aquitard was not known. However, the computed result is not particularly sensitive to the exact aquifer thickness, because sediments far above or below the screen have little effect on yield and drawdown response. Therefore, the calculation based on the assumed aquifer thickness value was deemed to be adequate. For screen 2, an arbitrary thickness of 200 ft was assigned in the calculations.

Computing the lower-bound estimate of hydraulic conductivity can provide a useful frame of reference for evaluating the other pumping test calculations.

C-7.0 BACKGROUND DATA ANALYSIS

Background aquifer pressure data collected during the R-44 tests were plotted along with barometric pressure to determine the barometric effect on water levels and to look for pumping response in the surrounding observation wells. The four screen zones in R-44 and R-45 were monitored using nonvented pressure transducers, while the remaining wells—R-11, R-13, and R-28—were monitored using vented transducers.

Figure C-7.0-1 shows aquifer pressure data from R-44 screen 1 along with barometric pressure data from TA-54 that have been corrected to equivalent barometric pressure in feet of water at the water table. The R-44 data are referred to in the figure as the “apparent hydrograph” because the measurements reflect the sum of water pressure and barometric pressure, having been recorded using a nonvented pressure transducer. The times of the pumping periods for the screen 1 and screen 2 pumping tests are included on the figure for reference.

The transducers used in screens 1 and 2 were switched between tests, accounting for the different appearance in the data output from the screen 1 tests to the screen 2 tests. The transducer used initially (during the screen 1 test) showed substantial scatter, giving the thick-appearing plot of data points. The second transducer (right side of graph) showed less scatter, except during the pumping periods when significant scatter was observed. This resulted from the transducer having to be located adjacent to the pump power cable (inevitable when pumping screen 2 and monitoring screen 1), which interfered with transducer operation when the pump was running. The second transducer showed some sort of a dry problem (oil-canning) as indicated by the “striped” or “layered” effect seen in the data trace on February 19, 22 and 23. This effect had been seen previously during the testing of R-16r in 2005 and is believed to indicate a transducer malfunction of some sort.

To minimize the data scatter on Figure C-7.0-1, a rolling average of the data was plotted in Figure C-7.0-2. The average included data over a 1-h interval.

It appeared in Figures C-7.0-1 and C-7.0-2 that changes in barometric pressure had no discernible effect on water levels. An example illustrating this was the abrupt drop and subsequent rise in barometric pressure on February 20 that appeared to have no corresponding effect on the total aquifer pressure.

As a check on this, a plot was made of background data collected subsequently from R-44 screen 1 during the R-45 pumping tests conducted in late February and early March. Figure C-7.0-3 shows the observed apparent hydrograph and the corresponding barometric pressure. Figure C-7.0-3 confirmed that changes in barometric pressure had no effect on the aquifer pressure. The clincher was the tremendous change in barometric pressure that occurred from February 27 to 28 with no corresponding perturbation in the apparent hydrograph. This implied a high barometric efficiency for screen 1, essentially 100%.

Aside from the lack of response to barometric pressure changes, there were two other key observations made from the data shown in Figures C-7.0-1 and C-7.0-2. First, during the background data collection before the screen 1 pumping test, there was a distinct, steady decline in aquifer pressure totaling about 0.03 ft over 2.5 d. It was believed that this was a response to operation of Los Alamos County well PM-4, which began pumping on February 11 and ran continuously until March 4. The other supply wells cycled randomly throughout this period (illustrated below) and would not have caused the observed effect.

Second, there was a distinct response in screen 1 to pumping screen 2—both during the trial tests on February 19 and the 24-h test on February 21. During the 24-h screen 2 pumping period, the observed drawdown in screen 1 was about 0.05 ft. Following pump shutoff, there was a slow recovery effect, typical of the response of distant observation wells or wells separated from the pumped zone by an aquitard.

Figure C-7.0-4 shows the apparent hydrograph for R-44 screen 2 recorded during the screen 1 and 2 test periods. The times of the screen 1 and 2 pumping tests are included in the figure for reference. Again, the transducers were switched between tests, accounting for the difference in the appearance of the data plots from one test to the other. Note that the transducer used during the screen 2 test (right side of Figure C-7.0-4) was the same one that was used to monitor screen 1 during the screen 1 pumping test (left side of Figure C-7.0-1). The broad data scatter was consistent in both plots and apparently unique to that particular transducer.

To remove some of the scatter in the plot, a rolling average of the data was prepared as shown in Figure C-7.0-5.

Finally, an additional plot was prepared in Figure C-7.0-6 comparing the aquifer pressure response with the times of operation of Los Alamos County production wells PM-3, PM-5, and O-4. PM-4 was not included in the plot, as it operated continuously throughout the time period shown on the graph.

The data from Figures C-7.0-4, C-7.0-5, and C-7.0-6 were examined to discern the relationships between aquifer pressure and both barometric pressure fluctuations and municipal pumping. There were some hints of a possible correlation of aquifer pressure and changes in barometric pressure. For example, a decline in barometric pressure on February 16 seemed to coincide with a drop in aquifer pressure, while rises in barometric pressure late on February 18 and 20 matched increases in aquifer pressure. The decline in aquifer pressure on February 16 occurred during the 24-h pumping test, so it may have been a response to pumping. However, there was no such analogous explanation for the aquifer pressure increases.

To provide further insight into the relationship between screen 2 water levels and barometric pressure, a plot was made of background data collected subsequently from R-44 screen 2 during the R-45 pumping tests conducted in late February and early March. Figure C-7.0-7 shows the observed apparent hydrograph and the corresponding barometric pressure. Because of the scatter in the data set, a rolling average plot was prepared as shown in Figure C-7.0-8.

The data shown in Figures C-7.0-7 and C-7.0-8 showed that barometric pressure changes, in fact, caused no change in aquifer pressure. This was best illustrated by the observations made from February 27 to 28. The tremendous rise in barometric pressure during this period had no effect on aquifer pressure. This implied essentially a 100% barometric efficiency for screen 2, similar to what was observed for screen 1. This meant that the aquifer pressure increases seen on February 18 and 20, as well as the decline observed on February 16, were not attributable to barometric pressure fluctuations. (The diurnal fluctuations having a magnitude of about 0.03 ft in Figures C-7.0-7 and C-7.0-8 were responses to Earth tides.)

The data in Figures C-7.0-4 and C-7.0-5 were reexamined in light of knowing that barometric pressure fluctuations did not affect the apparent hydrograph. The background data leading up to the screen 1 24-h pumping test (February 13 to 16) showed a steady pressure decline of about 0.06 ft in 2.5 d. This was likely attributable to operation of PM-4, which was started on February 11 and run continuously. The response to pumping PM-4 in screen 2 was twice as great as that in screen 1 (0.06 ft versus 0.03 ft). This implied the possibility of a zone of limited permeability separating screens 1 and 2, effectively providing greater hydraulic isolation of screen 1 from the effects of PM-4 operation. This was also consistent with the minimal drawdown observed in each screen (0.05 ft) due to pumping the other screen.

During the 24-h pumping test on screen 1, the rate of water-level decline in screen 2 increased, indicating a response to the pumping test. Following pump shutoff, there was a slow recovery, typical of the response of distant observation wells or wells separated from the pumped zone by an aquitard. A rough estimate of the drawdown induced in screen 2 by pumping screen 1 was 0.05 ft.

An examination of the production well operation schedule in Figure C-7.0-6 showed no correlation between screen 2 aquifer pressure and cycling of production wells PM-3, PM-5, and O-4. There was no obvious explanation for the aquifer pressure increases observed on February 18 and 20 and no such response was observed in R-44 screen 1. It is possible that these fluctuations may have been attributable to Earth tides.

Figure C-7.0-9 shows the apparent hydrograph for R-45 screen 1. The transducer output showed the same bizarre striped/layered effect that was observed from one of the transducers used to monitor R-44. There appeared to be no aquifer pressure response to changes in barometric pressure, implying a barometric efficiency of essentially 100%.

The gradual decline in pressure from February 13 to February 19 likely was caused by the startup and continuous operation of production well PM-4 beginning on February 11. The aquifer pressure declined approximately 0.11 ft over a 6-d period.

There was no discernible response in R-45 screen 1 to test pumping R-44 screen 1, although the unusual transducer output may have masked subtle changes in water level. There appeared to be a response, however, to pumping R-44 screen 2. The water level in R-45 screen 1 dropped roughly 0.02 ft during the 24-h pumping test in R-44 screen 2.

The data in Figure C-7.0-9 were replotted in Figure C-7.0-10 along with operating times for production wells PM-3, PM-5 and O-4. Examination of the data showed that there was no discernible response in R-45 screen 1 to cycling these three production wells.

Figure C-7.0-11 shows the apparent hydrograph for R-45 screen 2. To eliminate some of the data scatter, a rolling average plot was prepared also as shown in Figure C-7.0-12. Several observations can be made from these graphs.

The aquifer pressure declined for a few days before the R-44 screen 1 pumping test and continued to show declines during the test as well. During the 3-d period leading up to the test, the water-level decline was approximately 0.04 ft. This change in water level probably was caused by the startup and continuous operation of PM-4.

Water-level perturbations (diurnal in places) having a magnitude of a few hundredths of a foot appeared throughout the monitoring period. It was believed that these were Earth tide effects.

Pumping R-44 screen 1 appeared to induce slight drawdown in R-45 screen 2. The magnitude of the effect was estimated to be about 0.02 ft. Figure 13 shows an expanded-scale plot of the apparent R-45 screen 2 hydrograph along with a straight line of fit for visual reference. The effect caused by pumping R-44 screen 1 was slight, but distinct.

Pumping R-44 screen 2 caused a greater effect in R-45 screen 2 than did pumping R-44 screen 1. The drawdown in R-45 screen 2 was approximately 0.06 ft during the 24-h constant-rate pumping test conducted in R-44 screen 2.

There was a prominent water-level rise in R-45 screen 2 from late February 20 to the start of the 24-h test on R-44 screen 2 on February 21. This was similar to the water-level rise seen in R-44 screen 2 during the same period. This response was absent from the R-45 screen 1 data. Thus, the distinct water-level increase from this period was observed in both R-44 screen 2 and R-45 screen 2 but was absent from screen 1 in both wells. It was suspected that Earth tides may have caused these perturbations in the water level because no other cause could be identified. The other such rise seen in R-44 screen 2 from February 18 to 19 (Figures C-7.0-4 and C-7.0-5) was not evident in R-45 screen 2.

Figure C-7.0-14 shows a plot of the R-45 screen 2 apparent hydrograph along with operating times for production wells PM-3, PM-5, and O-4. There was no discernible correlation between production well cycling and water-level fluctuations in R-45 screen 2. It appeared that continuous operation of PM-4 beginning on February 11 caused the only identifiable water-level changes in R-45 screen 2.

Figure C-7.0-15 shows the hydrograph obtained from well R-11 located in Sandia Canyon less than half a mile north of R-44. The data were recorded using the permanently installed vented transducer, so the hydrograph fluctuated with barometric pressure rather than showing the more flat-line response typical of nonvented transducers. The times of the pumping tests on R-44 screens 1 and 2 are included on the graph for reference.

Visual examination of the hydrograph and barometric pressure curve showed that they nearly coincided. There was, however, a clear downward water-level trend from the start of monitoring on February 13 to about February 19. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. Beginning February 19, the hydrograph and barometric pressure curve pretty much coincided. It was likely that the decline in water level from February 13 to February 19 was caused by startup and operation of PM-4, which began on February 11.

Because the barometric pressure fluctuations in the hydrograph were large, it was necessary to correct the water-level data by removing the barometric effect. This was done in two ways. One procedure involved correcting the data using BETCO (barometric and Earth tide correction) software, a mathematically complex correction algorithm that uses regression deconvolution (Toll and Rasmussen 2007, 104799) to modify the data. The BETCO correction not only removes barometric pressure effects, but Earth tides as well. The BETCO corrected data are shown in Figure 15.

A visual examination of the corrected hydrograph showed minor perturbations on the order of a few hundredths of a foot, but no identifiable response to pumping either screen 1 or screen 2 in R-44.

A second correction approach was applied to the hydrograph data by correcting directly for the change in barometric pressure assuming 100% barometric efficiency and immediate response. Figure C-7.0-16 shows the hydrograph corrected in this manner. The BETCO correction was retained on the graph for comparison.

The direct correction method seemed to produce better results for R-11. The corrected hydrograph reflected the small, steady drop in level from February 13 to 19 caused by operation of PM-4. The water-level decline attributable to pumping PM-4 was about 0.06 ft over a 6-d period. Then, beginning February 19, the corrected hydrograph was nearly flat. Visual examination of the hydrograph showed no correlation between water-level fluctuations and the pumping of either screen in R-44.

Figure C-7.0-17 shows the hydrograph obtained from R-13 located roughly 980 ft east of R-44. Again the times of the R-44 pumping tests and the BETCO hydrograph correction are included on the graph.

Visual examination of the hydrograph and barometric pressure curve showed that they nearly coincided. Similar to the R-11 response, there was a clear downward water-level trend from the start of monitoring on February 13 to about February 20. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. The initial gap between the curves was wider than in R-11, indicating a more rapid water-level decline in R-13 caused by the operation of PM-4.

Beginning February 20, the hydrograph and barometric pressure curve coincided, except for a departure that occurred during the R-44 screen 2 pumping test. This indicated a possible response to pumping screen 2. Indeed, the BETCO correction showed a clear pumping response to the screen 2 test of approximately 0.06 ft. The BETCO plot suggested the lack of a response, however, to the pumping test conducted on R-44 screen 1. Finally, the BETCO plot indicated roughly 0.15 ft of water-level decline due to PM-4 pumping over roughly a 7-d period.

A second correction was performed, this time using the direct approach of correcting for barometric pressure only, assuming 100% barometric efficiency and immediate response. Figure C-7.0-18 shows the resulting corrected hydrograph. The BETCO hydrograph was retained on the figure for comparison purposes.

Similar to the BETCO plot, the corrected hydrograph in Figure C-7.0-18 showed a clear response to pumping R-44 screen 2. Unlike the BETCO plot, however, the corrected hydrograph suggested a possible subtle response to pumping screen 1. This was evidenced by the increase in the slope of the hydrograph during the pumping period followed by a flattening (cessation of the downward background trend) during recovery. To clarify this, an expanded-scale plot of the corrected hydrograph was prepared. Figure C-7.0-19 shows the expanded-scale plot of the R-13 corrected hydrograph along with a straight line of fit for visual reference. The resulting data indicated a possible pumping effect from R-44 screen 1 of about 0.03 ft. Because of the small magnitude of the effect and the fact that the BETCO correction removed it altogether, it is possible that it was an Earth tide or delayed barometric effect rather than a response to pumping R-44 screen 1.

A final observation from the corrected hydrograph in Figures C-7.0-18 and C-7.0-19 was the abrupt rise in water level from late February 20 to early February 21. This was similar to that seen in R-44 screen 2 (Figures C-7.0-4 and C-7.0-5) and R-45 screen 2 (Figures C-7.0-11 through C-7.0-14). This effect was absent, however, on the BETCO hydrograph correction (Figures C-7.0-17 and C-7.0-18). Because the BETCO algorithm removes Earth tide effects, this may be evidence that this prominent feature in all three wells was indeed caused by Earth tides.

Figure C-7.0-20 shows the hydrograph obtained from R-28 located roughly 1620 ft northwest of R-44. Again, the times of the R-44 pumping tests and the BETCO hydrograph correction are included on the graph.

Visual examination of the hydrograph and barometric pressure curve showed that they were nearly identical. Similar to the R-11 and R-13 responses, there was a clear downward water-level trend from the start of monitoring on February 13 to about February 20. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. As in the other wells, this background trend was likely caused by startup and continuous operation of PM-4.

Beginning February 20, the hydrograph and barometric pressure curve coincided, except for a departure that occurred during the R-44 screen 2 pumping test. This indicated a possible response to pumping screen 2. Indeed, the BETCO correction showed a clear pumping response to the screen 2 test of approximately 0.03 ft. The BETCO plot suggested the lack of a response, however, to the pumping test conducted on R-44 screen 1. Finally, the BETCO plot indicated roughly 0.14 ft of water-level decline due to PM-4 pumping over roughly a 7-d period.

A second correction was performed, this time using the direct approach of correcting for barometric pressure only, assuming 100% barometric efficiency and immediate response. Figure C-7.0-21 shows the resulting corrected hydrograph. The BETCO hydrograph was retained in the figure for comparison purposes.

Similar to the BETCO plot, the corrected hydrograph in Figure C-7.0-21 showed a clear response to pumping R-44 screen 2. Unlike the BETCO plot, however, there was a hint of a possible response to pumping R-44 screen 1. This was evidenced by a slight increase in the slope of the hydrograph during the pumping period followed by a reduction in slope during recovery. To clarify these subtle effects, an expanded-scale plot of the corrected hydrograph was prepared. Figure C-7.0-22 shows the expanded-scale plot of the R-28 hydrograph along with a straight line of fit for visual reference. The data indicated a possible tiny response to pumping R-44 screen 1 of no more than about 0.01 ft. (Perhaps a logarithmic-shaped reference curve of some sort would have been more appropriate than the straight line shown on Figure C-7.0-22, but the conclusion of a deflection in the hydrograph would have been the same.) Because the effect was so small and the BETCO correction removed it altogether, it is possible that it was caused by something other than the R-44 screen 1 pumping test such as Earth tides or delayed barometric response. However, the striking coincidence of occurring during the screen 1 pumping test coupled with similar, though larger, responses observed in the other monitored wells, made it possible that the screen 1 pumping effects reached R-28.

A final observation from the corrected hydrograph in Figure C-7.0-22 was the data segment from late February 20 to early February 21. Contrary to the antecedent decline in water levels, there was a small rise in level during this period—a subdued version of the pronounced water-level rise seen in R-13, R-44 screen 2, and R-45 screen 2 probably caused by Earth tides.

C-8.0 R-44 SCREEN 1 DATA ANALYSIS

This section presents the data obtained from the R-44 screen 1 pumping tests and the results of the analytical interpretations. Data are presented for drawdown and recovery for trials 1 and 2 and the 24-h constant-rate pumping test.

R-44 Screen 1 Trial 1

Figure C-8.0-1 shows a semilog plot of the trial 1 drawdown data. The early data showed exaggerated drawdown because the pumping rate was elevated as the drop pipe was being filled for the first time and the pump operated against reduced head.

The middle data on the plot showed drawdown changes associated with erratic pump operation. The generator used for the trials tests on R-44 was defective, showing below normal alternating current output frequency as well as variable current frequency and voltage.

The late data showed drawdown changes associated with discharge rate adjustments made using the flow control valve at the surface. The average discharge rate during trial 1 was 19.2 gpm, while the rate over the last half of the test was 19.5 gpm. The many discharge rate changes during trial 1 precluded analysis of the drawdown data.

Figure C-8.0-2 shows a semilog plot of the trial 1 recovery data. The transmissivity value computed from the early data was 4210 gpd/ft. Based on the screen length of 10 ft, the computed hydraulic conductivity was 421 gpd/ft^2, or 56.3 ft/d.

The slope of the data trace began declining at just a few seconds, and within minutes the slope became essentially flat. Figure C-8.0-3 shows an expanded-scale plot of the late-recovery data.

The flattening of the recovery curve showed the effects of a complex combination of vertical growth of the cone of impression (partial penetration), leakage from below, and delayed drainage of the unconfined aquifer. For illustration purposes, a transmissivity was computed for the late data yielding a value of 270,000 gpd/ft. This was likely not an actual transmissivity but rather an artifact of the delayed yield and leakage. It does suggest, however, the possibility of a large aquifer transmissivity at the R-44 location.

R-44 Screen 1 Trial 2

Figure C-8.0-4 shows a semilog plot of the trial 2 drawdown data. The discharge rate for trial 2 was 20.0 gpm. The transmissivity value computed from the early data was 3550 gpd/ft, making the computed hydraulic conductivity 355 gpd/ft^2, or 47.5 ft/d. Note that the early data showed the effects of a minimal amount of antecedent drainage of the drop pipe through leaky coupling joints.

Later data in Figure C-8.0-4 showed erratic pumping water levels associated with discharge rate fluctuations induced by the inconsistent operation of the electric generator.

Figure C-8.0-5 shows an expanded-scale plot of the trial 2 drawdown data. At this scale, the plot showed more clearly the erratic drawdown induced by the variable generator output. The discharge rate variations precluded analysis of the late data. Nevertheless, the late data showed the effects of the combination of delayed yield and vertical expansion of the cone of depression.

Figure C-8.0-6 shows a semilog plot of the trial 2 recovery data. The transmissivity value computed from the early data was 3350 gpd/ft, making the computed hydraulic conductivity 355 gpd/ft^2, or 47.5 ft/d. The late-recovery data showed flattening associated with a combination of delayed yield, partial penetration and leakage.

Figure C-8.0-7 shows an expanded-scale plot of the late trial 2 recovery data.

The curve appeared to flatten completely (delayed yield) and did not support calculation of a representative transmissivity value. The severe data scatter coupled with the tiny changes in head precluded a rigorous analysis of the late data. The 44-h duration of the recovery period should have been long enough to exhaust the delayed drainage effect, implying the possibility of a high transmissivity for the regional aquifer at this location.

C-8.1 R-44 Screen 1 24-H Constant-Rate Pumping Test

Figure C-8.1-1 shows a semilog plot of the drawdown data recorded during the 24-h constant-rate pumping test conducted at a discharge rate of 24.2 gpm. The early data showed that some antecedent drainage of the drop pipe had occurred during the background monitoring period. The magnitude of the drawdown spike during the first minute of pumping allowed estimating the early pumping rate before refilling the void in the drop pipe, roughly 28 gpm. Using this discharge rate estimate, the transmissivity computed from the initial drawdown data was 3860 gpd/ft, making the computed hydraulic conductivity 3817 gpd/^{2}, or 42.4 ft/d.

The late data showed a reduction in drawdown over time. There was no obvious explanation for the odd occurrence because the measured discharge rates were constant, especially over the last half day of pumping. It is possible that minor sediment removal with the pumped water could have increased the well efficiency somewhat during the test.

Figure C-8.1-2 shows the recovery data measured following the 24-h constant-rate pumping test. The transmissivity calculated from the early data was 3510 gpd/ft, making the hydraulic conductivity 351 gpd/ft^2, or 46.9 ft/d.

As with all of the data plots, the late-recovery data showed severe flattening associated with delayed yield and vertical expansion of the cone of impression. Figure C-8.1-3 shows an expanded-scale plot of the late-recovery data.

Again, the latest slope in Figure C-8.1-3 did not support calculation of a meaningful transmissivity value because of possible lingering delayed yield effects as well as the broad data scatter. Nevertheless, the data suggested the possibility of a transmissive regional aquifer at the R-44 location.

Packer Deflation

Following the 24-h recovery period, the packer was deflated in preparation for pulling the pump. When this was done, water above the packer that had leaked through coupling joints in the drop pipe bypassed the packer and was delivered to the pressure transducer while the water drained back into the well and formation. This caused a pressure increase that was recorded by the transducer.

Figure C-8.1-4 shows the resulting head buildup and decay that occurred when the packer was deflated. Data were recorded at 1-min intervals so the maximum head buildup was not revealed in the data set. The high specific capacity of R-44 meant that a substantial volume of water could have flowed into the screen zones before the first head measurement in Figure C-8.1-4. The head data confirmed that pipe joints had leaked throughout the R-44 screen 1 pumping tests.

R-44 Screen 1 Specific Capacity Data

Specific capacity data were used along with well geometry to estimate a lower-bound conductivity value for the R-44 screen 1 zone for comparison to the pumping test values. In addition to specific capacity, other input values used in the calculations included the assumed aquifer thickness of 67 ft (from the static water level to the midpoint of the blank pipe section between screens 1 and 2), a storage coefficient of 0.1 and a borehole radius of 0.51 ft. The calculations are somewhat insensitive to the assigned aquifer thickness, as long as the selected value is substantially greater than the screen length.

R-44 screen 1 produced 24.2 gpm with a drawdown of 4.27 ft after 24 h of pumping for a specific capacity of 5.67 gpm/ft. Applying the Brons and Marting method to these inputs yielded a lower-bound hydraulic conductivity value for the screened interval of 432 gpd/ft^2, or 57.7 ft/d. Because the calculation method did not factor in the effects of leakage, it was possible that the computed value could be overestimated. Indeed, the value was somewhat greater than the values obtained from the early pumping test data but similar enough that it was not considered an unreasonable result. Overall, it provided corroboration of the pumping test values and suggested a hydraulically efficient completion.

R-44 Screen 1 Summary

Table 8.1-1 summarizes the hydraulic conductivity values obtained from the R-44 screen 1 pumping test analyses. The average hydraulic conductivity computed from the recovery data was 50.2 ft/d. The recovery average was used because of the effects of antecedent drop pipe drainage on the drawdown data.

The specific capacity obtained from screen 1 suggested a lower-bound hydraulic conductivity of 57.7 ft/d. However, that value did not factor in the effects of leakage that occurred in screen 1 and, thus, was considered consistent with the pumping test values. The results suggested a highly efficient screened interval.

Within seconds of startup or shutdown, vertical expansion of the cone of depression (leakage and partial penetration) and delayed yield affected the pumping and recovery data. The late data suggested an enormous transmissivity for the regional aquifer at the R-44 location, perhaps in excess of 100,000 gpd/ft.

C-9.0 R-44 SCREEN 2 DATA ANALYSIS

This section presents the data obtained from the R-44 screen 2 pumping tests and the results of the analytical interpretations. Data are presented for drawdown and recovery for trials 1 and 2 and the 24-h constant-rate pumping test.

Analysis of the screen 2 data was challenging because of the lack of early data from most of the tests. A programming oversight made in setting up the transducer data collection scheme, coupled with slightly varying clock speeds of the wristwatch used during the tests and the transducers, led to losing the very early data in all but one of the tests.

R-44 Screen 2 Trial 1

Figure C-9.0-1 shows a semilog plot of the drawdown data collected from trial 1 conducted at a discharge rate of 23.9 gpm. The early data showed exaggerated drawdown because the pumping rate was elevated as the drop pipe was being filled for the first time and the pump operated against reduced head.

The varying pumping rate associated with filling the drop pipe precluded analysis of the early drawdown data.

The data following filling of the drop pipe were plotted on an expanded scale as shown in Figure C-9.0-2. The slope of the graph became continuously flatter throughout the trial test. This was caused by a combination of vertical expansion of the cone of depression and leakage from the screen 1 zone. It is also possible that the data included indirect effects of delayed yield of the overlying unconfined screen 1 interval

The latest data on the graph supported a transmissivity calculation of 48,500 gpd/ft. This likely represented the thickness of sediment corresponding to the depth of the cone of depression at that particular time. The total aquifer transmissivity is likely greater than indicated by this calculation because the cone of depression was probably still expanding vertically and the slope of the graph likely would have continued flattening at later time. Note also that the substantial data scatter added uncertainty to the calculation. In fact, the amount of data scatter exceeded the drawdown change on which the analysis was based.

Figure C-9.0-3 shows the recovery data collected following shutdown of the trail 1 pumping test. The transmissivity computed from the early data on the graph was 3430 gpd/ft. Dividing this value by the screen length of 9.9 ft yielded a hydraulic conductivity estimate of 346 gpd/ft^2, or 46.3 ft/d. However, the earliest recovery data were not collected. Note that the first data point corresponded to a residual drawdown of less than a foot out of more than 18 ft of drawdown at the end of the pumping period. It was suspected that the recovery cone of impression had expanded vertically well beyond the length of the screened interval to some greater effective thickness and that the hydraulic conductivity was likely substantially less than computed from Figure C-9.0-3. Subsequent data, presented below, corroborated this idea.

Figure C-9.0-4 shows an expanded-scale plot of the trial 1 recovery data. The slope of the recovery curve continued to flatten throughout the monitored period. The latest data supported a transmissivity calculation of 124,000 gpd/ft.

There was uncertainty in this transmissivity calculation because of the tiny water-level changes and the substantial data scatter. Furthermore, there was no way to know the height of the cone of impression corresponding to the analysis or whether delayed yield from the upper aquifer zone was affecting the data. The analysis did suggest, however, a large aquifer transmissivity.

R-44 Screen 2 Trial 2

Figure C-9.0-5 shows a semilog plot of the drawdown data collected from trial 2 conducted at a discharge rate of 24.0 gpm. The data from the first few seconds of pumping showed exaggerated drawdown associated with minor antecedent drainage of a portion of the drop pipe through a leaky coupling joint. This precluded capturing the very early data for analysis.

The early data following refilling of the void in the drop pipe were plotted on an expanded scale as shown in Figure C-9.0-6. The transmissivity computed from the graph was 2900 gpd/ft. Diving this value by the screen length of 9.9 ft yielded a hydraulic conductivity estimate of 293 gpd/ft^2, or 39.2 ft/d.

It was likely that the cone of depression had already expanded well beyond the thickness of screened sediment so the hydraulic conductivity values computed based on the screen length of 9.9 ft were considered overestimates of the actual value. Note that the earliest data used in the analysis already showed nearly 16 ft of drawdown. In other words, the snapshot of the early data associated with initial lateral expansion of the cone of depression was masked by the discharge rate fluctuations caused by changing head conditions as the void in the drop pipe refilled. By the time postrefill data were collected, the cone of depression had expanded vertically.

Figure C-9.0-7 shows an expanded-scale plot of the late drawdown data from trial 2. The transmissivity value obtained from the latest slope on the graph was 146,000 gpd/ft. There was uncertainty in the calculated value because of wide data scatter and ongoing vertical expansion of the cone of depression as well as leakage and possible delayed yield effects from the overlying zone.

Figure C-9.0-8 shows the recovery data recorded following the trial 2 test on R-44 screen 2. The data set included earlier data than any other data set obtained in the testing effort, as evidenced by the fact that the residual drawdown was still at about 7 ft when data collection began. This data set allowed obtaining a reasonably representative snapshot of the early-recovery response.

The transmissivity computed from the early-recovery data was 820 gpd/ft, much smaller than previous values. Based on the screen length of 9.9 ft, the computed hydraulic conductivity was 82.8 gpd/^{2}, or 11.1 ft/d. This is likely a good representation of the hydraulic conductivity of the screened sediments. It implied that the larger values obtained previously were biased by vertical expansion of the drawdown or recovery cone.

Figure C-9.0-9 shows an expanded-scale plot of the late trial 2 recovery data. As with previous graphs of late drawdown or recovery data, the slope of the data trace flattened continuously throughout the recovery period, becoming essentially horizontal at the end of the monitoring period.

The continuous flattening resulted from partial penetration effects including both vertical expansion of the cone of impression and leakage from the upper zone, including possible delayed yield effects. No transmissivity value was calculated from the graph in Figure C-9.0-9, as it would have supported computation of any arbitrarily large value, depending on which portion of the curve was used for constructing the line of fit. As stated earlier, the late-time response was consistent with a large transmissivity for the regional aquifer at the R-44 location.

C-9.1 R-44 Screen 2 24-H Constant-Rate Pumping Test

Figure C-9.1-1 shows a semilog plot of the drawdown data recorded during the 24-h constant-rate pumping test conducted at a discharge rate of 23.9 gpm. The early data showed that antecedent drainage of the drop pipe had occurred during the background monitoring period.

The effect of variable discharge rates while filling the void in the drop pipe precluded analysis of the early data from the drawdown curve.

Figure C-9.1-2 shows an expanded-scale plot of the drawdown data following refilling of the drop pipe. The data set was quite noisy and did not support a useful analysis.

The drawdown spike at a time of 3 min was accompanied by a noise at the well head that sounded like air moving through the discharge piping. It was not known if these two events were cause-and-effect or merely coincidental. Generally, air is not entrained in the drop pipe when it drains, because the drained void is typically under vacuum conditions. However, if there were two leaky coupling joints between adjacent check valves, it is possible that water could have drained from the lower one while air was pulled into the upper one.

The abrupt drop in pumping water level that occurred between 600 and 1000 min was caused by temporarily pumping the discharge water to a lower elevation. Against the reduced head, the pumping rate increased by about 1%.

All other perturbations in the drawdown data were caused by uncontrollable discharge rate fluctuations associated with submersible pump and electric generator operation. Such variations in flow rate are not unusual.

Figure C-9.1-3 shows the recovery data measured following the 24-h constant-rate pumping test. The transmissivity calculated from the early data was 1760 gpd/ft, making the computed hydraulic conductivity 178 gpd/ft^2, or 23.8 ft/d.

Because the residual drawdown corresponding to the first data point on Figure C-9.1-3 was only 1.4 ft, most of the recovery had already occurred and it was likely that the earliest data, required to identify the properties of the screened sediments, were not collected. Indeed, the computed hydraulic conductivity was substantially greater than the value obtained from the trial 2 recovery data. It was likely that the cone of impression had expanded vertically beyond the screened interval before the first data point was recorded.

As with all of the data plots, the late-recovery data showed severe flattening associated with vertical expansion of the cone of impression, leakage and perhaps delayed yield. Figure C-9.1-4 shows an expanded-scale plot of the late-recovery data.

The slope of the data plot decreased continuously, eventually becoming and remaining essentially flat. It was hypothesized that at late time, the water level would eventually reach the initial static level. A line of fit was constructed to pass through the late data and the upper le corner of the graph (corresponding to zero residual drawdown at infinite time). The transmissivity computed from the artificially constructed line of fit was 39,100 gpd/ft.

There was uncertainty in the computed transmissivity value. The scatter in the data set exceeded the magnitude of water-level change on which the analysis was based. Further, any change in the background water level would have meant that targeting zero residual drawdown in the analysis was incorrect.

Regardless of the approach used to analyze the late-recovery data, the flat slope of the late data implied a high transmissivity for the regional aquifer at the R-44 location.

R-44 Screen 2 Specific Capacity Data

Specific capacity data were used along with well geometry to estimate a lower-bound conductivity value for the R-44 screen 2 zone for comparison to the pumping test values. In addition to specific capacity, other input values used in the calculations included the assumed an arbitrarily assigned aquifer thickness of 200 ft, storage coefficient values of 0.01 and 0.001 (for leaky-confined and confined conditions, respectively), and a borehole radius of 0.51 ft.

R-44 screen 2 produced 23.9 gpm with a drawdown of 17.72 ft after 24 h of pumping for a specific capacity of 1.35 gpm/. Applying the Brons and Marting method to these inputs yielded lower-bound hydraulic conductivity values for the screened interval of 111 gpd/^{2}, or 14.9 ft/d, for leaky-confined conditions and 113 ft^2, or 15.1 ft/d, for confined conditions.

These values were greater than the hydraulic conductivity estimate of 11.1 ft/d obtained from the trial 2 recovery data. However, the calculations were based on the assumption of homogeneous conductivity. It was known that the overlying sediments (screen 1) have a hydraulic conductivity around 50 ft/d. In addition, the rapid and severe flattening of all of the screen 2 drawdown and recovery curves was consistent with substantial transmissivity adjacent to the screened horizon. A reasonable explanation of the calculation results was that the screen 2 interval has a lower permeability than the adjacent overlying and/or underlying sediments. The greater permeability of the surrounding sediments (above and/or below the screen 2 interval) enhanced the specific capacity of screen 2. In this light, the computed lower-bound hydraulic conductivity based on the specific capacity performance of screen 2 appeared reasonable and consistent with the trial 2 recovery analysis.

R-44 Screen 2 Summary

Failure to collect very early data from most of the tests on screen 2 made determining formation properties a challenge. The best estimate of the hydraulic conductivity of the screened sediments was 11.1 ft/d from the trial 2 recovery data. The surrounding sediments (above and/or below the screened interval), however, appeared to have a substantially greater conductivity.

The specific capacity obtained from screen 2 suggested a lower-bound hydraulic conductivity of about 15 ft/d. However, that value did not factor in the effects of higher permeability adjacent sediments. In that light, the results were consistent with the estimated hydraulic conductivity of the screened interval. The results also implied an efficient completion.

Within seconds of startup or shutdown, vertical expansion of the cone of depression (leakage and partial penetration) and possibly delayed yield from the upper zone sediments affected the pumping and recovery data. The late data suggested an enormous transmissivity for the regional aquifer at the R-44 location, perhaps in excess of 100,000 gpd/ft.

C-10.0 LEAKANCE/RESISTANCE OF SEDIMENTS BETWEEN SCREENS 1 AND 2

Data from the pumping tests were used to estimate the leakance of the sediments separating R-44 screen 1 from screen 2. Each of the 24-h tests supported estimation of this parameter.

Pumping R-44 screen 1 at 24.2 gpm produced approximately 0.05 ft of drawdown in screen 2, while pumping screen 2 at 23.9 gpm resulted in about the same 0.05 ft of drawdown in screen 1. These responses to pumping were simulated analytically using Equations C-10 and C-11, assuming a uniform vertical anisotropy ratio. For each pumping test, the vertical anisotropy was adjusted until the observed drawdown in the nonpumped zone matched the field observation. The actual sediments are layered, not homogeneous, so the calculations just supported determination of an overall effective vertical resistance to flow.

The following assumptions were used in the calculations:

- aquifer thickness = 300 ft
- hydraulic conductivity = 50 ft/d
- storage coefficient ranged from 0.002 to 0.05.
- pumping rate = 24.2 gpm/23.9 gpm.
- static water level = 879 ft
- screen 1: 895 to 905 ft
- screen 2: 985.3 to 995.2 ft
- pumping time = 1440 min

The assumed hydraulic conductivity value of 50 ft/d matched the value obtained from the screen 1 pumping test. The screen 2 test produced a lower value, but the overall response of screen 2 was consistent with a highly transmissive aquifer. It was possible that screen 2 was set in lower permeability sediments than the aquifer average. Although the accuracy of the estimate was uncertain and the actual aquifer thickness assignment was arbitrary, the calculations were useful to provide a sense of the vertical permeability of the aquifer.

Using the above inputs, Equation C-10 was solved for anisotropy ratio by adjusting the ratio until the drawdown at 1440 min was equal to 0.05 ft. The computations were repeated for a few values of storage coefficient ranging from 0.002 to 0.05. Figure C-10.0-1 shows the computed relationship between storage coefficient and vertical anisotropy ratio. Virtually identical results were obtained for the two pumping tests.

The figure showed that there was insufficient data to determine the vertical anisotropy ratio accurately. Its value varied substantially as a function of storage coefficient and therefore its estimate was only as accurate as the estimate of storage coefficient.

For example, according to Figure C-10.0-1 for an assumed storage coefficient value of 0.01, the computed vertical anisotropy ratio was 0.015. Based on the assumed hydraulic conductivity of 50 ft/d, this made the estimated vertical permeability 0.015 × 50 = 0.75 ft/d. The corresponding leakance of the 80 ft of sediments separating the two screens was 0.75/80 = 0.00938 inverse days and the computed resistance was 1/0.00938 = 107 d. These calculations showed moderate vertical permeability, indicating the absence of a real aquitard. The results suggested moderate vertical movement of groundwater in the vicinity of R-44 screens 1 and 2.

These results implied a fairly conductive separating layer between screen 1 and screen 2, similar to formation characteristics at R-43, but different than what has been observed at other locations on the Plateau where the head separation between the uppermost screens in multi-screened wells is greater than observed here. As a comparison, similar analysis at R-35a and R-35b yielded hydraulic resistance on an order of magnitude greater than computed for R-44, while analysis of R-10 screens 1 and 2 data showed resistance more than two orders of magnitude greater. Note that part of the greater resistance at the other locations is attributable to the greater distance between the well screens. R-44 screens 1 and 2 are 80 ft apart, whereas the separation distance at R-35a/b is about 167 ft (accounting for elevation difference between the two wells) and that at R-10 is about 144 ft. From screen center to screen center, the downward gradients in R-35 a/b and R-10 are 0.031 ft and 0.083 /, respectively, compared with 0.0022 in R-44. Although computations like this have not been made for R-33, it is likely that the hydraulic resistance between screens 1 and 2 at that location is similar to what was determined for R-10 based on the large head difference between the screens in R-33. Thus, compared with other locations on the Plateau, the potential for vertical groundwater movement at R-44 (as well as R-43) is relatively favorable.

C-11.0 SUMMARY

Constant-rate pumping tests were conducted on R-44 screens 1 and 2 in Mortandad Canyon. The tests were conducted to gain an understanding of the hydraulic characteristics of the aquifers in which the screens were installed as well as the intervening sediments between the screens. Additionally, several surrounding wells were monitored to check for hydraulic cross connection to R-44.

Numerous observations and conclusions were drawn for the tests as summarized below.

The static water level in R-44 screen 1 was only 0.2 ft higher than in screen 2, suggesting minimal vertical hydraulic resistance of the intervening sediments. Consistent with this idea, analysis of interference effects between screen 1 and screen 2 (about 0.05 ft after 24 h of pumping 24 gpm) suggested moderate leakance.

All monitored wells and screen zones (R-44 screens 1 and 2, R-45 screens 1 and 2, R-11, R-13, and R-28) showed immediate water-level response to barometric pressure with a barometric efficiency of essentially 100%.

There was no correlation between water levels in any of the monitored wells and cycling of production wells PM-3, PM-5, and O-4. PM-4, on the other hand, which was started up a few days before the test program and ran continuously throughout, induced a small but steady drawdown trend in each of the monitored wells.

In addition to screens 1 and 2 affecting one another when pumping was performed, most (but not all) of the monitored screen zones showed slight pumping response. Table C-11.0-1 summarizes the pumping effects induced by testing R-44 screens 1 and 2, as well as that caused by continuous operation of PM-4.

Leaky threaded joints in the drop pipe used to hang the submersible test pump allowed drainage of a portion of the pipe between pumping events. Pumping against reduced head briefly until the void in the drop pipe was refilled resulted in chaotic discharge rate changes at the onset of pumping, corrupting much of the early drawdown data and rendering it unusable for determining aquifer properties. The early-recovery data, however, were usable. The leaky joints were likely attributable to a combination of worn threads, improperly manufactured threads, and the need to avoid over-tightening the threads to avoid galling.

The pumping test data indicated a hydraulic conductivity for the screen 1 sediments of about 50 ft/d.

Specific capacity analysis showed that screen 1 produced 24.2 gpm with 4.27 ft of drawdown, for a specific capacity of 5.67 gpm/ft. The lower-bound hydraulic conductivity computed from this information was 57.7 ft/d. Considering that this calculation did not consider the effects of leakage and therefore was probably overestimated, it provided reasonable corroboration of the pumping test hydraulic conductivity value.

The pumping test data indicated a hydraulic conductivity for the screen 2 sediments of about 11 ft/d.

Specific capacity analysis showed that screen 2 produced 23.9 gpm with 17.72 ft of drawdown, for a specific capacity of 1.35 gpm/ft. The lower-bound hydraulic conductivity computed from this information was about 15 ft/d. Considering that this calculation was based on homogeneous conditions and did not consider the effects of the greater permeability of adjacent sediments and therefore was probably overestimated, it provided reasonable corroboration of the pumping test hydraulic conductivity value.

All of the pumping tests showed immediate flattening of the drawdown and/or recovery curves. This reflected the effects of a combination of delayed yield and partial penetration (vertical expansion of the cone of depression). The fact that the drawdown and recovery curves remained flat at late time suggested a very large aquifer transmissivity, perhaps as great as 100,000 gpd/ft. At late time, the change in water level was within the “noise” level and accurate quantification of aquifer transmissivity was not possible.

C-12.0 REFERENCES

The following list includes all documents cited in this appendix. Parenthetical information following each reference provides the author(s), publication date, and ER ID. This information is also included in text citations. ER IDs are assigned by the Environmental Programs Directorate’s Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

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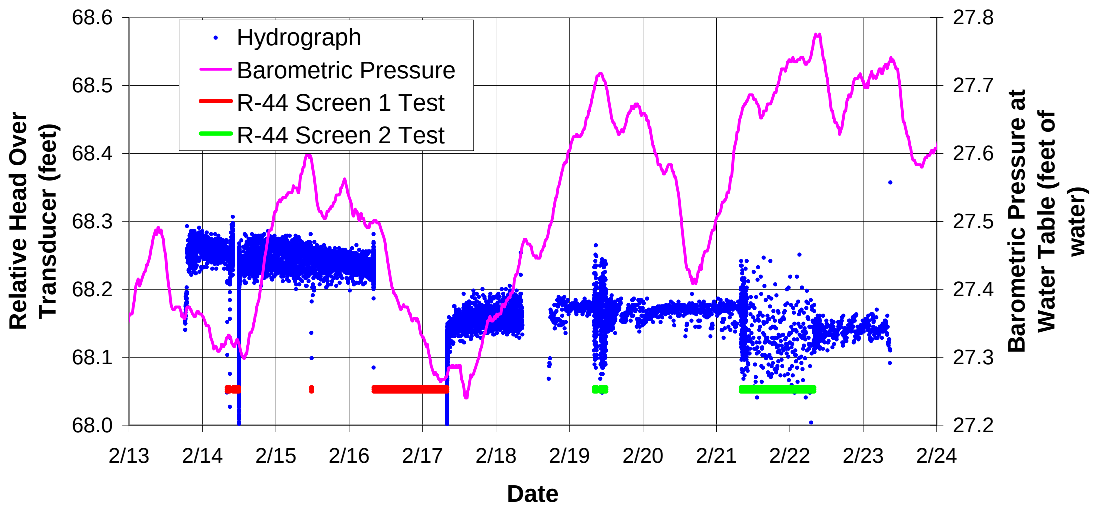

Figure C-7.0-1 R-44 screen 1 apparent hydrograph during R-44 screen 1 and 2 tests

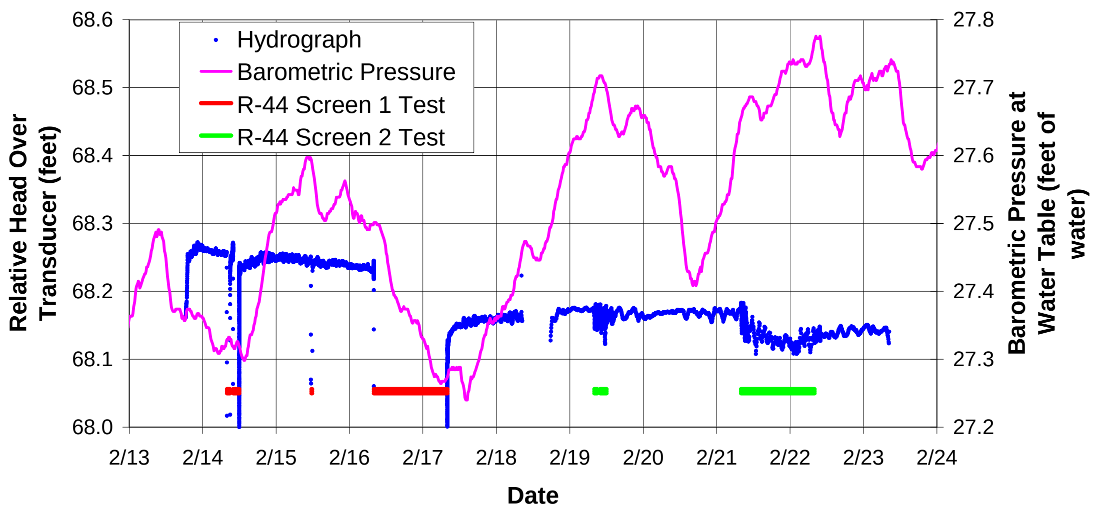

Figure C-7.0-2 R-44 screen 1 rolling apparent hydrograph during R-44 screen 1 and 2 tests

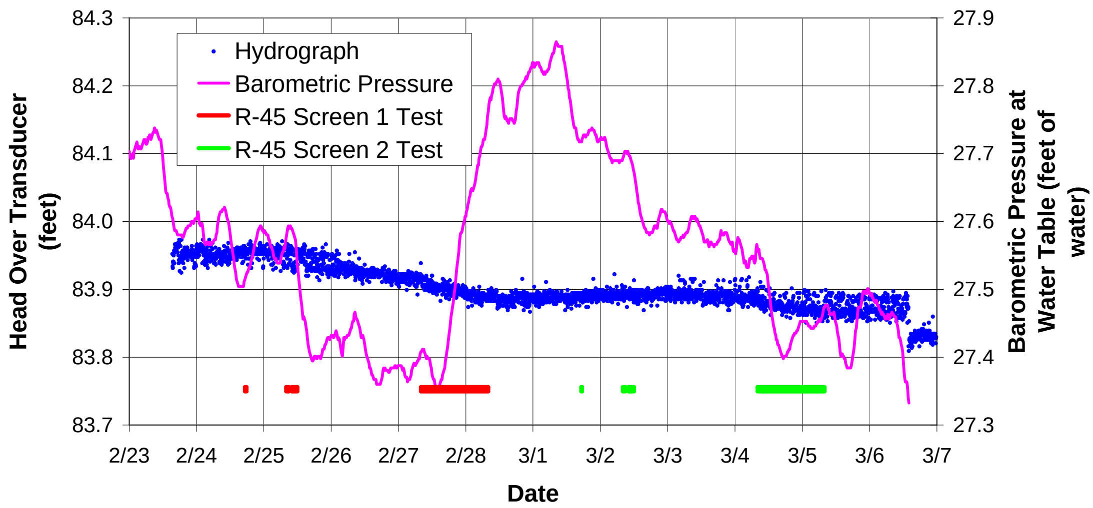

Figure C-7.0-3 R-44 screen 1 apparent hydrograph during R-45 screen 1 and 2 tests

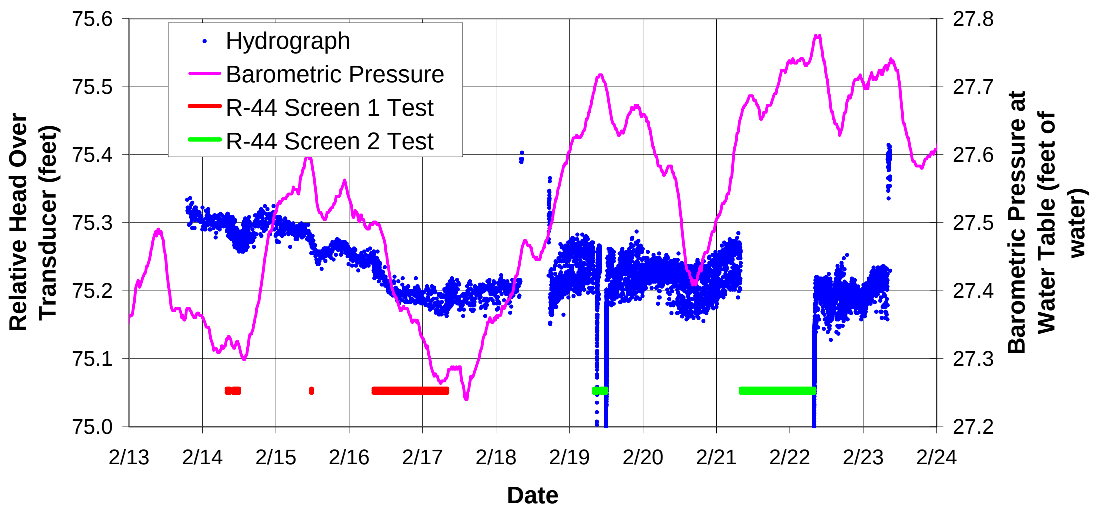

Figure C-7.0-4 R-44 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests

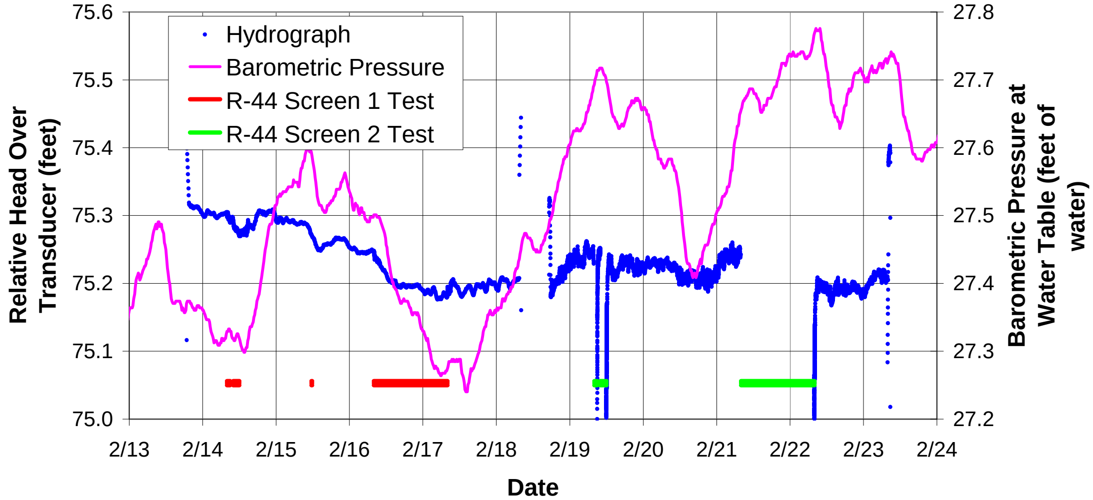

Figure C-7.0-5 R-44 screen 2 rolling average apparent hydrograph during R-44 screen 1 and 2 tests

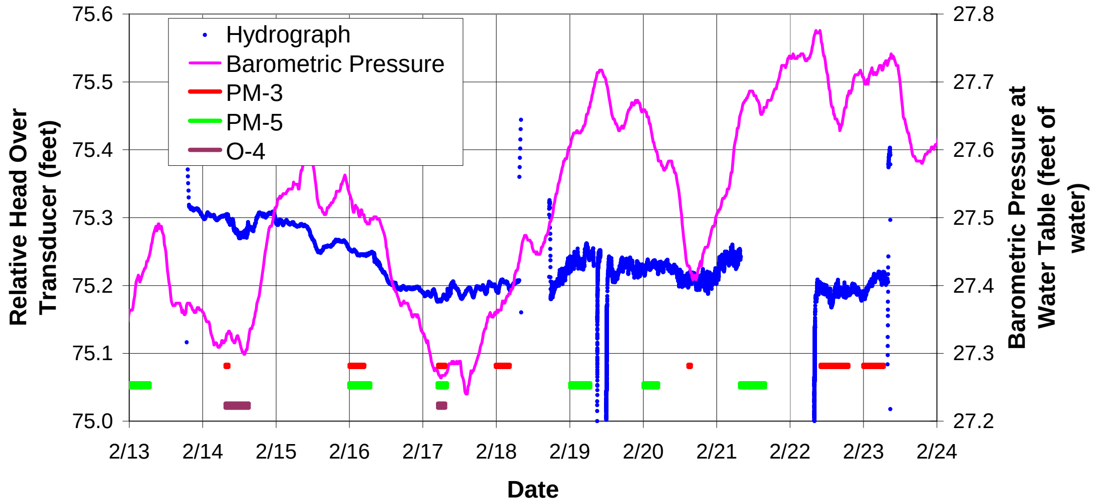

Figure C-7.0-6 R-44 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation

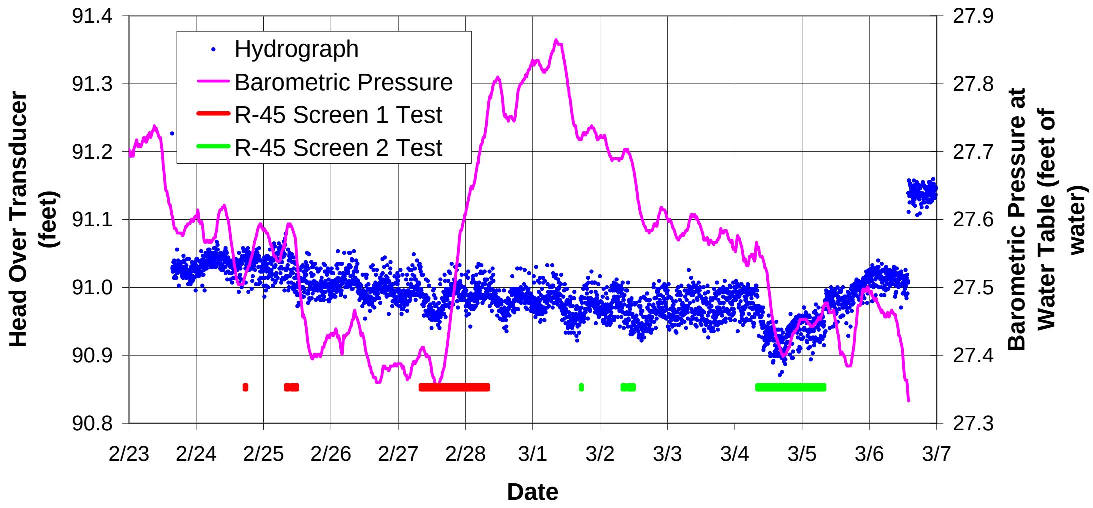

Figure C-7.0-7 R-44 screen 2 apparent hydrograph during R-45 screen 1 and 2 tests

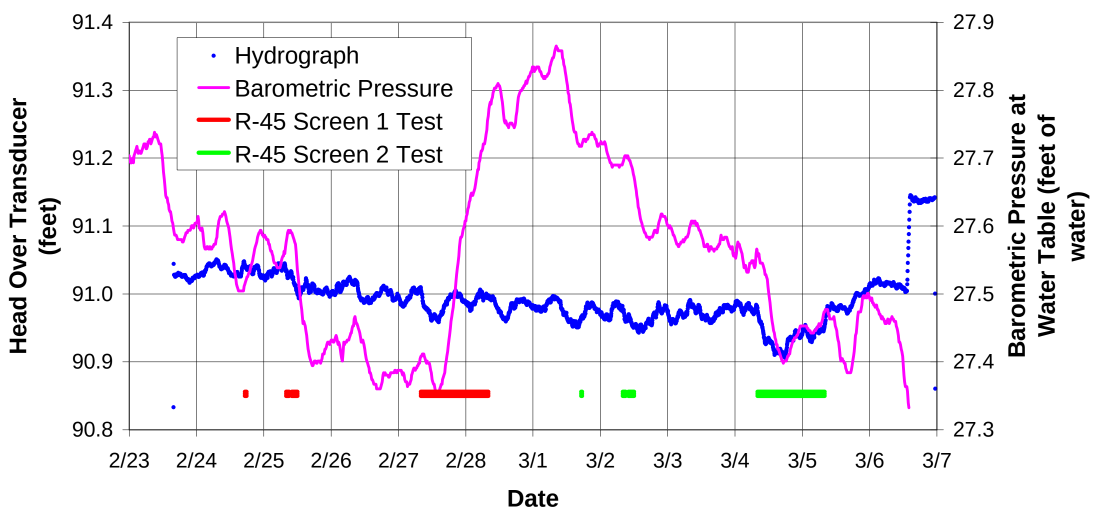

Figure C-7.0-8 R-44 screen 2 rolling average apparent hydrograph during R-45 screen 1 and 2 tests

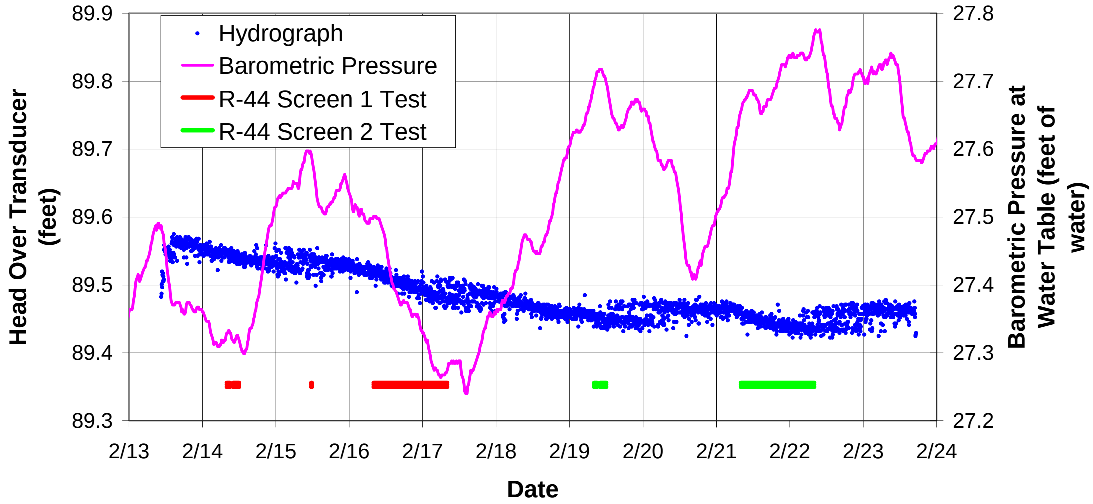

Figure C-7.0-9 R-45 screen 1 apparent hydrograph during R -44 screen 1 and 2 tests

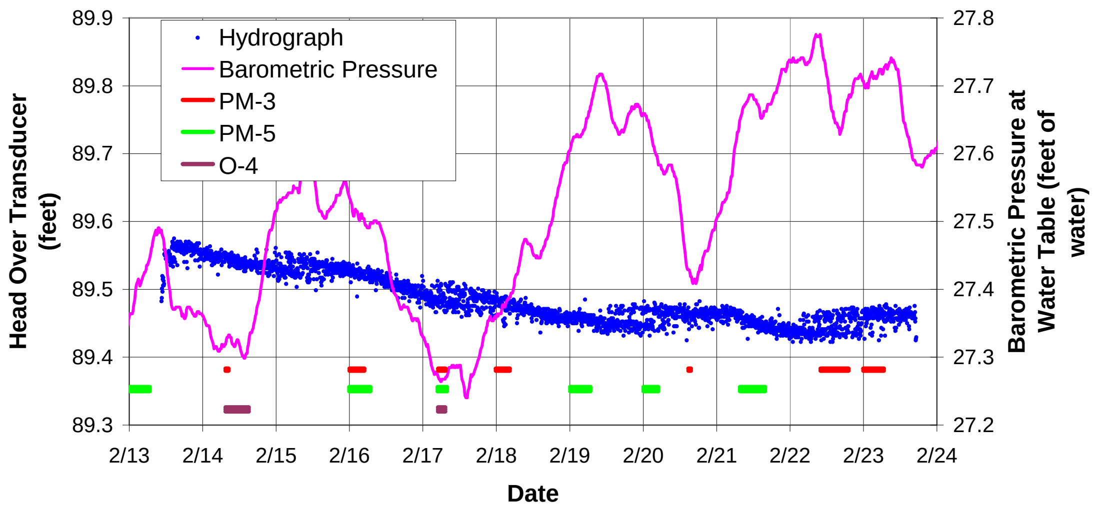

Figure C-7.0-10 R-45 screen 1 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation

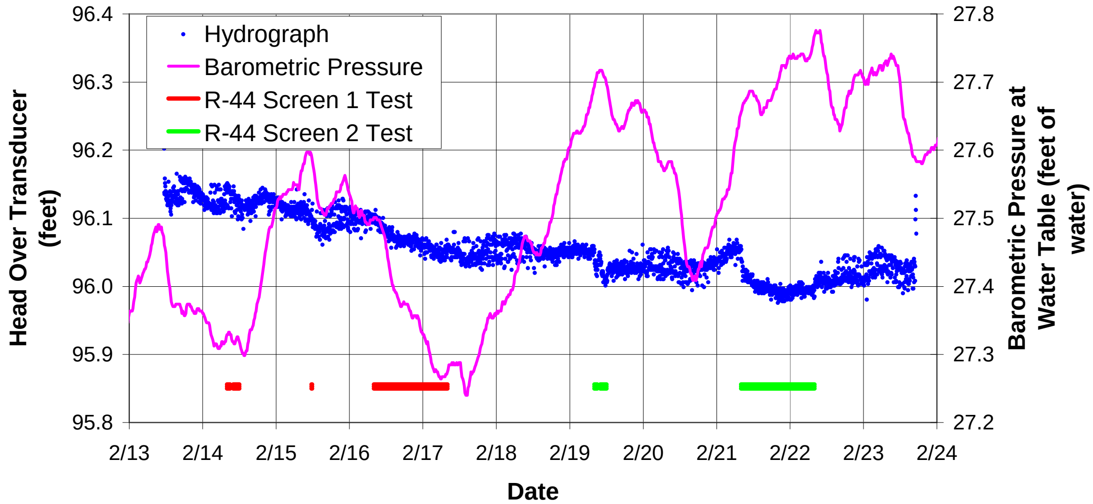

Figure C-7.0-11 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests

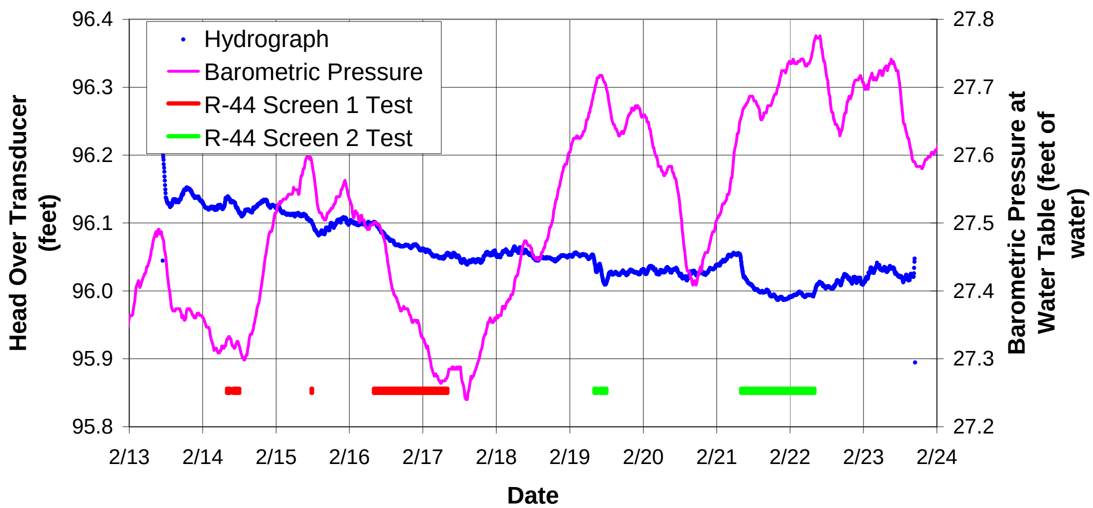

Figure C-7.0-12 R-45 screen 2 rolling average apparent hydrograph during R-44 screen 1 and 2 tests

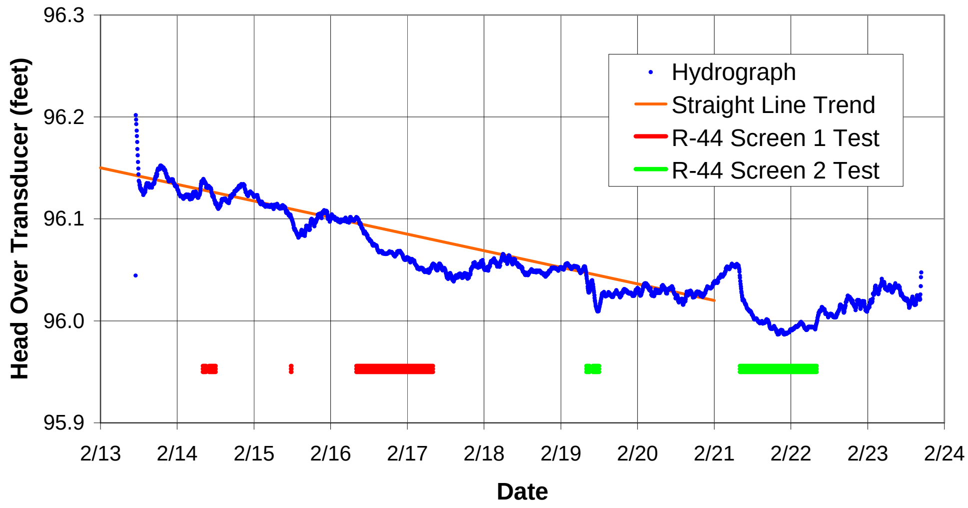

Figure C-7.0-13 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests—expanded scale

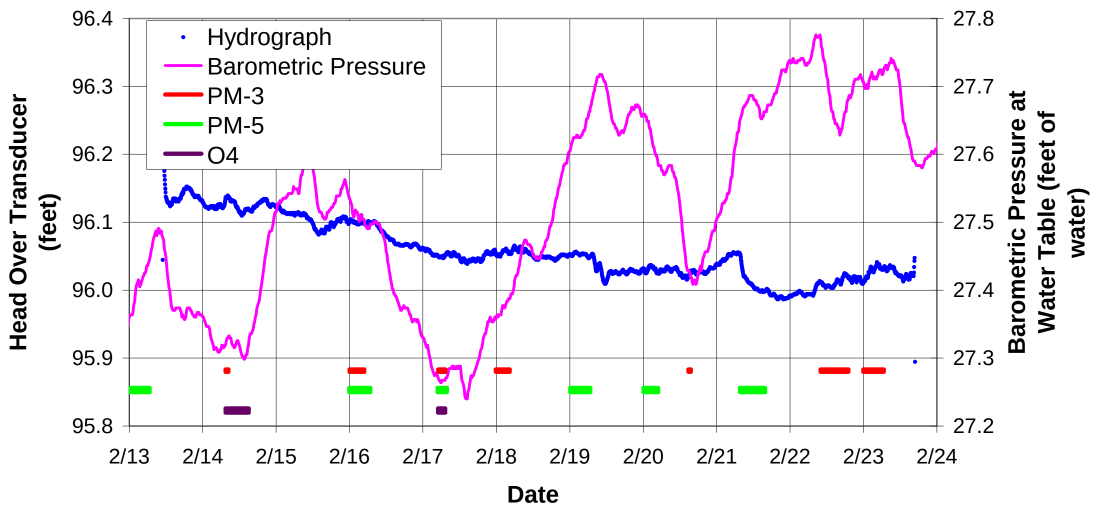

Figure C-7.0-14 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation

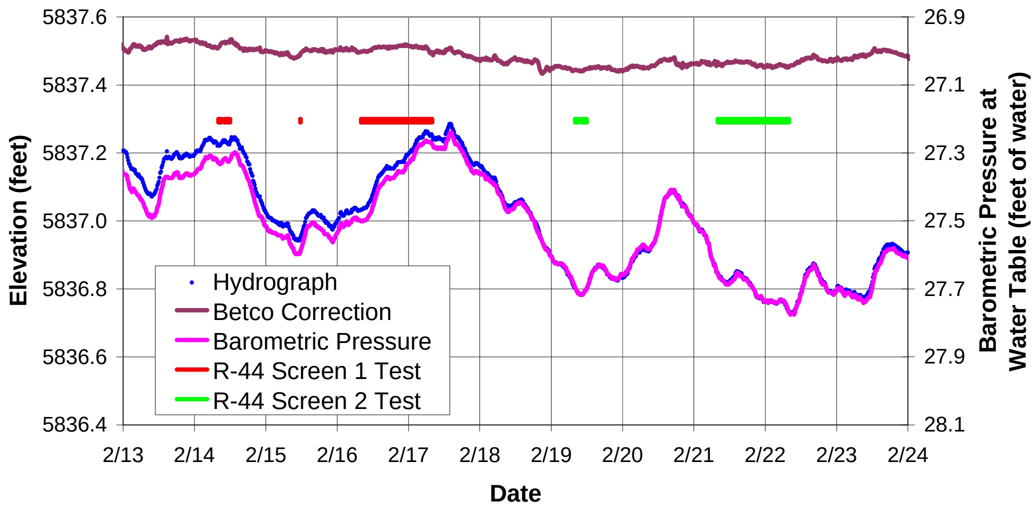

Figure C-7.0-15 R-11 hydrograph during R-44 screen 1 and 2 tests

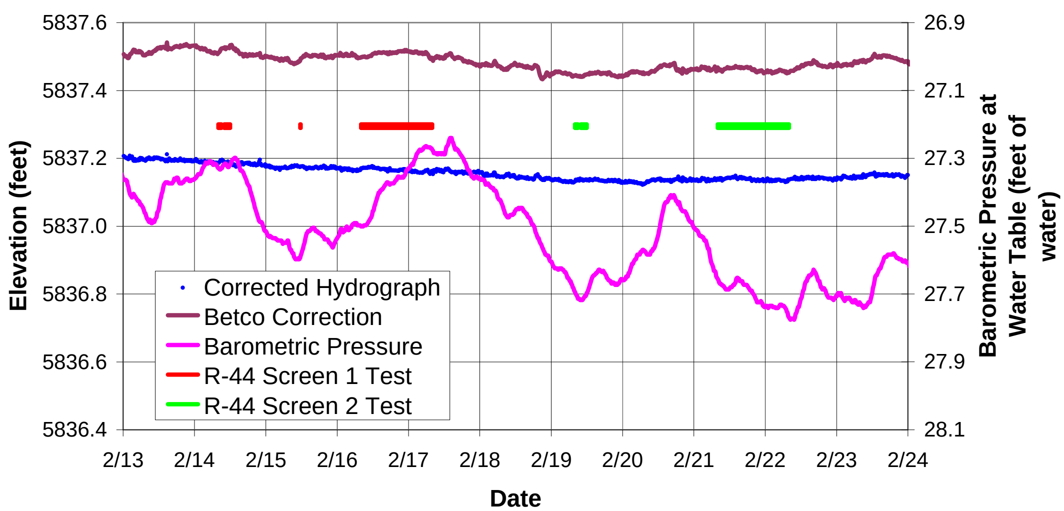

Figure C-7.0-16 R-11 corrected hydrograph during R-44 screen 1 and 2 tests

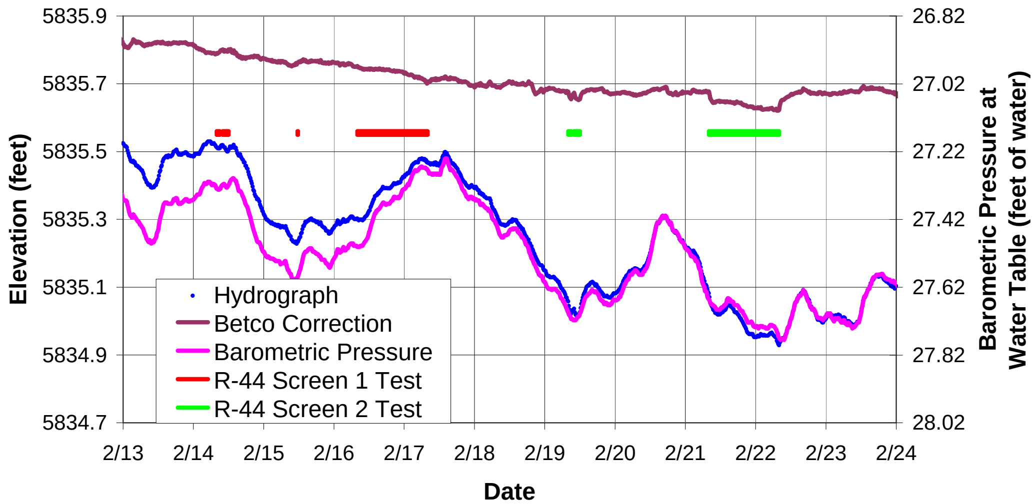

Figure C-7.0-17 R-13 hydrograph during R-44 screen 1 and 2 tests

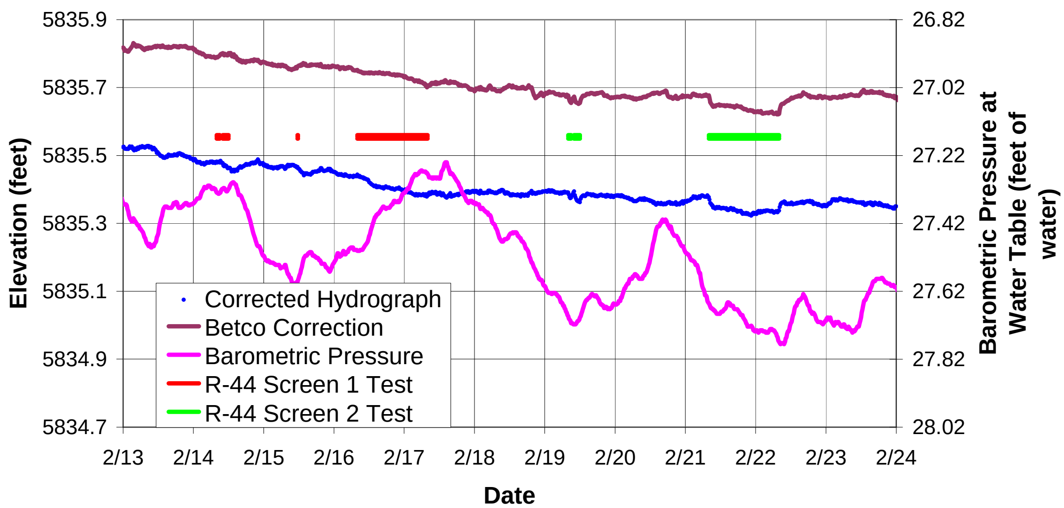

Figure C-7.0-18 R-13 corrected hydrograph during R-44 screen 1 and 2 tests

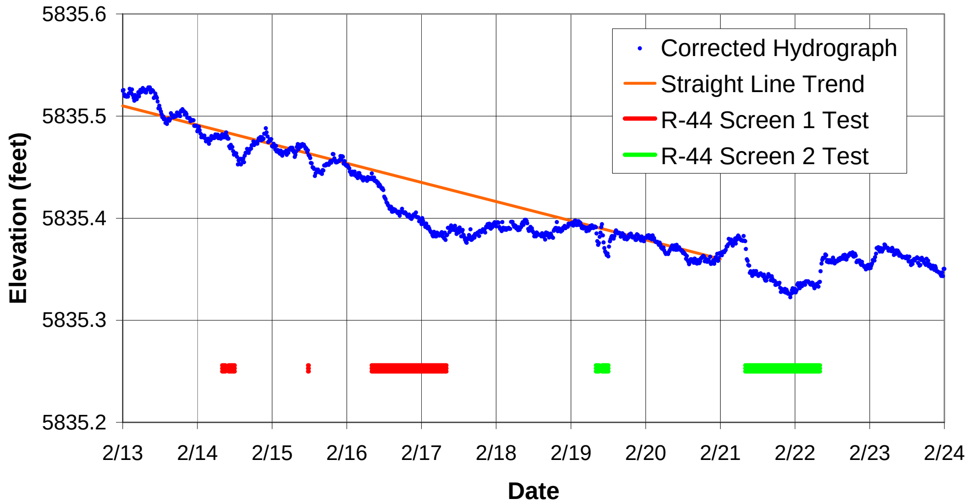

Figure C-7.0-19 R-13 corrected hydrograph during R-44 screen 1 and 2 tests—expanded scale

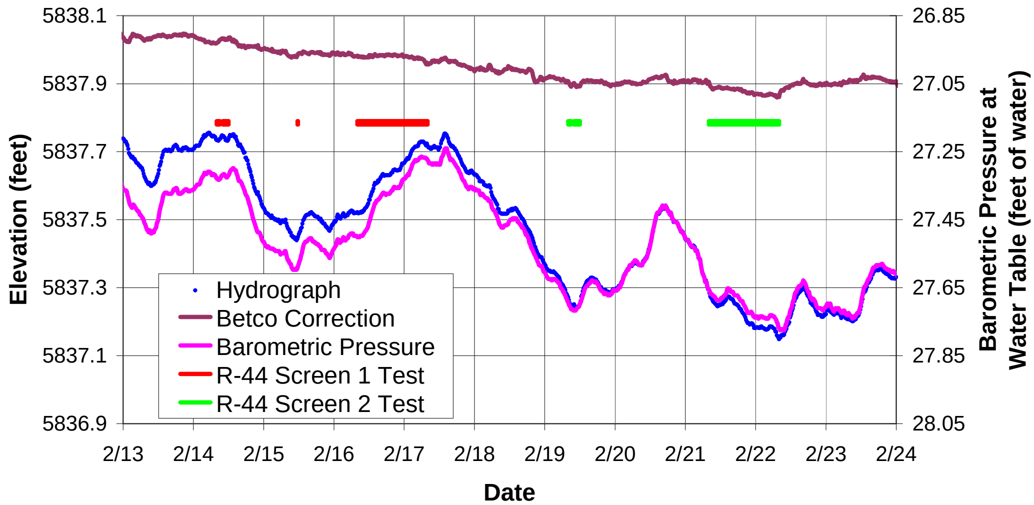

Figure C-7.0-20 R-28 hydrograph during R-44 screen 1 and 2 tests

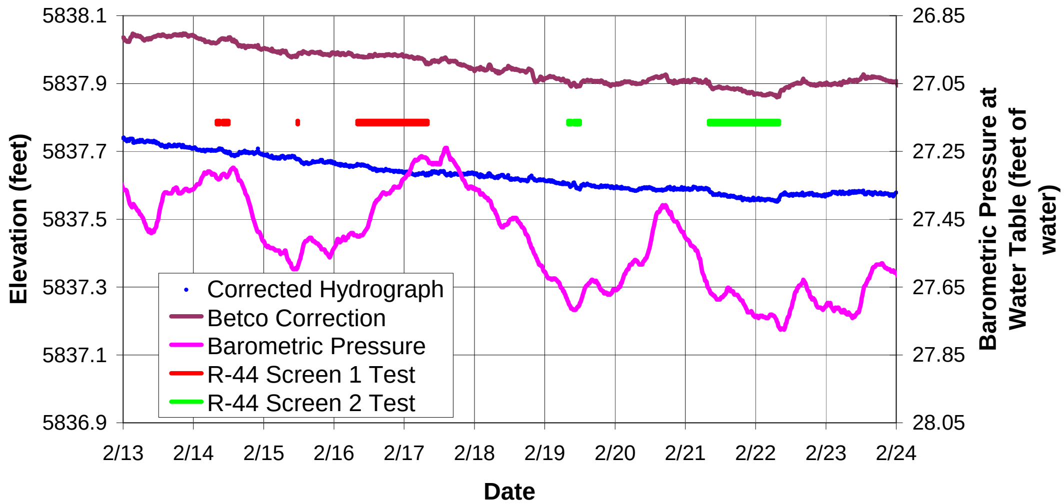

Figure C-7.0-21 R-28 corrected hydrograph during R-44 screen 1 and 2 tests

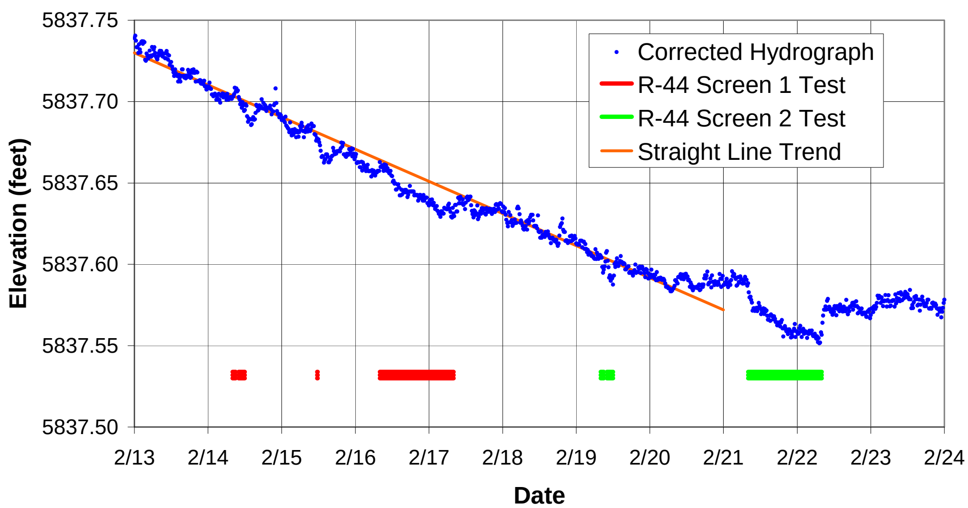

Figure C-7.0-22 R-28 corrected hydrograph during R-44 screen 1 and 2 tests—expanded scale

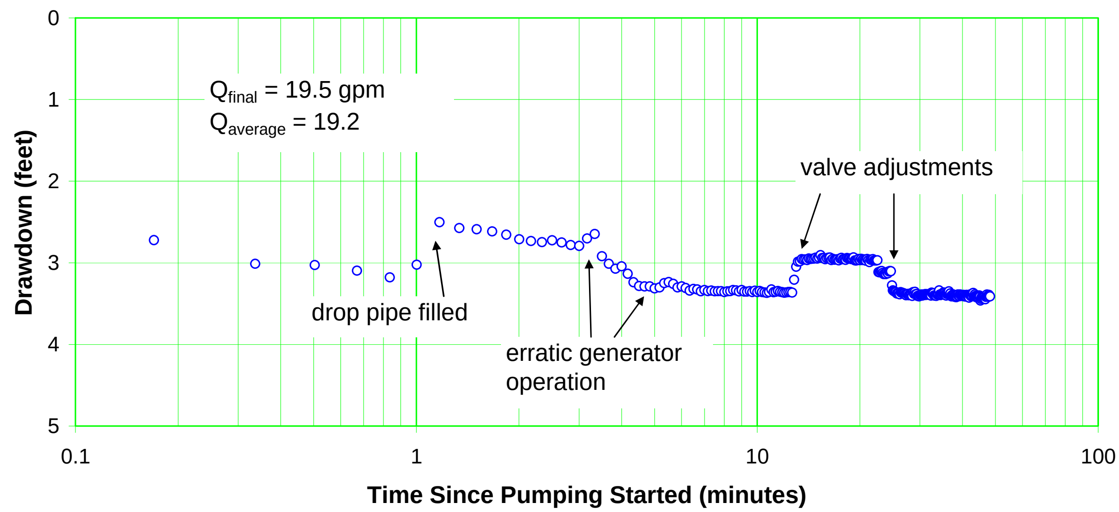

Figure C-8.0-1 Well R-44 screen 1 trail 1 drawdown

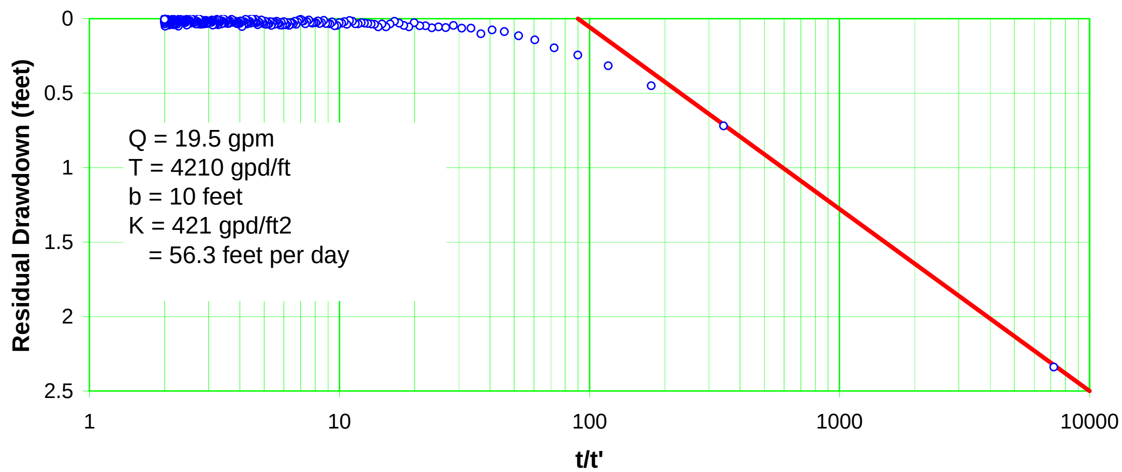

Figure C-8.0-2 Well R-44 screen 1 trail 1 recovery

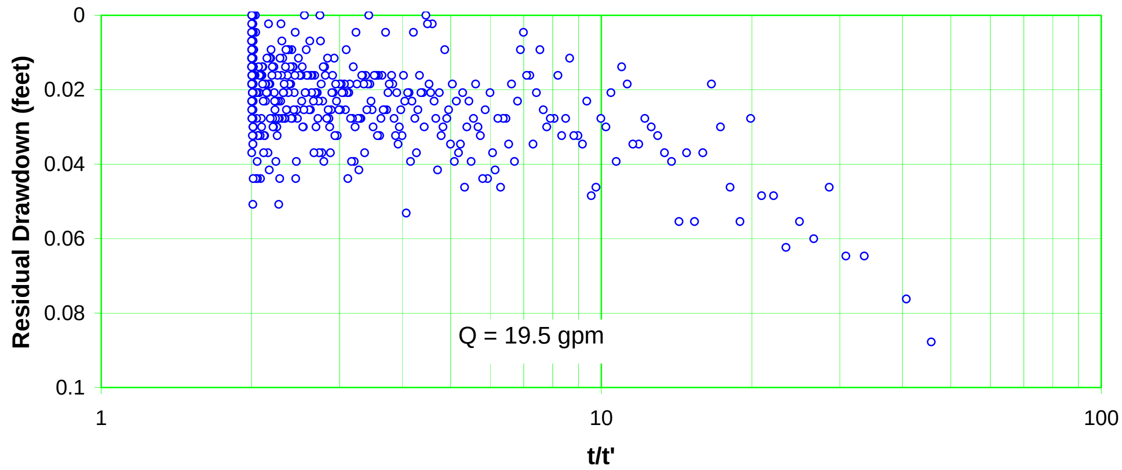

Figure C-8.0-3 Well R-44 screen 1 trail 1 recovery—expanded scale

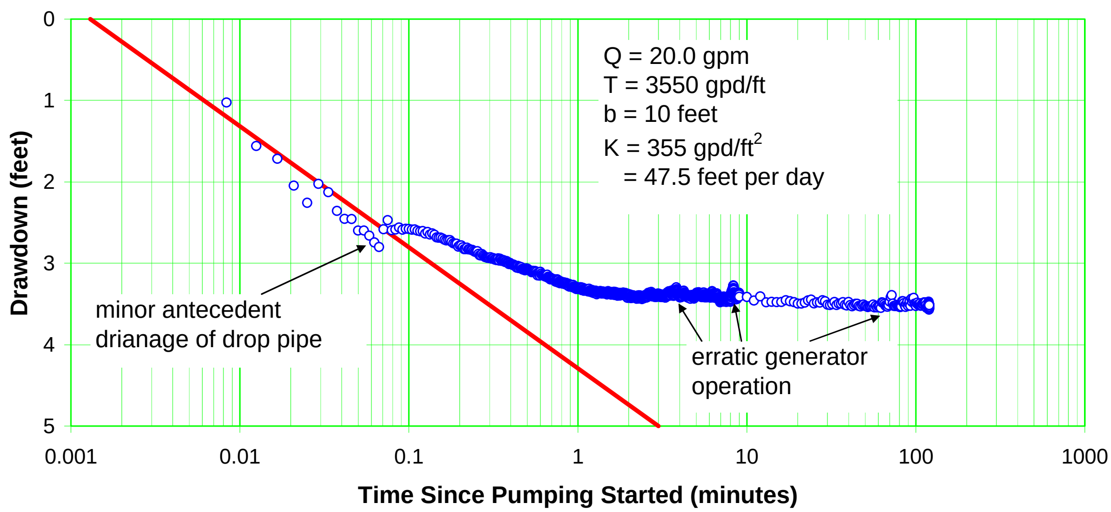

Figure C-8.0-4 Well R-44 screen 1 trail 1 drawdown

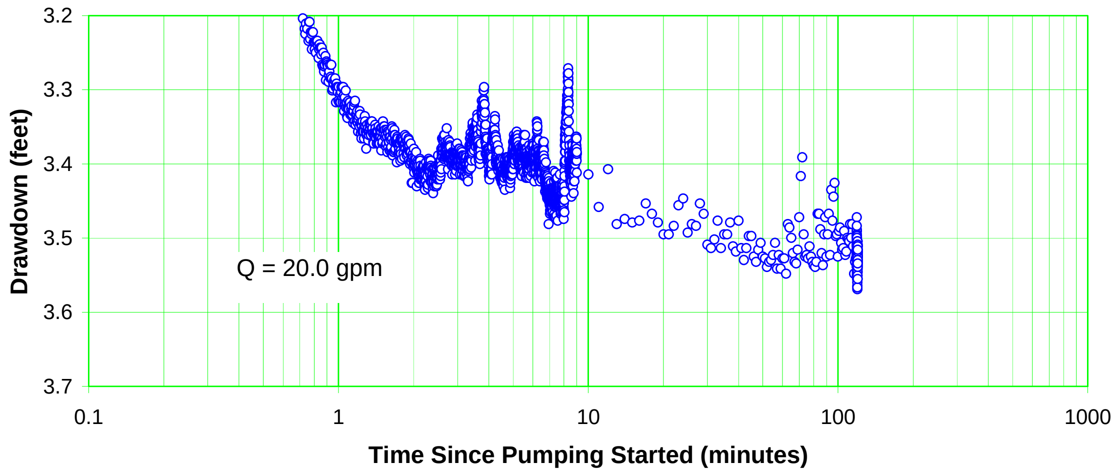

Figure C-8.0-5 Well R-44 screen 1 trail 2 drawdown—expanded scale

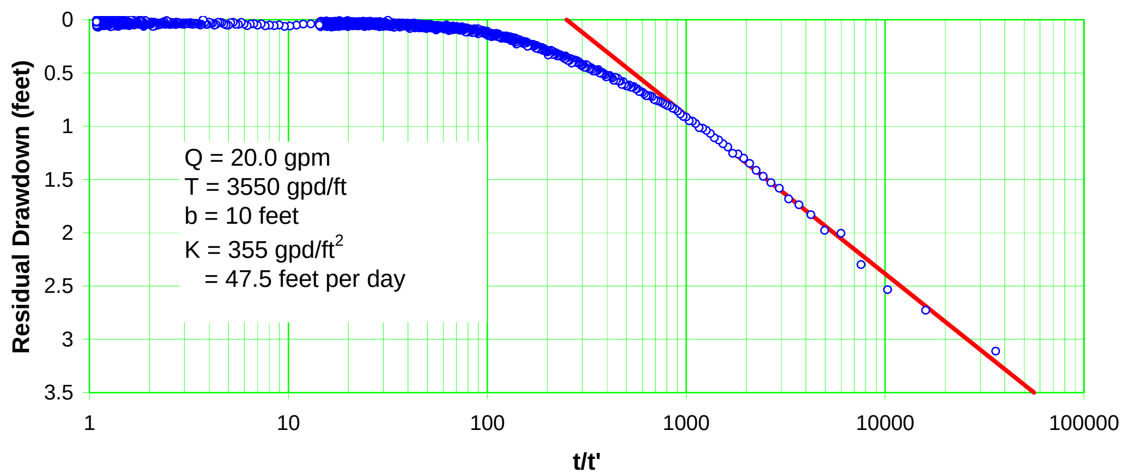

Figure C-8.0-6 Well R-44 screen 1 trail 2 recovery

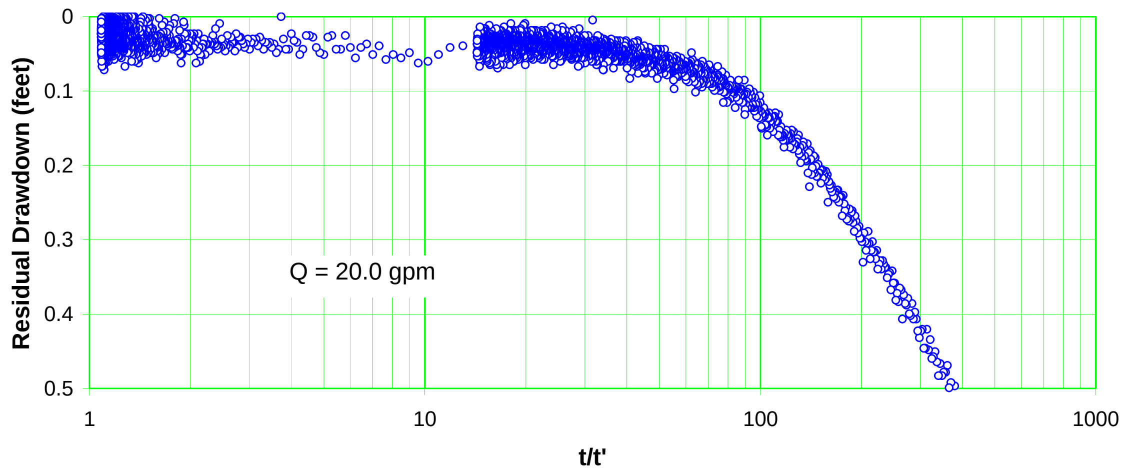

Figure C-8.0-7 Well R-44 screen 1 trail 2 recovery—expanded scale

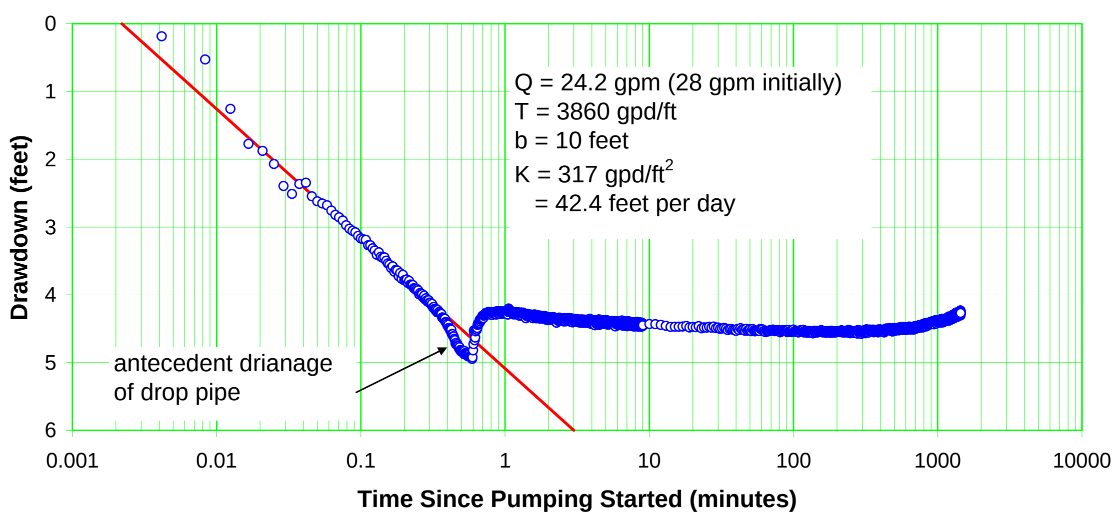

Figure C-8.1-1 Well R-44 screen 1 drawdown

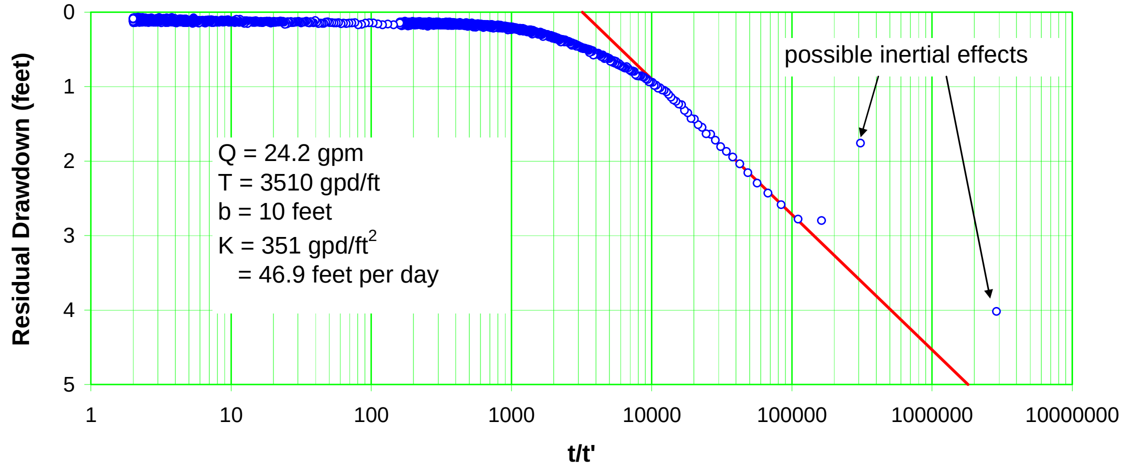

Figure C-8.1-2 Well R-44 screen 1 recovery

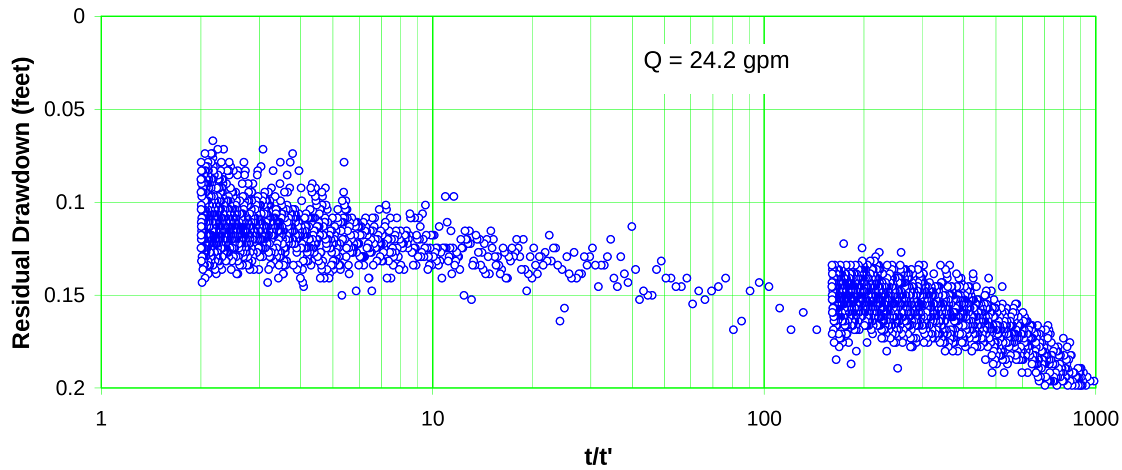

Figure C-8.1-3 Well R-44 screen 1 recovery—expanded scale

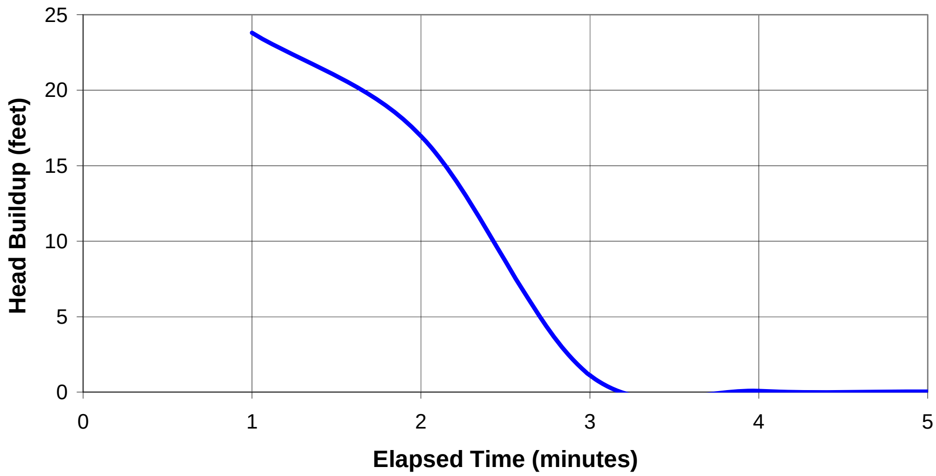

Figure C-8.1-4 Head buildup following packer deflation

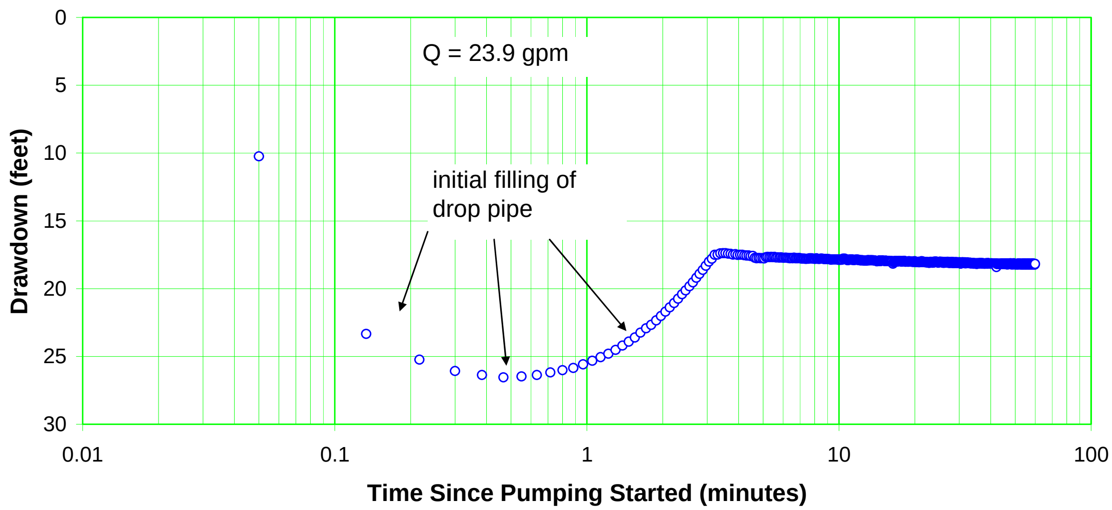

Figure C-9.0-1 Well R-44 screen 2 trail 1 drawdown

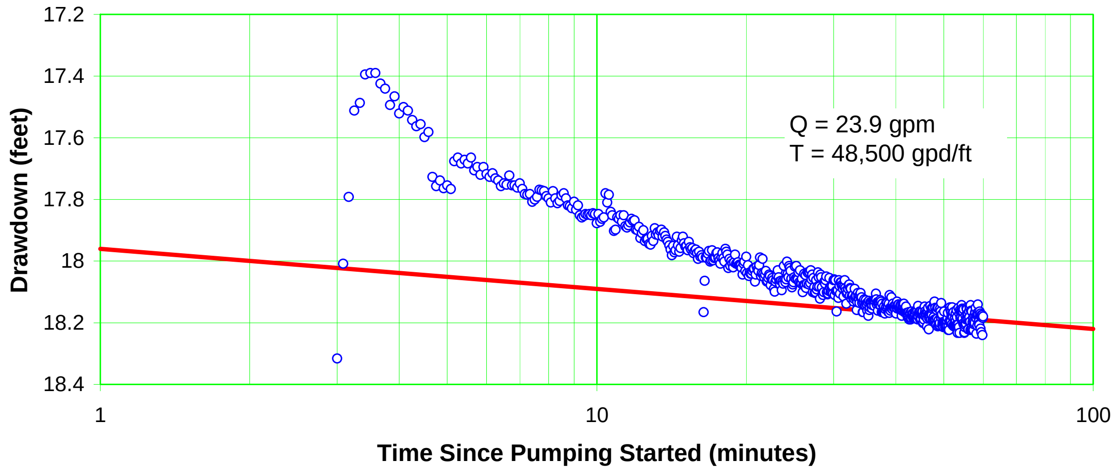

Figure C-9.0-2 Well R-44 screen 2 trial 1 drawdown—expanded scale

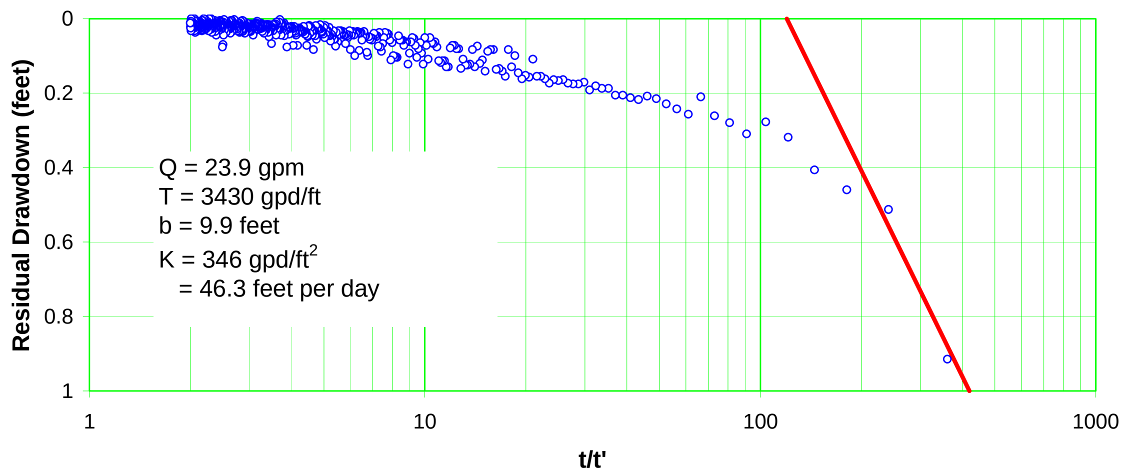

Figure C-9.0-3 Well R-44 screen 2 trial 1 recovery

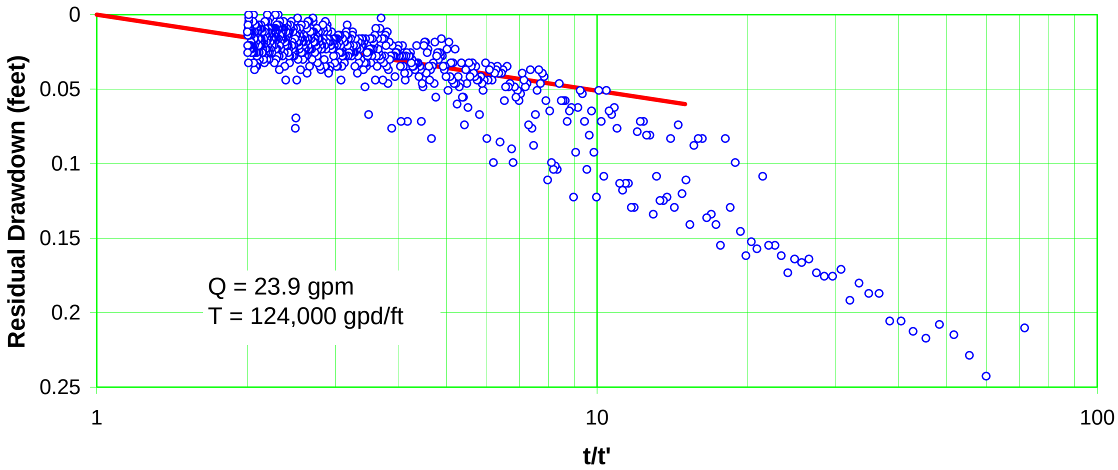

Figure C-9.0-4 Well R-44 screen 2 trial 1 recovery—expanded scale

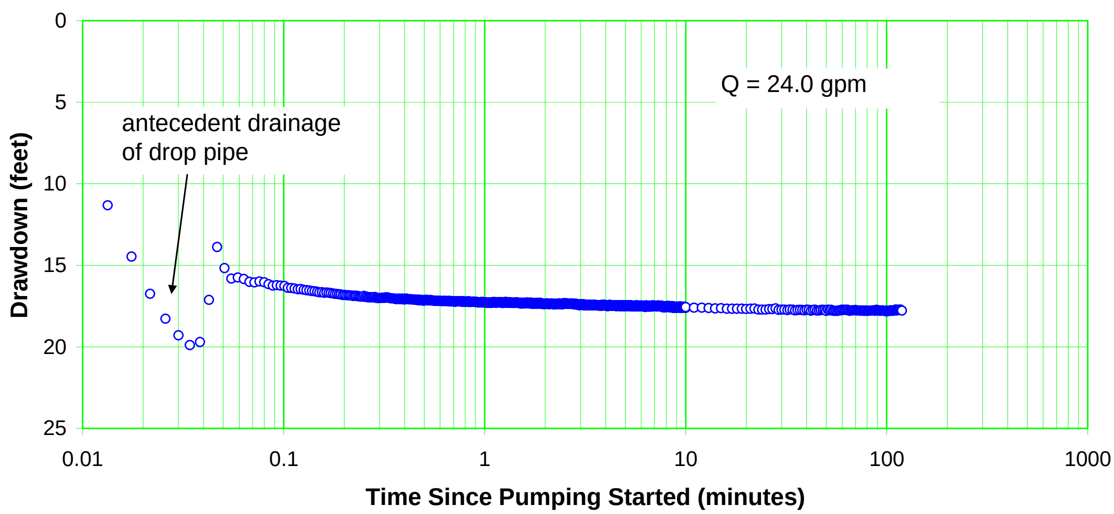

Figure C-9.0-5 Well R-44 screen 2 trial 2 drawdown

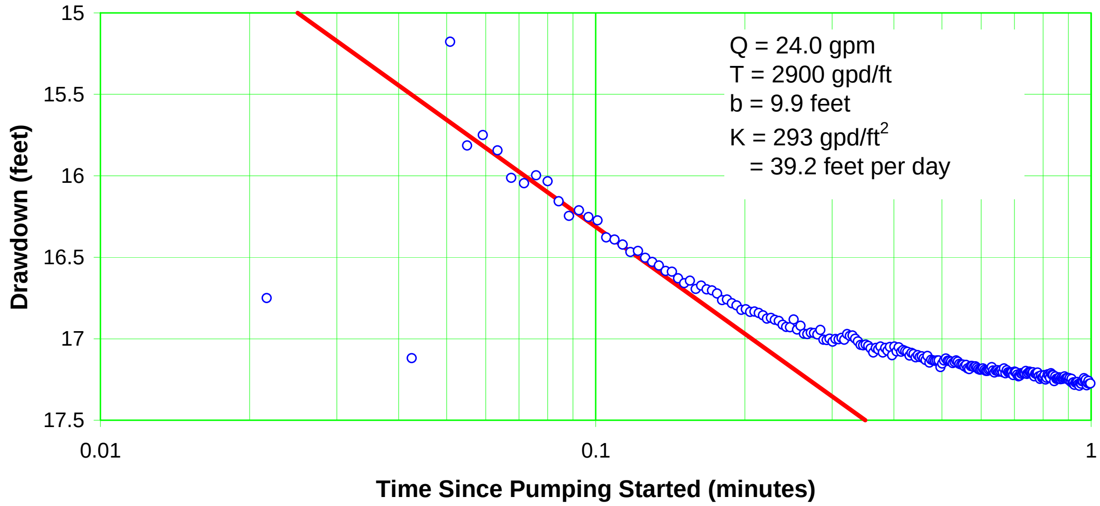

Figure C-9.0-6 Well R-44 screen 2 trial 2 drawdown—early data

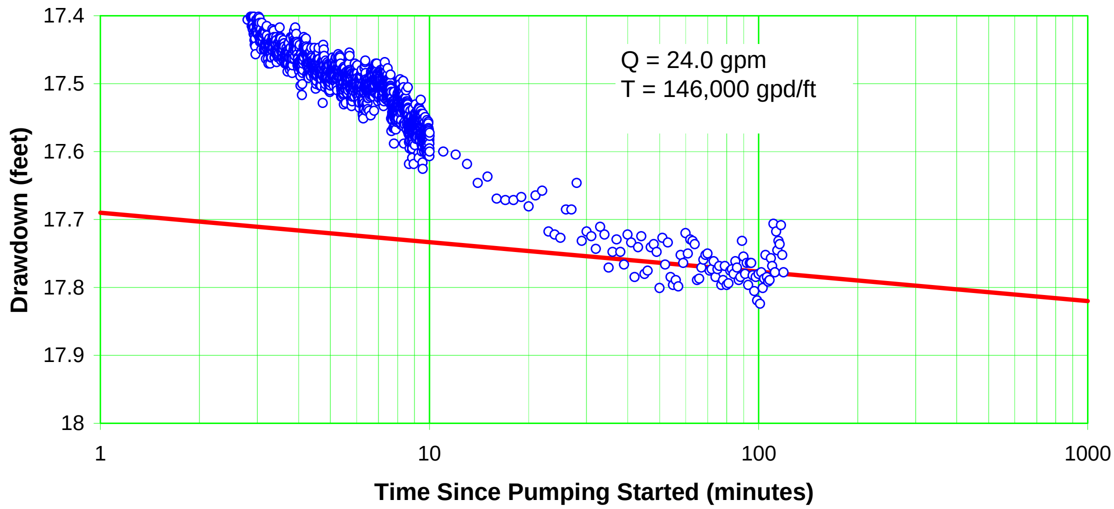

Figure C-9.0-7 Well R-44 screen 2 trial 2 drawdown—late data

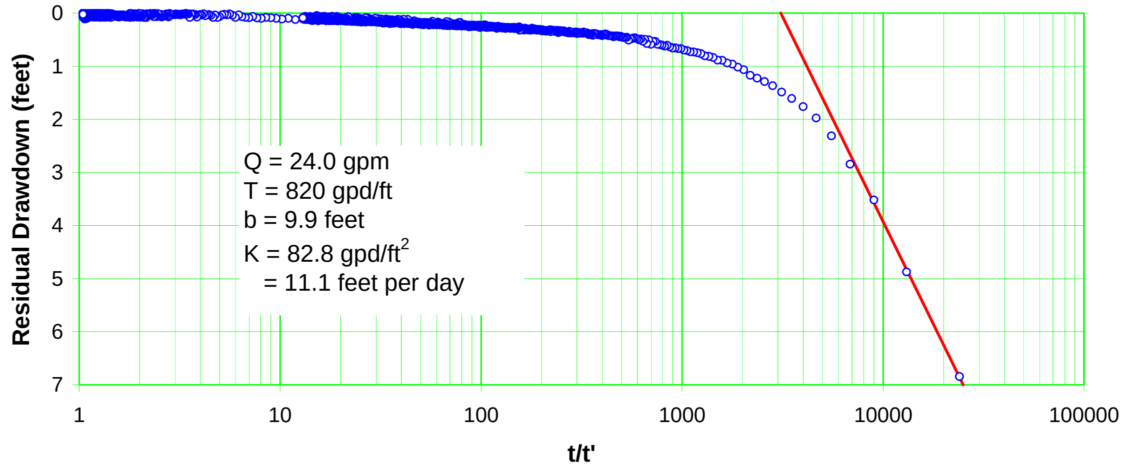

Figure C-9.0-8 Well R-44 screen 2 trial 2 recovery

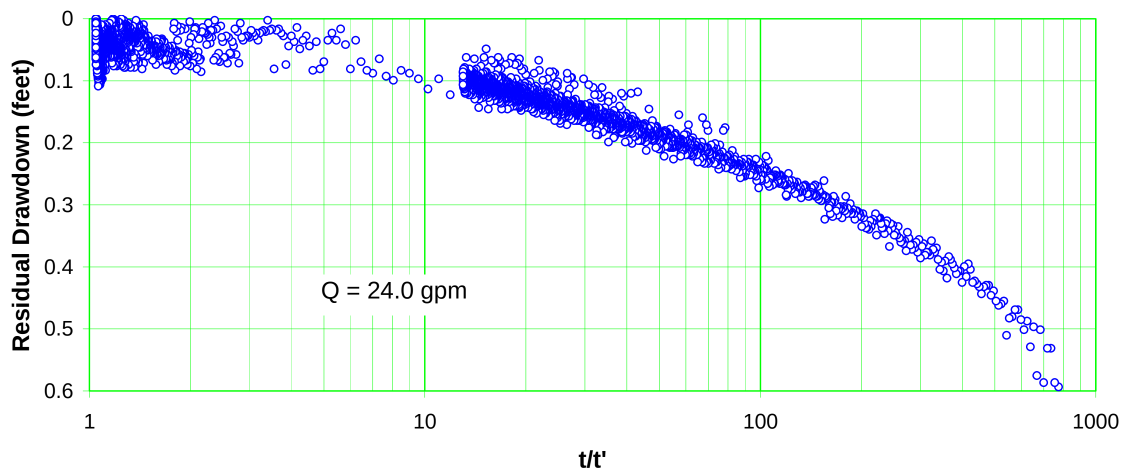

Figure C-9.0-9 Well R-44 screen 2 trial 2 recovery—expanded scale

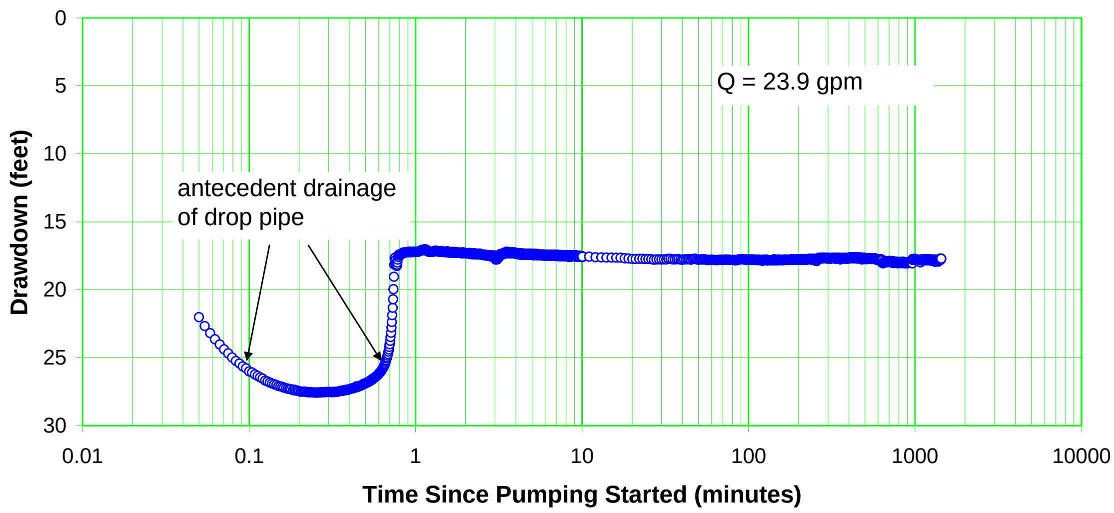

Figure C-9.1-1 Well R-44 screen 2 drawdown

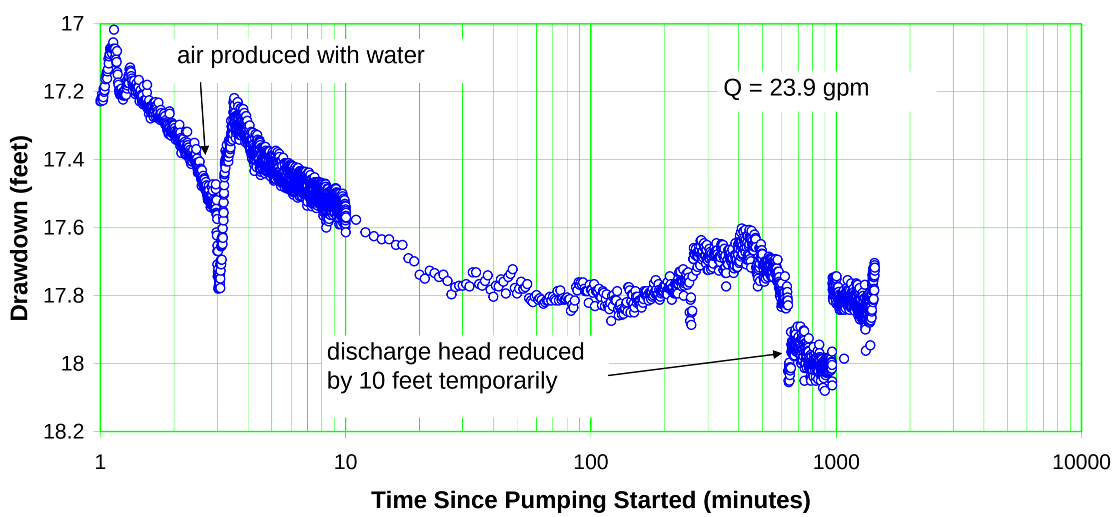

Figure C-9.1-2 Well R-44 screen 2 drawdown—expanded scale

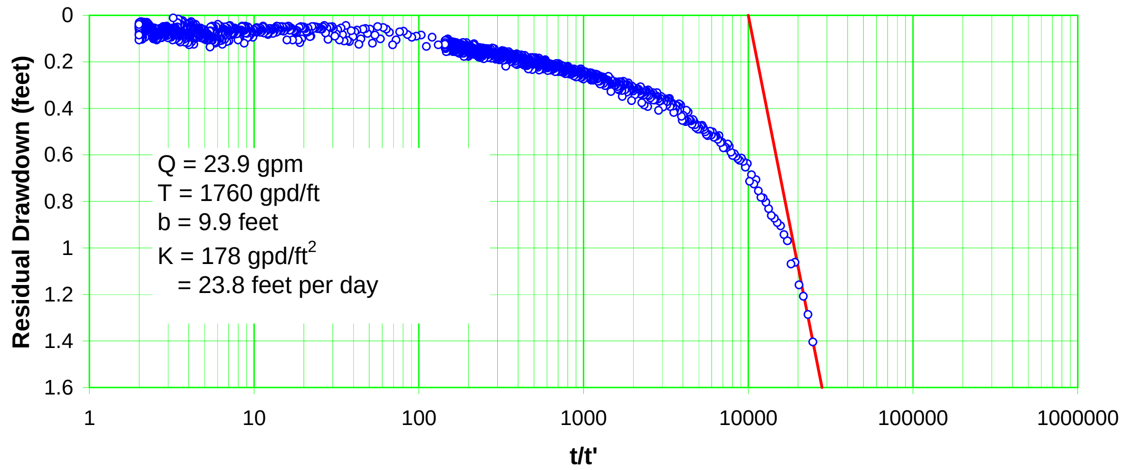

Figure C-9.1-3 Well R-44 screen 2 recovery

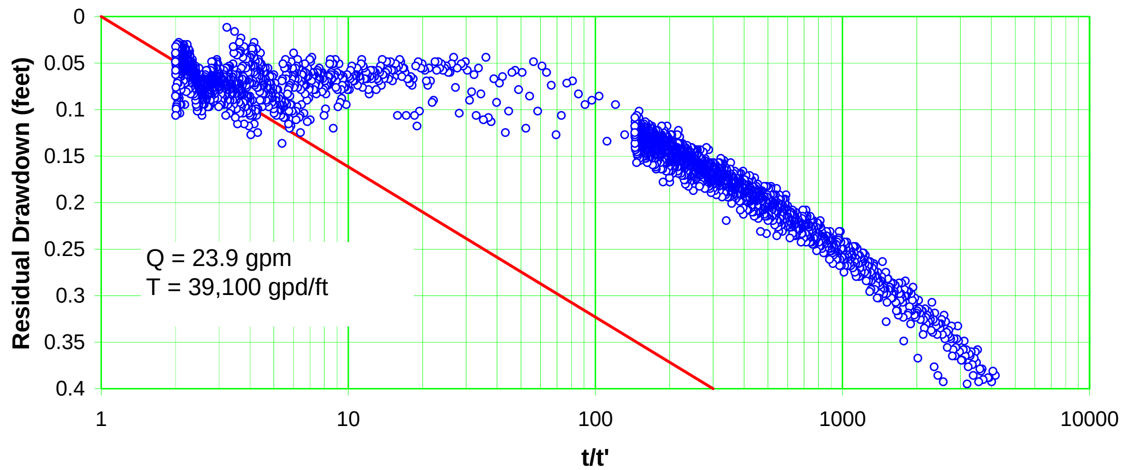

Figure C-9.1-4 Well R-44 screen 2 recovery—expanded scale

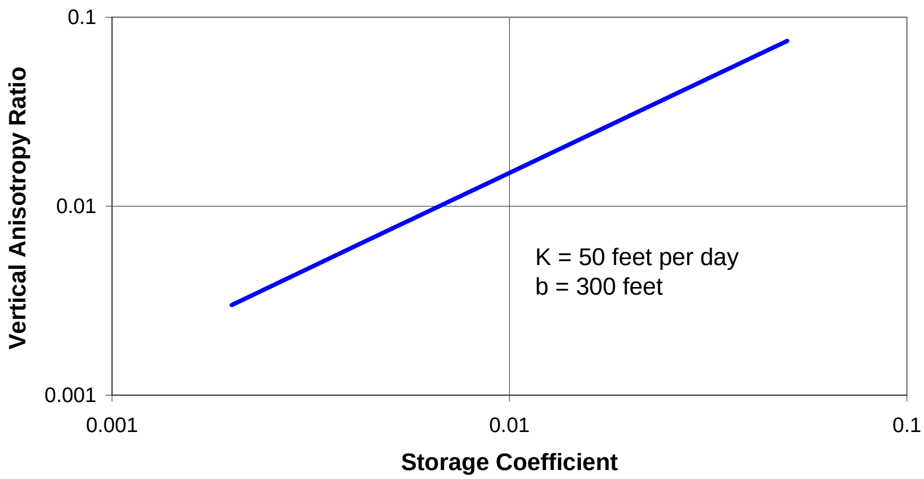

Figure C-10.0-1 Correlation of storage coefficient and anisotropy

Table C-8.1-1
R-44 Screen 1 Pumping Test Results

Analysis	Hydraulic Conductivity (ft/d)
Trial 1 Recovery	56.3
Trial 2 Drawdown	47.5
Trial 2 Recovery	47.5
24-H Drawdown	42.5
24-H Recovery	46.9
Recovery Average	50.2

Table C-11.0-1
R-44 Interference Effects

Drawdown (ft)			
Well Name (Screen ID)	Pump PM-4	Pump R-44 Screen 1	Pump R-44 Screen 2
R-44 Screen 1	0.03 (2.5 d)	n/a[a]	0.05
R-44 Screen 2	0.06 (2.5 d)	0.05	n/a
R-45 Screen 1	0.11 (6 d)	0.00	0.02
R-45 Screen 2	0.04 (3 d)	0.02	0.06
R-11	0.06 (6 d)	0.00	0.00
R-13	0.15 (7 d)	0.03[b]	0.06
R-28	0.14 (7 d)	0.01[b]	0.03

[a] n/a = Not applicable.

[b] Subtle effect.

Appendix D

Borehole Video Logging
(on DVD included with this document)

Appendix E

Schlumberger Geophysical Logging Report
(on CD included with this document)

Appendix C

Borehole Video Logging
(on DVD included with this document)

N3B RECORDS	
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